



PDP SWQMP

PRIORITY DEVELOPMENT PROJECT (PDP) STORM WATER QUALITY MANAGEMENT PLAN (SWQMP)

Project Name Otay Ranch Village 7, R-3, R-4 & R-8 TM

Assessor's Parcel Number(s) 644-241-06, 07,08 & 644-241-10

Permit Application Number TM 23-0001

Drawing Numbers N/A

CIVIL ENGINEER NAME: Alisa S. Vialpando ; PE # 47945



Wet Signature and Stamp



PREPARED FOR: Applicant Name: Baldwin & Sons LLC & Otay Project, L.P.

Address: 20 Corporate Plaza Dr.

Newport Beach, CA 92660

Telephone # (949) 640-8300

PREPARED BY: Company Name: Hunsaker & Associates, San Diego Inc.

Address: 9707 Waples Street

San Diego, CA 92121

Telephone # (858) 558-4500

DATE: 07/25/2024

Approved By: City of Chula Vista
(print Name & Sign)

Date: _____

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ACRONYMS

APN	Assessor's Parcel Number
BMP	Best Management Practice
HMP	Hydromodification Management Plan
HSG	Hydrologic Soil Group
MS4	Municipal Separate Storm Sewer System
N/A	Not Applicable
NRCS	Natural Resources Conservation Service
PDP	Priority Development Project
PE	Professional Engineer
SC	Source Control
SD	Site Design
SDRWQCB	San Diego Regional Water Quality Control Board
SIC	Standard Industrial Classification
SWQMP	Storm Water Quality Management Plan

Otay Ranch Village 7, R-3, R-4, & R-8

Project Name/ _____

Certification Page

Otay Ranch Village 7, R-8 & R-4

Project Name: _____

Permit Application Number: TM 23-0001

I hereby declare that I am the Engineer in Responsible Charge of design of storm water best management practices (BMPs) for this project, and that I have exercised responsible charge over the design of the BMPs as defined in Section 6703 of the Business and Professions Code, and that the design is consistent with the PDP requirements of the City of Chula Vista BMP Design Manual, which is based on the requirements of the San Diego Regional Water Quality Control Board Order No. R9-2013-0001 as amended by R9-2015-0001 and R9-2015-0100 (MS4 Permit).

I have read and understand that the City Engineer has adopted minimum requirements for managing urban runoff, including storm water, from land development activities, as described in the BMP Design Manual. I certify that this PDP SWQMP has been completed to the best of my ability and accurately reflects the project being proposed and the applicable BMPs proposed to minimize the potentially negative impacts of this project's land development activities on water quality. I understand and acknowledge that the plan check review of this PDP SWQMP by the City Engineer is confined to a review and does not relieve me, as the Engineer in Responsible Charge of design of storm water BMPs for this project, of my responsibilities for project design.

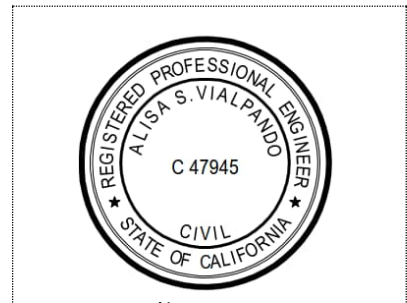
Alisa S. Vialpando
Engineer of Work's Signature

07/25/2024
Date

47945, 12/31/23
PE # Expiration Date

Alisa S. Vialpando
Print Name

Hunsaker & Associates, San Diego Inc.
Company



Otay Ranch Village 7, R-3, R-4, & R-8

Project Name/ _____

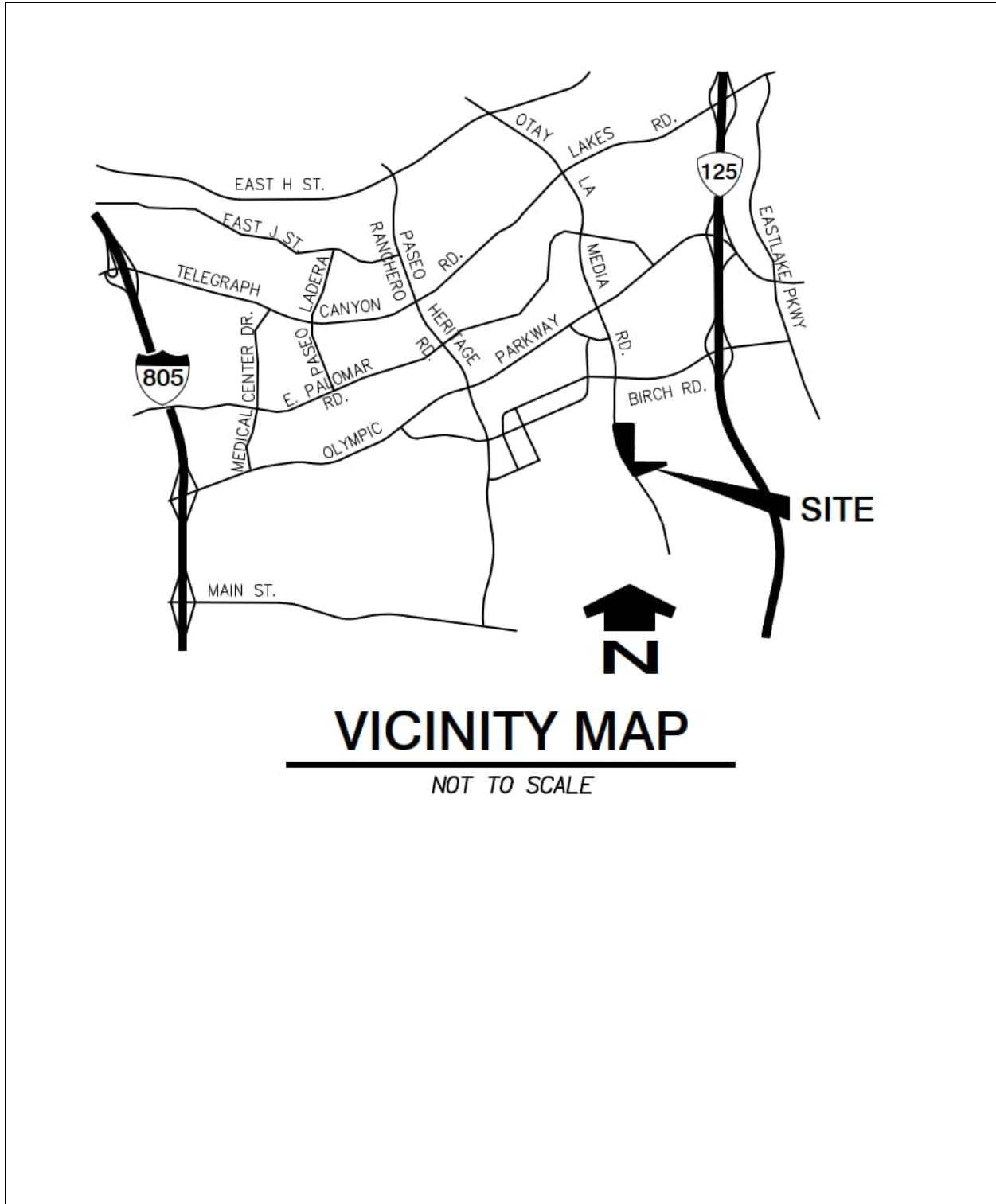
SUBMITTAL RECORD

Use this Table to keep a record of submittals of this PDP SWQMP. Each time the PDP SWQMP is re-submitted, provide the date and status of the project. In column 4 summarize the changes that have been made or indicate if response to plancheck comments is included. When applicable, insert response to plancheck comments behind this page.

Submittal Number	Date	Project Status	Summary of Changes
1	04/07/2023	<input checked="" type="checkbox"/> Preliminary Design / Planning/ CEQA <input type="checkbox"/> Final Design	Initial Submittal
2	07/25/2024	<input checked="" type="checkbox"/> Preliminary Design / Planning/ CEQA <input type="checkbox"/> Final Design	Addressed plan check comments
3		<input type="checkbox"/> Preliminary Design / Planning/ CEQA <input type="checkbox"/> Final Design	
4		<input type="checkbox"/> Preliminary Design / Planning/ CEQA <input type="checkbox"/> Final Design	



Project Vicinity Map



Project Name/ _____

**Insert Completed Intake Form (Storm Water Requirements
Applicability Checklist)**

<https://www.chulavistaca.gov/departments/public-works/services/storm-water-pollution-prevention/documents-and-reports>



Storm Water Requirements Applicability Checklist for All Permit Applications

Intake Form

August 2023 Update

Project Information

Project Address: La Media Road and Santa Luna Street	Project Application # TM 23-0001	
Project Name: Otay Ranch Village 7, R-3, R-4, & R-8	APN(s) 644-241-06,07,08 & 644-241-10	
	Parcel Area (ft ²) 818,811	Project Disturbed Area (ft ²) 745,217

Brief Description of Work Proposed:
Twenty nine (29) multi-family residential buildings, recreational and open space areas, associated roads, driveways, and sidewalks. One DG access road to the Vortac site

The project is (select one):

New Development Total Impervious Area 374,952 **ft²**
(New development is the creation of impervious surface on an undeveloped site.)

Redevelopment Total new and/or replaced Impervious Area _____ **ft²**
(Redevelopment is the creation and/or replacement of impervious surface on an already developed site).

Other: _____
(Includes projects such as: Roof Mount Solar, Residential Minor Utility, Residential Interior Remodel, Commercial Interior Tenant Improvement, Cell Site Modification, etc.)

Name of Person Completing this Form: _____

Role: Property Owner Contractor Architect Engineer Other _____

Email: AVialpando@HunsakerSD.com Phone: (858) 558-4500

Signature: Alisa S. Vialpando Date Completed: 07/25/2024

Answer each section below, starting with Section 1 and progressing through each section. Additional information for determining the requirements is found in the Chula Vista BMP Design Manual available on the City's website at <http://www.chulavistaca.gov/departments/public-works/services/storm-water-pollution-prevention/documents-and-reports>.

SECTION 1: Storm Water BMP Requirements

Please answer the following two questions:

1) Does the project involve repair or improvements to an existing building or structure that do not alter the size such as: tenant improvements, interior remodeling, electrical work, fire alarm, fire sprinkler system, HVAC work, gas, plumbing, etc.?	<input type="checkbox"/> Yes <input checked="" type="checkbox"/> No
2) Does the project involve routine maintenance activities such as: roof or exterior structure surface replacement; resurfacing existing roadways and parking lots including dig outs, slurry seal, overlay and restriping; repair damaged sidewalks or pedestrian ramps on existing roads without expanding the impervious footprint; routine replacement of damaged pavement, trenching and resurfacing associated with utility work (i.e. sewer, water, gas or electrical laterals, etc.), and pot holing or geotechnical investigation borings?	<input type="checkbox"/> Yes <input checked="" type="checkbox"/> No

CHECK ONE:

If you answered **YES** to **either** question 1 or 2, review and sign "**Construction Storm Water BMP Certification Statement**" on Page 2. **DO NOT** complete Sections 2, 3, 4, or 5. The Project is **NOT** subject to Permanent Storm Water BMP requirements. It **IS** subject to Construction BMP requirements.

If you answered **NO** to **both** questions 1 and 2, **Skip to Section 2, Page 3.**

Construction Storm Water BMP Certification Statement

The following storm water quality protection measures are required by City Chula Vista Municipal Code Chapter 14.20 and the City's Jurisdictional Runoff Management Program.

- 1) All applicable construction BMPs and non-stormwater discharge BMPs shall be installed and maintained for the duration of the project in accordance with the Appendix K "Construction BMP Standards" of the Chula Vista BMP Design Manual.
- 2) Erosion control BMPs shall be implemented for all portions of the project area in which no work has been done or is planned to be done over a period of 14 or more days. All onsite drainage pathways that convey concentrated flows shall be stabilized to prevent erosion.
- 3) Run-on from areas outside the project area shall be diverted around work areas to the extent feasible. Run-on that cannot be diverted shall be managed using appropriate erosion and sediment control BMPs.
- 4) Sediment control BMPs shall be implemented, including providing fiber rolls, gravel bags, or other equally effective BMPs around the perimeter of the project to prevent transport of soil and sediment offsite. Any sediment tracked onto offsite paved areas shall be removed via sweeping at least daily.
- 5) Trash and other construction wastes shall be placed in a designated area at least daily and shall be disposed of in accordance with applicable requirements.
- 6) Materials shall be stored to avoid being transported in storm water runoff and non-storm water discharges. Concrete washout shall be directed to a washout area and shall not be washed out to the ground.
- 7) Stockpiles and other sources of pollutants shall be covered when the chance of rain within the next 48 hours is at least 50%.

I certify that the storm water quality protection measures listed above will be implemented at the project described on Intake Form. I understand that failure to implement these measures may result in monetary penalties or other enforcement actions. This certification is signed under penalty of perjury and does not require notarization.

Name: _____ Title: _____

Signature: _____ Date: _____

SECTION 2: Determine if Project is a Standard Project or Priority Development Project**Is the project in any of the following categories, (a) through (f)?**

- (a) New development that **creates 10,000 square feet** or more of impervious surfaces (collectively over the entire project site). This includes commercial, industrial, residential, mixed-use, and public development projects on public or private land. **Yes** **No**
- (b) Redevelopment project that **creates and/or replaces 5,000 square feet** or more of impervious surface (collectively over the entire project site on an existing site of 10,000 square feet or more of impervious surfaces). This includes commercial, industrial, residential, mixed-use, and public development projects on public or private land. **Yes** **No**
- (c) New development or redevelopment projects that **creates and/or replaces a combined total of 5,000 square feet** or more of impervious surface (collectively over the entire project site) and support one or more of the following uses: **Yes** **No**
- (i) **Restaurant.** This category is defined as a facility that sells prepared foods and drinks for consumption, including stationary lunch counters and refreshment stands selling prepared foods and drinks for immediate consumption (Standard Industrial Classification Code 5812).
- (ii) **Hillside development projects.** This category includes development on any natural slope that is twenty-five percent or greater.
- (iii) **Parking Lots.** This category is defined as a land area or facility for the temporary parking or storage of motor vehicles used personally, for business, or for commerce.
- (iv) **Streets, roads, highways, freeways, and driveways.** This category is defined as any paved impervious surface used for the transportation of automobiles, trucks, motorcycles, and other vehicles.
- (d) New development or redevelopment project that **creates and/or replaces 2,500 square feet** or more of impervious surface (collectively over the entire project site), discharging directly to an Environmentally Sensitive Area (ESA). "Discharging directly to" includes flow that is conveyed overland a distance of 200 feet or less from the project to the ESA, or conveyed in a pipe or open channel any distance as an isolated flow from the project to the ESA (i.e. not commingled with flows from adjacent lands). **Yes** **No**
- (e) New development or redevelopment project that creates and/or replaces a combined total of 5,000 square feet or more of impervious surface, that support one or more of the following used: **Yes** **No**
- (i) **Automotive repair shops.** This category is defined as a facility that is categorized in any one of the following Standard Industrial Classification (SIC) codes: 5013, 5014, 5541, 7532-7534, or 7536-7539.
- (ii) **Retail gasoline outlets.** This category includes retail gasoline outlets that meet the meet one of the following criteria: (a) 5,000 square feet or more or (b) a projected Average Daily Traffic (ADT) of 100 or more vehicles per day.
- (f) New development or redevelopment that result in the disturbance of **one or more acres** of land and are expected to generate pollutants post construction. **NOTE:** Pollutant generating development projects are those projects that generate pollutants at levels greater than background levels. Background pollutant levels means the pollutants generated from an undeveloped site. Projects disturbing one or more acres of land are presumed to generate pollutants post construction unless the applicant presents a design that satisfies the City Engineer that pollutants in storm water discharges will not exceed preconstruction background levels. **Yes** **No**

The project is (select one):

- If "No" is checked for every category in Section 2, Project is a "Standard Development Project." Site Design and Source Control BMP requirements apply. **Complete and submit Standard SWQMP** (refer to Chapter 4 and Appendix E of the BMP Design Manual for guidance). **Skip to Section 5.**
- If "Yes" is checked for ANY category in Section 2, Project is a "Priority Development Project (PDP)." **Complete next part, if applicable, and continue to Section 3.**

Complete for PDP Redevelopment Projects ONLY:

The total existing (pre-project) impervious area at the project site is: _____ ft² (A)

The total proposed newly created or replaced impervious area is _____ ft² (B)

Percent impervious surface created or replaced (B/A) x 100 = _____%

The percent impervious surface created or replaced is (select one based on the above calculation):

- Less than or equal to fifty percent (50%) – **only new impervious areas are considered a PDP**
OR
 Greater than fifty percent (50%) – **the entire project site is considered a PDP**

Continue to Section 3**SECTION 3: Determine if Project is PDP Exempt**

1) Does the project ONLY include new or retrofit sidewalk, bicycle lane or trails that:

- Are designed and constructed to direct storm water runoff to adjacent vegetated areas, or other non-erodible permeable areas? Or;
- Are designed and constructed to be hydraulically disconnected from paved streets or roads? Or;
- Are designed and constructed with permeable pavements or surfaces in accordance with USEPA Green Streets guidance?

- Yes. Project is PDP Exempt.** **No. Next question**

Complete and submit **Standard SWQMP** (refer to Chapter 4 of the BMP Design Manual for guidance). **Continue to Section 5.**

2) Does the project ONLY include retrofitting or redevelopment of existing paved alleys, streets or roads designed and constructed in accordance with Green Streets standards?

- Yes. Project is PDP Exempt.** **No. Project is a PDP.**

Complete and submit Standard SWQMP (refer to Chapter 4 of the BMP Design Manual for guidance). **Continue to Section 5.**

Continue to Section 4.

SECTION 4: Alternative Compliance

Does the project elect to meet stormwater obligations onsite?

- Yes. Onsite Compliance.** **No. Please contact City Storm Water Management Section at stormwater@chulavistaca.gov**

Site design, source control, and structural pollutant control BMPs apply. Complete and submit **PDP SWQMP** (refer to Chapters 4, 5 & 6 of the BMP Design Manual for guidance). **Continue to Section 5.**

On-site site design, source control, and flow-through pollutant control BMPs apply. Complete and submit **PDP SWQMP** (refer to Chapters 4, 5 & 6 of the BMP Design Manual for guidance) and **Alternative Compliance forms** for pollutant control obligations (refer to the Alternative Compliance Program Guidelines and Development in Appendix J of the BMP Design Manual). **Continue to Section 5.**

SECTION 5: Construction Storm Water BMP Requirements:

All construction sites are required to implement construction BMPs in accordance with the performance standards in the BMP Design Manual. Some sites are additionally required to obtain coverage under the State Construction General Permit (CGP), which is administered by the State Water Resource Control Board.

1) Does the project include Building/Grading/Construction permits proposing less than 5,000 square feet of ground disturbance and has less than 5-foot elevation change over the entire project area?

- Yes. Review and sign Construction Storm Water Certification Statement on Page 2, *skip questions 2-4* No; next question

2) Does the project propose construction or demolition activity, including but not limited to, clearing grading, grubbing, excavation, or other activity that results in ground disturbance of less than one acre and more than 5,000 square feet?

- Yes. Complete and submit Construction Storm Water Pollution Control Plan (CSWPCP), *skip questions 3-4* No; next question

3) Does the project result in the disturbance of an acre or more of total land area and is considered a regular maintenance project performed to maintain original line and grade, hydraulic capacity, or original purpose of the facility? (Projects such as sewer/storm drain/utility replacement)

- Yes. Complete and submit Construction Storm Water Pollution Control Plan (CSWPCP), *skip question 4* No; next question

4) Is the project proposing land disturbance greater than or equal to one acre OR the project is part of a larger common plan of development disturbing 1 acre or more?

- Yes. Storm Water Pollution Prevention Plan (SWPPP) is required. Refer to online CASQA or Caltrans Template. Visit the SWRCB web site at:
https://www.waterboards.ca.gov/water_issues/programs/stormwater/construction.html

Note: Projects that result in disturbance of one to five acres of total land area and can demonstrate that there will be no adverse water quality impacts by applying for a Construction Rainfall Erosivity Waiver, may be allowed to submit a CSWPCP in lieu of a SWPPP.

Project Name/_____

HMP Exemption Exhibit

Attach this Exhibit (if Applicable) that shows direct storm water runoff discharge from the project site to HMP exempt area. Include project area, applicable underground storm drains line and/or concrete lined channels, outfall information and exempt waterbody. Reference applicable drawing number(s). **Exhibit must be provided on 11"x17" or larger paper.**

NOT APPLICABLE

Project Name/ _____

Insert Completed Form I-3B: Site Information Checklist for PDPs

<https://www.chulavistaca.gov/departments/public-works/services/storm-water-pollution-prevention/documents-and-reports>

Otay Ranch Village 7, R-3, R-4, & R-8

Project Name: _____

Site Information Checklist		Form I-3B
Project Summary Information		
Project Name	Otay Ranch Village 7, R-3, R-4 & R-8 North	
Project Address	and South from intersection of La Media Road and Santa Luna Street	
Assessor's Parcel Number(s) (APN(s))	644-241-07, 644-241-08 & 644-241-10	
Permit Application Number	TM 23-0001	
Project Watershed	<input checked="" type="checkbox"/> San Diego Bay	
Hydrologic Subarea name with Numeric Identifier up to two decimal places	Select One: <input type="checkbox"/> Pueblo San Diego 908 <input type="checkbox"/> Sweetwater 909 <input checked="" type="checkbox"/> Otay 910	
Project Area (total area of Assessor's Parcel(s) associated with the project or total area of the right-of-way)	<u>18.80</u> Acres (<u>818,811</u> Square Feet) This area includes all three lots (R-3, R-4, and R-8)	
Area to be Disturbed by the Project (Project Footprint)	<u>17.05</u> Acres (<u>745,217</u> Square Feet)	
Project Proposed Impervious Area (subset of Project Footprint)	<u>8.61</u> Acres (<u>374,952</u> Square Feet)	
Project Proposed Pervious Area (subset of Project Footprint)	<u>8.44</u> Acres (<u>370,265</u> Square Feet)	
Note: Proposed Impervious Area + Proposed Pervious Area = Area to be Disturbed by the Project. This may be less than the Parcel Area.		
The proposed increase or decrease in impervious area in the proposed condition as compared to the pre-project condition	<u>49</u> %	

**Please note that the no impervious area proposed per this permit . R-8 is to be mass graded with R-4 currently mass graded. Water quality, hydromodification, and minimum retention requirements for R-8 and R-4 do not have to be met within this SWQMP. However, the mass graded pads will be assumed to have 78% imperviousness so structural BMPs can be sized. Additional SWQMPs will be submitted to show compliance with this assumption and to cover minimum retention requirements. Imperviousness per this SWQMP: 7,432 SFT.
 Assumed imperviousness for R-8 and R-4: 367,520 SFT**



Project Name: Otay Ranch Village 7, R-3, R-4, & R-8

Form I-3B Page 3 of 10

Description of Existing Site Condition and Drainage Patterns

Current Status of the Site (select all that apply):

- Existing development
- Previously graded but not built out
- Demolition completed without new construction
- Agricultural or other non-impervious use
- Vacant, undeveloped/natural

Description / Additional Information:

Lot R-8 is a vacant space with an asphalt access road through the site while lot R-4 is mass graded with a desilt basin on-site. R-3 is a slope with a portion of a mass graded pad.

Existing Land Cover Includes (select all that apply):

- Vegetative Cover
- Non-Vegetated Pervious Areas
- Impervious Areas

Description / Additional Information:

There's an existing asphalt access road. Almost 50% of the proposed area to be disturbed presents a vegetative cover, the other half is a non-vegetative pervious area

Underlying Soil belongs to Hydrologic Soil Group (select all that apply):

- NRCS Type A
- NRCS Type B
- NRCS Type C
- NRCS Type D

Approximate Depth to Groundwater (GW):

- GW Depth < 5 feet
- 5 feet < GW Depth < 10 feet
- 10 feet < GW Depth < 20 feet
- GW Depth > 20 feet

Existing Natural Hydrologic Features (select all that apply):

- Watercourses
- Seeps
- Springs
- Wetlands
- None

Description / Additional Information:



Project Name: _____

Form I-3B Page 3 of 10

Description of Existing Site Drainage Patterns

How is storm water runoff conveyed from the site? At a minimum, this description should answer:

1. whether existing drainage conveyance is natural or urban;
2. Is runoff from offsite conveyed through the site? if yes, quantify all offsite drainage areas, design flows, and locations where offsite flows enter the project site, and summarize how such flows are conveyed through the site;
3. Provide details regarding existing project site drainage conveyance network, including any existing storm drains, concrete channels, swales, detention facilities, storm water treatment facilities, natural or constructed channels; and
4. Identify all discharge locations from the existing project site along with a summary of conveyance system size and capacity for each of the discharge locations. Provide summary of the pre-project drainage areas and design flows to each of the existing runoff discharge locations.

Describe existing site drainage patterns:

Lot R-8 is a 12.60-acre site that is currently vacant and composed of slopes, brow ditches, and an asphalt road that crosses the site and provides access to Vortac site from La Media Road.

In existing conditions, runoff from R-8 site sheet-flows westerly and northwesterly towards La Media Road where is intercepted by brow ditches that slopes northwesterly to direct the flows to catch basins and storm drain system that discharges into the existing detention basin at node 850, immediately north of the project site.

The existing graded slopes, brow ditches, road and storm drain were constructed per the Rough Grading Plans for Otay Ranch Village 7 (DWG 05017), prepared by Hunsaker and Associates, San Diego Inc. and dated 09-29-2005. The 50-year and 100-year storm events for the existing detention basin and storm drain were calculated in the Rough Grading Hydrology Study for Otay Ranch Village 7, prepared by Hunsaker and Associates, San Diego Inc. and dated 07-19-2005. The mentioned study and plans were used as the existing conditions for R-8 in this report.

Lot R-4 is approximately 3.11 acres that has been mass graded as part of the Otay Ranch Village 8 West Mass Grading Plan DWG. 14011-07 prepared by hale Engineering and dated 11/13/2023. Runoff from the site sheet flows southwesterly to be captured and routed via brow ditches and swales to the existing sediment basin at the southwest corner of the lot, where sediment control is provided. The discharge from the sediment basin is routed through a 30" CMP riser to the existing 18" storm drain pipe before tying into the exiting 60" pipe along La Media Road that ultimately discharges into the existing Village 8 West Wolf Canyon basin.

Lot R-3 is approximately 3.08 acres and consists of a slope and a portion of a mass-graded pad, which was graded as part of the Rough Grading Plans for Otay Ranch Village 7, DWG 05017. The site includes a vegetated slope adjacent to Magdalena Avenue, which slopes away from the street. The slope's runoff is directed to the pad either through brow ditches or surface flow. The flows from both the slope and the pad drain westerly, where they are captured and routed by the existing brow ditches. These flows then converge with runoff from the Vortac site, eventually discharging into the existing headwall at node 893.

Project Name: _____

Form I-3B Page 4 of 10

Description of Proposed Site Development and Drainage Patterns

Project Description / Proposed Land Use and/or Activities:

The project will be developed to be multi-family housing for both lots R-8 and R-4. The sites will include recreational areas, open space, roads, sidewalks, and underground utilities. Lot R-3 will remain in its current condition and no grading or improvement will occur under this permit. Otay Ranch Village 7 Neighborhood R-3 has been added to Tentative Map CVT 23-0001 in order to transfer residential units to this neighborhood for a future development. Any future development will need to process entitlements for a site plan and preliminary grading plan. The future site plan and preliminary grading plan will include a drainage report and SWQMP to comply with City requirements. Future development will not be required to process a tentative map.

List/describe proposed impervious features of the project (e.g., buildings, roadways, parking lots, courtyards, athletic courts, other impervious features):

Imperiousness features include roofs from the attached buildings, sidewalks, and roads.

List/describe proposed pervious features of the project (e.g., landscape areas):

Landscape areas and slopes.

Does the project include grading and changes to site topography?

- Yes
- No

Description / Additional Information:

The project will create a triangular 7.97-acre pad draining southerly towards the intersection of La Media Road and Santa Luna Street for lot R-8.
There will not be grading activities for lot R-4 under this permit.
An access road will be provided for the Vortac site off Magdalena Avenue; grading will include ditches around the road, the road itself, slopes, and a biofiltration basin.

Otay Ranch Village 7, R-3, R-4, & R-8

Project Name: _____

Form I-3B Page 5 of 10

Does the project include changes to site drainage (e.g., installation of new storm water conveyance systems)?

- Yes
 No

If yes, provide details regarding the proposed project site drainage conveyance network, including storm drains, concrete channels, swales, detention facilities, storm water treatment facilities, natural or constructed channels, and the method for conveying offsite flows through or around the proposed project site. Identify all discharge locations from the proposed project site along with a summary of the conveyance system size and capacity for each of the discharge locations. Provide a summary of pre- and post-project drainage areas and design flows to each of the runoff discharge locations. Reference the drainage study for detailed calculations.

Describe proposed site drainage patterns:

The Otay Ranch Village 7 R-8 site will be mass graded with 1.5% slope developed with 21 multi-family buildings, recreational area, landscaped area, associated parking areas, sidewalks and roads. This development will involve the removal of the road providing access to the Vortac Site off La Media Road. A new access road is being proposed as part of this project east of R-8 and connecting to Magdalena Avenue. This area is tributary to the basin within Village 8 West but it's imperviousness was not accounted for. A flow-based proprietary biofiltration unit will be proposed adjacent to the road to meet water quality requirements. The road will be super elevated to the south and will include cuts to allow runoff generated from the road to disperse in the landscape adjacent to the road. The use of dispersion areas will help meet minimum retention for this segment of the project.

Runoff from developed conditions of the project site will be captured via inlets, catch basins, area drains and will be routed by a proposed storm drain to the proprietary biofiltration BMPs located immediately north of the project site's entrance to address water quality requirements and routed again to the existing detention basin located immediately north of the project site to address hydromodification and detention requirements.

Lot R-4 will also consist of multi-family buildings with associated open space areas, roads, and sidewalks. The run-off from the project will be conveyed on the streets to a low-point at the site's entrance. Storm water generated on-site will leave in a 18" pipe and tie into the existing 60" pipe that runs on La Media Road. The drainage report for Village 8 West assumes a runoff coefficient of 0.78 which is consistent with the assumed imperviousness in this report. Therefore no increase in runoff is expected from this site. However, the approved SWQMP assumes the lot to be open space and pervious. Due to the increase in imperviousness, a MWS unit will be proposed downstream of the inlets to provide water quality measures associated with the development. The SWMM model prepared to address the imperviousness of the access road mentioned above will also include this lot as part of the analysis.

Lot R-3 will remain in its current condition and no grading or improvement will occur under this permit.

Table 1 - Summary of R-4 Existing Peak Flows

Discharge Location	Existing Conditions			
	Area (acres)	Runoff Coeff. "C"	Q50 (cfs)	Q100 (cfs)
Node 164	92.5	0.71	217.12	269.46

Table 2 - Summary of Peak Flows

Discharge Location	Designed Conditions ⁽¹⁾				Proposed Conditions ⁽²⁾				Difference		
	Area (acres)	Runoff Coeff. "C"	Q50 (cfs)	Q100 (cfs)	Area (acres)	Runoff Coeff. "C"	Q50 (cfs)	Q100 (cfs)	Area (acres)	Q50 (cfs)	Q100 (cfs)
Inflow to Basin	152.7	0.678	331.03	378.37	152.7	0.682	310.50	355.27	0	-20.53	-23.10
Outflow from Basin	152.7	0.678	151.20⁽³⁾	151.20	152.7	0.682	119.00⁽⁴⁾	146.55⁽⁴⁾	0	-32.20	-4.65
Node 893	28.9	0.60	29.15	33.74	28.9	0.60	28.76⁽⁵⁾	33.30⁽⁵⁾	0	-0.39	-0.44
Node 164	93.9	0.72	221.91	274.89	92.5	0.72	221.24	274.14	-1.4⁽⁶⁾	-0.67	-0.75

- (1) For Node 850, 851, and 893 values are from Rough Grading Hydrology Study for Otay Ranch Village 7 prepared by H&A and dated July 19, 2005. For Node 164 values from the Drainage Study for Otay Ranch Village 8 West, prepared by Hale Engineering, and dated October 25, 2019. The peak flows under existing conditions are the design flow rates presented in the approved studies for Village 7 and Village 8 West, which were used to design the downstream infrastructure.
- (2) Assuming ultimate conditions for R-8 and R-4
- (3) Rough Grading Hydrology Study for Otay Ranch Village 7 does not provide detention performance for 100-yr event. It uses the same peak flow as the 50-yr result. It is expected that the flows for the 100-yr are higher.
- (4) The unmitigated flows in proposed conditions are lower than the existing unmitigated flows, so mitigation is not required. However, the riser in the existing basin is being modified per PDP SWQMP for this site to meet hydromodification requirements. Due to the proposed modification, the existing riser alters the detention capabilities of the basin. Chapter 4 analyzes this change.
- (5) No mitigation is proposed for these areas as the generated runoff decreases slightly due to the increased time of concentration.
- (6) The 1.4-acre difference in areas between the existing and proposed delineations originates from the preliminary area delineation in the master drainage study Drainage Study for Otay Ranch Village 8 West, prepared by Hale Engineering, and dated October 25, 2019. Initially, the slope south of R-4 drained towards the existing browditch and discharged into the sediment basin on R-4, making it part of the studied area for R-4. This slope has since been graded and incorporated into Parcel D. Consequently, the flows from this area were included and addressed in the Drainage Report for Parcel D, Permit No. GR23-0002, DWG No. 23021 Therefore, this minor area difference does not increase the calculated peak flow or negatively impact the downstream storm drain, as the routed flow to Parcel D and capacity of the existing storm drain were evaluated in the Drainage Report for Parcel D.

Project Name: _____

Form I-3B Page 6 of 10

Identify whether any of the following features, activities, and/or pollutant source areas will be present (select all that apply):

- On-site storm drain inlets
- Interior floor drains and elevator shaft sump pumps
- Interior parking garages
- Need for future indoor & structural pest control
- Landscape/Outdoor Pesticide Use
- Pools, spas, ponds, decorative fountains, and other water features
- Food service
- Refuse areas
- Industrial processes
- Outdoor storage of equipment or materials
- Vehicle and Equipment Cleaning
- Vehicle/Equipment Repair and Maintenance
- Fuel Dispensing Areas
- Loading Docks
- Fire Sprinkler Test Water
- Miscellaneous Drain or Wash Water
- Plazas, sidewalks, and parking lots

Description / Additional Information:

The development will consist of multi-family residential units. The BMPs above reflect the proposed source control BMPs which are typically applicable to this type of development. The site will include inlet stenciling for public awareness of pollution concerns related to street pollutants. The use of pesticides for landscape use will be discouraged and designated refuse areas (where applicable) will be protected from stormwater.

Otay Ranch Village 7, R-3, R-4, & R-8

Project Name: _____

Form I-3B Page 7 of 10

Identification and Narrative of Receiving Water and Pollutants of Concern

Describe flow path of storm water from the project site discharge location(s), through urban storm conveyance systems as applicable, to receiving creeks, rivers, and lagoons as applicable, and ultimate discharge to the Pacific Ocean (or bay, lagoon, lake or reservoir, as applicable):

The runoff from the project has two POCs that discharge into the wolf canyon at different location and ultimately flow into the San Diego Bay. POC 1 is an existing detention basin that outlets into the Wolf canyon through a riser structure and underground storm drain. POC 2 is an existing clean out on La Media Road where the flows of Village 7 and R-4 commingle before entering the Village 8 West development. The storm water continues downstream in an existing 60" RCP pipe and eventually discharge into an existing biofiltration/hydromodification basin in Village 8 West. Similarly, the flows go through a riser and into underground storm drain that discharges into the Wolf Canyon.

List any 303(d) impaired water bodies within the path of storm water from the project site to the Pacific Ocean (or bay, lagoon, lake or reservoir, as applicable), identify the pollutant(s)/stressor(s) causing impairment, and identify any TMDLs and/or Highest Priority Pollutants from the WQIP for the impaired water bodies:

303(d) Impaired Water Body	Pollutant(s)/Stressor(s)	TMDLs / WQIP Highest Priority Pollutant
San Diego Bay	PCBs, Mercury, and PAHs	Bacteria

Identification of Project Site Pollutants*

*Identify all pollutants that are expected to be discharged from the project site. s are implemented in an alternative design. is demonstrated. Identify all pollutants that are expected to be discharged from the project site. Design Manual Appendix 2.07. s are participate ments BMP

Not applicable since the MWS units are designed as proprietary biofiltration

Pollutant	Not Applicable to the Project Site	Expected from the Project Site	Also a Receiving Water Pollutant of Concern
Sediment	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
Nutrients	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
Heavy Metals	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
Organic Compounds	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
Trash & Debris	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
Oxygen Demanding Substances	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
Oil & Grease	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
Bacteria & Viruses	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
Pesticides	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>



Form I-3B Page 8 of 10

Hydromodification Management Requirements

Do hydromodification management requirements apply (see Section 1.6)?

- Yes, hydromodification management flow control structural BMPs required.
- No, the project will discharge runoff directly to existing underground storm drains discharging directly to water storage reservoirs, lakes, enclosed embayments, or the Pacific Ocean.
- No, the project will discharge runoff directly to conveyance channels whose bed and bank are concrete-lined all the way from the point of discharge to water storage reservoirs, lakes, enclosed embayments, or the Pacific Ocean.
- No, the project will discharge runoff directly to an area identified as appropriate for an exemption by the WMAA for the watershed in which the project resides.

Description / Additional Information (to be provided if a 'No' answer has been selected above):

Note: If “No” answer has been selected the SWQMP must include an exhibit that shows the storm water conveyance system from the project site to an exempt water body. The exhibit should include details about the conveyance system and the outfall to the exempt water body.

Critical Coarse Sediment Yield Areas*

***This Section only required if hydromodification management requirements apply**

Based on Section 6.2 and Appendix H does CCSYA exist on the project footprint or in the upstream area draining through the project footprint?

- Yes
- No

Description / Additional Information:

Project Name: _____

Form I-3B Page 9 of 10

Flow Control for Post-Project Runoff*

***This Section only required if hydromodification management requirements apply**

List and describe point(s) of compliance (POCs) for flow control for hydromodification management (see Section 6.3.1). For each POC, provide a POC identification name or number correlating to the project's HMP Exhibit and a receiving channel identification name or number correlating to the project HMP Exhibit.

Point of compliance 1 (POC 1) is located northwest of R-8 and downstream of the existing Village 7 detention basin. Lot R-8 discharges into the existing detention basin and converges with other flows from the northern residential areas as well as runoff from La Media Road. An existing riser structure controls the peak flows and helps mitigate for the Village 7 existing development.

The riser will be modified so that the impact of R-8 can also be mitigated for hydromodification and detention.

Point of compliance 2 (POC 2) is being analyzed at the cleanout where the flows from Village 7 commingle with the runoff from lot R-4. This storm drain cleanout is tributary to a portion of the Olympian High school, as well as public roads, the Vortac site, lot R-3, and the access road proposed at Magdalena Avenue. Please see page 7 for additional information.

Has a geomorphic assessment been performed for the receiving channel(s)?

- No, the low flow threshold is 0.1Q2 (default low flow threshold)
- Yes, the result is the low flow threshold is 0.1Q2
- Yes, the result is the low flow threshold is 0.3Q2
- Yes, the result is the low flow threshold is 0.5Q2

If a geomorphic assessment has been performed, provide title, date, and preparer:

Discussion / Additional Information: (optional)

Project Name: _____

Form I-3B Page 10 of 10

Other Site Requirements and Constraints

When applicable, list other site requirements or constraints that will influence storm water management design, such as zoning requirements including setbacks and open space, or local codes governing minimum street width, sidewalk construction, allowable pavement types, and drainage requirements.

Optional Additional Information or Continuation of Previous Sections As Needed

This space provided for additional information or continuation of information from previous sections as needed.

Project Name/ _____

**Insert Completed Form I-4: Source Control BMP Checklist for All
Development Projects**

<https://www.chulavistaca.gov/departments/public-works/services/storm-water-pollution-prevention/documents-and-reports>

Project Name: _____

Source Control BMP Checklist for All Development Projects		Form I-4	
<p>All development projects must implement source control BMPs. Refer to Chapter 4 and Appendix E of the BMP Design Manual for information to implement BMPs shown in this checklist.</p> <p>Note: All selected BMPs must be shown on the site/construction plans.</p> <p>Answer each category below pursuant to the following:</p> <ul style="list-style-type: none"> • "Yes" means the project will implement the source control BMP as described in Chapter 4 and/or Appendix E of the BMP Design Manual. Discussion / justification is not required. • "No" means the BMP is applicable to the project but it is not feasible to implement. Discussion / justification must be provided. • "N/A" means the BMP is not applicable at the project site because the project does not include the feature that is addressed by the BMP (e.g., the project has no outdoor materials storage areas). Discussion / justification may be provided. 			
Source Control Requirement	Applied?		
4.2.1 Prevention of Illicit Discharges into the MS4	<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> N/A
Discussion / justification if 4.2.1 not implemented:			
4.2.2 Storm Drain Stenciling or Signage	<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> N/A
Discussion / justification if 4.2.2 not implemented:			
4.2.3 Protect Outdoor Materials Storage Areas from Rainfall, Run-On, Runoff, and Wind Dispersal	<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input checked="" type="checkbox"/> N/A
Discussion / justification if 4.2.3 not implemented:			
4.2.4 Protect Materials Stored in Outdoor Work Areas from Rainfall, Run-On, Runoff, and Wind Dispersal	<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input checked="" type="checkbox"/> N/A
Discussion / justification if 4.2.4 not implemented:			
4.2.5 Protect Trash Storage Areas from Rainfall, Run-On, Runoff, and Wind Dispersal	<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input checked="" type="checkbox"/> N/A
Discussion / justification if 4.2.5 not implemented:			

Otay Ranch Village 7, R-3, R-4, & R-8

Project Name: _____

Source Control BMP Checklist for All Development Projects		Form I-4 (Page 2 of 2)	
4.2.6 Additional BMPs Based on Potential Sources of Runoff Pollutants (must answer for each source listed below)	<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> N/A
SC-A Onsite storm drain inlets	<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> N/A
SC-B Interior floor drains and elevator shaft sump pumps	<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input checked="" type="checkbox"/> N/A
SC-C Interior parking garages	<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input checked="" type="checkbox"/> N/A
SC-D1 Need for future indoor & structural pest control	<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> N/A
SD-D2 Landscape/outdoor pesticide use	<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> N/A
SC-E Pools, spas, ponds, decorative fountains, and other water features	<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> N/A
SC-F Food Service	<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input checked="" type="checkbox"/> N/A
SC-G Refuse areas	<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input checked="" type="checkbox"/> N/A
SC-H Industrial processes	<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input checked="" type="checkbox"/> N/A
SC-I Outdoor storage of equipment or materials	<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input checked="" type="checkbox"/> N/A
SC-J Vehicle and equipment cleaning	<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input checked="" type="checkbox"/> N/A
SC-K Vehicle/equipment repair and maintenance	<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input checked="" type="checkbox"/> N/A
SC-L Fuel dispensing areas	<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input checked="" type="checkbox"/> N/A
SC-M Loading docks	<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input checked="" type="checkbox"/> N/A
SC-N Fire sprinkler test water	<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> N/A
SC-O Miscellaneous drain or wash water	<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> N/A
SC-P Plazas, sidewalks, and parking lots	<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> N/A
SC-Q: Large Trash Generating Facilities	<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input checked="" type="checkbox"/> N/A
SC-R: Animal Facilities	<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input checked="" type="checkbox"/> N/A
SC-S: Plant Nurseries and Garden Centers	<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input checked="" type="checkbox"/> N/A
SC-T: Automotive Facilities	<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input checked="" type="checkbox"/> N/A
Discussion / justification if 4.2.6 not implemented. Justification must be provided for all "No" answers shown above.			

Project Name/_____

**Insert Completed Form I-5: Site Design BMP Checklist for All
Development Projects**

<https://www.chulavistaca.gov/departments/public-works/services/storm-water-pollution-prevention/documents-and-reports>

Project Name.: _____

Site Design BMP Checklist for All Development Projects		Form I-5	
<p>All development projects must implement site design BMPs where applicable and feasible. See Chapter 4 and Appendix E of the manual for information to implement site design BMPs shown in this checklist. Note: All selected BMPs must be shown on the site/construction plans.</p> <p>Answer each category below pursuant to the following.</p> <ul style="list-style-type: none"> • "Yes" means the project will implement the site design BMP as described in Chapter 4 and/or Appendix E of the manual. Discussion / justification is not required. • "No" means the BMP is applicable to the project but it is not feasible to implement. Discussion / justification must be provided. • "N/A" means the BMP is not applicable at the project site because the project does not include the feature that is addressed by the BMP (e.g., the project site has no existing natural areas to conserve). Discussion / justification may be provided. 			
Site Design Requirement	Applied?		
4.3.1 Maintain Natural Drainage Pathways and Hydrologic Features	<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> N/A
4.3.2 Conserve Natural Areas, Soils, and Vegetation	<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> N/A
4.3.3 Minimize Impervious Area	<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> N/A
4.3.4 Minimize Soil Compaction	<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> N/A
4.3.5 Impervious Area Dispersion	<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input checked="" type="checkbox"/> N/A
<p>There are no impervious surfaces proposed under this permit. The access road will be decomposed granite or equivalent semi-pervious surface and will be considered a self retaining DMA. Impervious area dispersion will be incorporated into the site design by proposing roof downspouts that drain into adjacent landscaped areas. The typical locations of these downspouts will be shown on the DMA Exhibit.</p>			

Project Name/Address/N _____

Site Design BMP Checklist for All Development Projects		Form I-5	
Site Design Requirement	Applied?		
4.3.6 Runoff Collection	<input type="checkbox"/> Yes	<input checked="" type="checkbox"/> No	<input type="checkbox"/> N/A
Treatment of onsite stormwater will be treated via the proposed proprietary biofiltration MWS units.			
4.3.7 Landscaping with Native or Drought Tolerant Species	<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> N/A
4.3.8 Harvesting and Using Precipitation	<input type="checkbox"/> Yes	<input checked="" type="checkbox"/> No	<input type="checkbox"/> N/A
Discussion / justification for all "No" answers shown above:			
This site design is not feasible for this project.			

Project Name/ _____

Insert Completed Form I-6: Summary of PDP Structural BMPs

<https://www.chulavistaca.gov/departments/public-works/services/storm-water-pollution-prevention/documents-and-reports>

Project Name: _____

Summary of PDP Structural BMPs	Form I-6
PDP Structural BMPs	
<p>All PDPs must implement structural BMPs for storm water pollutant control (see Chapter 5 of the manual). Selection of PDP structural BMPs for storm water pollutant control must be based on the selection process described in Chapter 5. PDPs subject to hydromodification management requirements must also implement structural BMPs for flow control for hydromodification management (see Chapter 6 of the manual). Both storm water pollutant control and flow control for hydromodification management can be achieved within the same structural BMP(s).</p> <p>PDP structural BMPs must be verified by City at the completion of construction. This may include requiring the project owner or project owner's representative to certify construction of the structural BMPs (see Section 1.12 of the manual). PDP structural BMPs must be maintained into perpetuity (see Section 7 of the manual).</p> <p>Use this form to provide narrative description of the general strategy for structural BMP implementation at the project site in the box below. Then complete the PDP structural BMP summary information sheet (page 3 of this form) for each structural BMP within the project (copy the BMP summary information page as many times as needed to provide summary information for each individual structural BMP).</p>	
<p>Describe the general strategy for structural BMP implementation at the site. This information must describe how the steps for selecting and designing storm water pollutant control BMPs presented in Section 5.1 of the manual were followed, and the results (type of BMPs selected). For projects requiring hydromodification flow control BMPs, indicate whether pollutant control and flow control BMPs are integrated or separate.</p> <p>This site will include three Proprietary biofiltration MWS units at the downstream portion of the R-8, R-4, and the access road which will act to address pollution control. Two MWS units are sized and will be proposed as a flow-based units downstream R-8. R-4 will have a single MWS unit downstream of the site and of an underground storage unit. These BMPs are designed to address pollution and hydromodification control. Finally, there will be one flow-based Filterra located adjacent to the access road off Magdalena Avenue.</p> <p>In selection of the biofiltration BMP, the following steps were taken as presented in Section 5.1 of the BMP Design Manual.</p> <ol style="list-style-type: none"> 1. Delineate the DMAs and identify the self-mitigating (5.2.1) and De minimis areas (per 5.2.2). 2. Estimated DCV for each remaining area using worksheet B.2.1 . 3. Harvest considered not feasible per Harvest and use feasibility checklist Form I-7 . 4. Infiltration considered not feasible per infiltration feasibility checklist FormC-4-1 . 5. Computed sizing requirements. 6. Design BMP for DCV per design criteria and considerations listed in the fact sheets. 7. Provide minimum retention requirements using the MWS units capabilities as well as dispersion areas with amended soils. 	

Otay Ranch Village 7, R-3, R-4, & R-8

Project Name: _____

Form I-6 Page 2 of <u>7</u> (Copy and attach as many as needed)	
Structural BMP ID No. BF-3-1	
Construction Plan Sheet No. N/A	
Type of structural BMP: <input type="checkbox"/> Retention by harvest and use (e.g. HU-1, cistern) <input type="checkbox"/> Retention by infiltration basin (INF-1) <input type="checkbox"/> Retention by bioretention (INF-2) <input type="checkbox"/> Retention by permeable pavement (INF-3) <input type="checkbox"/> Partial retention by biofiltration with partial retention (PR-1) <input type="checkbox"/> Biofiltration (BF-1) <input type="checkbox"/> Flow-thru treatment control with prior lawful approval to meet earlier PDP requirements (provide BMP type/description in discussion section below) <input type="checkbox"/> Flow-thru treatment control included as pre-treatment/forebay for an onsite retention or biofiltration BMP (provide BMP type/description and indicate which onsite retention or biofiltration BMP it serves in discussion section below) <input type="checkbox"/> Flow-thru treatment control with alternative compliance (provide BMP type/description in discussion section below) <input type="checkbox"/> Detention pond or vault for hydromodification management <input checked="" type="checkbox"/> Other (describe in discussion section below) <div style="border: 1px solid red; padding: 2px; display: inline-block; margin-left: 20px;">Flow-based Proprietary Biofiltration</div>	
Purpose: <input checked="" type="checkbox"/> Pollutant control only <input type="checkbox"/> Hydromodification control only <input type="checkbox"/> Combined pollutant control and hydromodification control <input type="checkbox"/> Pre-treatment/forebay for another structural BMP <input type="checkbox"/> Other (describe in discussion section below)	
Who will certify construction of this BMP? Provide name and contact information for the party responsible to sign BMP verification forms if required by the City Engineer (See Section 1.12 of the manual)	Alisa S. Vialpando , PE # 47945 Hunsaker & Associates SD, Inc. 9707 Waples St, San Diego, CA 92121 (858) 558-4500
Who will be the final owner of this BMP?	Baldwin & Son LLC
Who will maintain this BMP into perpetuity?	Baldwin & Son LLC
What is the funding mechanism for maintenance?	Rent and fees to the Homeowners Association (HOA)



Project Name: _____

Form I-6 Page 3 of 7 *(Copy and attach as many as needed)*

Structural BMP ID No. BF-3-1

Construction Plan Sheet No. N/A

Discussion (as needed, must include worksheets showing BMP sizing calculations in the SWQMP):

- * 85th percentile 24-hr storm depth from iso map Figure B.1.1 is $d = 0.56$ in
- *The area drains to the biofiltration Proprietary MWS unit is delineated $A = 347,130$ sf
- * Total impervious area is 270,761 sf, and pervious area is 76,369 sf
- *The weighted area runoff factor is calculated as a composite coefficient made of the different runoff factor for the surfaces of the DMA area per equation
 $C = \{(0.9 * \text{Impervious surfaces}) + (0.1 * \text{pervious areas})\} / (\text{total area})$
 $C = \{(0.9 * 347,130) + (0.1 * 76,369)\} / (645,559) = 0.72$
- * Calculate DCV = $3630 \times C \times d \times A = 3630 \times 0.72 \times 0.53 \times (347,130 / 43560) = 11,100$ cft
 $1.5 \text{ DCV} = 16,650$ cft
- *Water Quality Flow Rate = $AF \times (C \times i \times A) \times 1.5$
 $Q = 1(0.72 \times 0.2 \times 7.97) \times 1.5 = 1.731$ cfs
- *2-8 x 24 MWS w/treatment capacity of 0.870 will be proposed.

Otay Ranch Village 7, R-3, R-4, & R-8

Project Name: _____

Form I-6 Page 4 of <u>7</u> (Copy and attach as many as needed)	
Structural BMP ID No. BF-3-2	
Construction Plan Sheet No. N/A	
Type of structural BMP: <input type="checkbox"/> Retention by harvest and use (e.g. HU-1, cistern) <input type="checkbox"/> Retention by infiltration basin (INF-1) <input type="checkbox"/> Retention by bioretention (INF-2) <input type="checkbox"/> Retention by permeable pavement (INF-3) <input type="checkbox"/> Partial retention by biofiltration with partial retention (PR-1) <input type="checkbox"/> Biofiltration (BF-1) <input type="checkbox"/> Flow-thru treatment control with prior lawful approval to meet earlier PDP requirements (provide BMP type/description in discussion section below) <input type="checkbox"/> Flow-thru treatment control included as pre-treatment/forebay for an onsite retention or biofiltration BMP (provide BMP type/description and indicate which onsite retention or biofiltration BMP it serves in discussion section below) <input type="checkbox"/> Flow-thru treatment control with alternative compliance (provide BMP type/description in discussion section below) <input type="checkbox"/> Detention pond or vault for hydromodification management <input checked="" type="checkbox"/> Other (describe in discussion section below) <div style="border: 1px solid red; padding: 2px; display: inline-block;">Flow-based Proprietary Biofiltration</div>	
Purpose: <input checked="" type="checkbox"/> Pollutant control only <input type="checkbox"/> Hydromodification control only <input type="checkbox"/> Combined pollutant control and hydromodification control <input type="checkbox"/> Pre-treatment/forebay for another structural BMP <input type="checkbox"/> Other (describe in discussion section below)	
Who will certify construction of this BMP? Provide name and contact information for the party responsible to sign BMP verification forms if required by the City Engineer (See Section 1.12 of the manual)	Alisa S. Vialpando , PE # 47945 Hunsaker & Associates SD, Inc. 9707 Waples St, San Diego, CA 92121 (858) 558-4500
Who will be the final owner of this BMP?	Otay Project, L.P.
Who will maintain this BMP into perpetuity?	Otay Project, L.P.
What is the funding mechanism for maintenance?	Rent and fees to the Homeowners Association (HOA) .



Project Name: _____

Form I-6 Page 5 of 7 *(Copy and attach as many as needed)*

Structural BMP ID No. BF-3-2

Construction Plan Sheet No. N/A

Discussion (as needed, must include worksheets showing BMP sizing calculations in the SWQMP):

- * 85th percentile 24-hr storm depth from iso map Figure B.1.1 is $d = 0.56$ in
- *The area drains to the biofiltration Proprietary MWS unit is delineated $A = 124,050$ sf
- * Total impervious area is 96,759 sf, and pervious area is 27,291 sf
- *The weighted area runoff factor is calculated as a composite coefficient made of the different runoff factor for the surfaces of the DMA area per equation
 $C = \{(0.9 * \text{Impervious surfaces}) + (0.1 * \text{pervious areas})\} / (\text{total area})$
 $C = \{(0.9 * 96,759) + (0.1 * 27,291)\} / (124,050) = 0.72$
- * Calculate DCV = $3630 \times C \times d \times A = 3630 \times 0.72 \times 0.53 \times (124,050 / 43560) = 3,967$ cft
 $1.5 \text{ DCV} = 5,950$ cft
- *Water Quality Flow Rate = $AF \times (C \times i \times A) \times 1.5$
 $Q = 1(0.72 \times 0.2 \times 2.85) \times 1.5 = 0.619$ cfs
- *1-8 x 24 MWS w/treatment capacity of 0.693 will be proposed.

Otay Ranch Village 7, R-3, R-4, & R-8

Project Name: _____

Form I-6 Page 6 of <u>7</u> (Copy and attach as many as needed)	
Structural BMP ID No. HMP-1	
Construction Plan Sheet No. N/A/	
Type of structural BMP: <input type="checkbox"/> Retention by harvest and use (e.g. HU-1, cistern) <input type="checkbox"/> Retention by infiltration basin (INF-1) <input type="checkbox"/> Retention by bioretention (INF-2) <input type="checkbox"/> Retention by permeable pavement (INF-3) <input type="checkbox"/> Partial retention by biofiltration with partial retention (PR-1) <input type="checkbox"/> Biofiltration (BF-1) <input type="checkbox"/> Flow-thru treatment control with prior lawful approval to meet earlier PDP requirements (provide BMP type/description in discussion section below) <input type="checkbox"/> Flow-thru treatment control included as pre-treatment/forebay for an onsite retention or biofiltration BMP (provide BMP type/description and indicate which onsite retention or biofiltration BMP it serves in discussion section below) <input type="checkbox"/> Flow-thru treatment control with alternative compliance (provide BMP type/description in discussion section below) <input checked="" type="checkbox"/> Detention pond or vault for hydromodification management <input type="checkbox"/> Other (describe in discussion section below)	
Purpose: <input type="checkbox"/> Pollutant control only <input checked="" type="checkbox"/> Hydromodification control only <input type="checkbox"/> Combined pollutant control and hydromodification control <input type="checkbox"/> Pre-treatment/forebay for another structural BMP <input type="checkbox"/> Other (describe in discussion section below)	
Who will certify construction of this BMP? Provide name and contact information for the party responsible to sign BMP verification forms if required by the City Engineer (See Section 1.12 of the manual)	Alisa S. Vialpando , PE # 47945 Hunsaker & Associates SD, Inc. 9707 Waples St, San Diego, CA 92121 (858) 558-4500
Who will be the final owner of this BMP?	Otay Project, L.P.
Who will maintain this BMP into perpetuity?	Otay Project, L.P.
What is the funding mechanism for maintenance?	Rent and fees to the Homeowners Association (HOA)



Project Name: _____

Form I-6 Page 7 of 7 *(Copy and attach as many as needed)*

Structural BMP ID No. HMP-1

Construction Plan Sheet No. N/A/

Discussion (as needed, must include worksheets showing BMP sizing calculations in the SWQMP):

Determine the required HMP volume using the BMP Sizing spread sheet V.3.1. SWMM was then used to properly sized an outflow structure, determine the height and the size of the unit. Please see HMP report on attachment 2.

Project Name/_____

ATTACHMENT 1

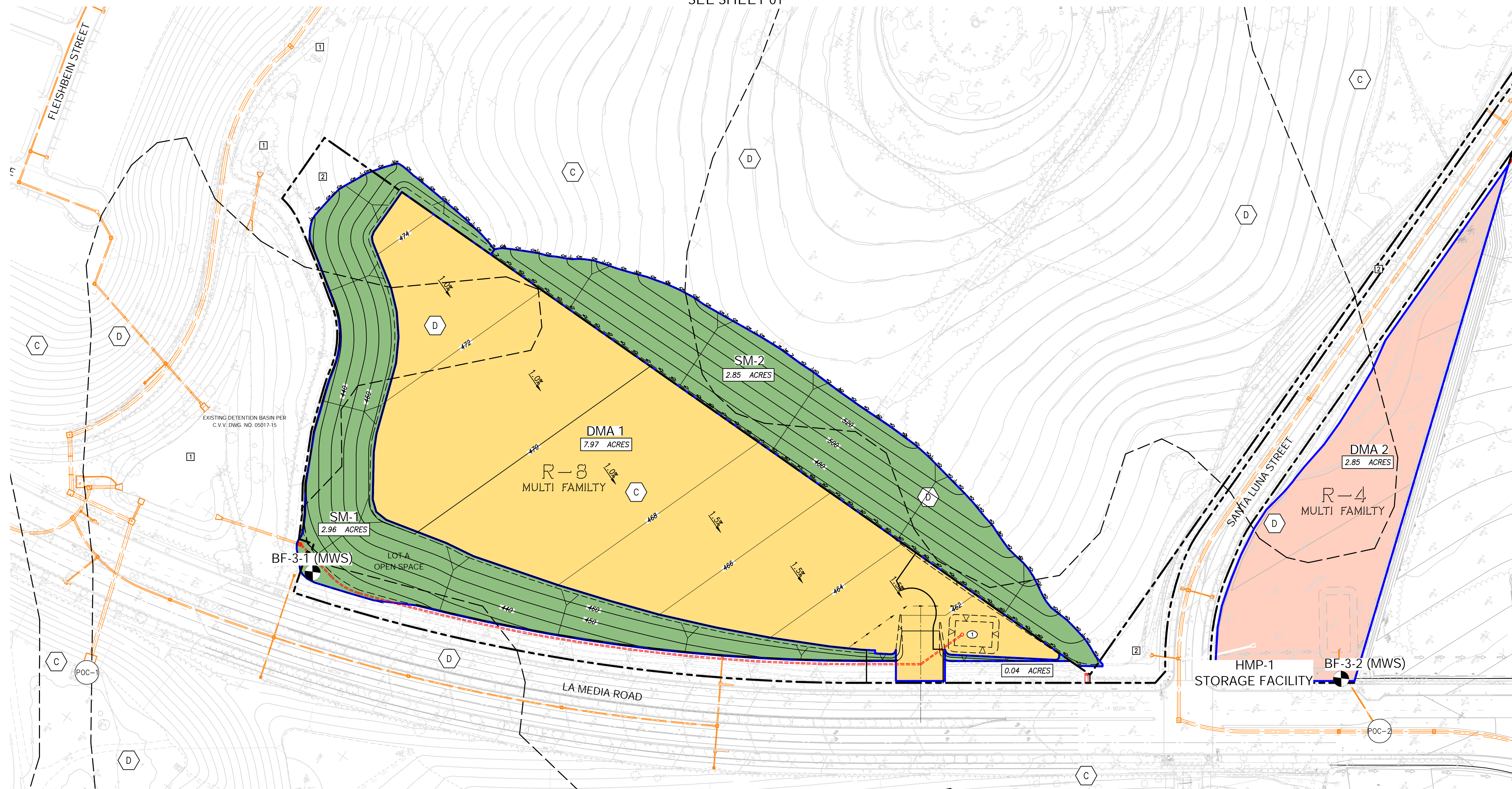
Backup for PDP Pollutant Control BMPs

Indicate which Items are Included:

Attachment Sequence	Contents	Checklist
Attachment 1A	DMA Exhibit (Required) See DMA Exhibit Checklist.	<input checked="" type="checkbox"/> Included
Attachment 1B	Tabular Summary of DMAs Showing DMA ID matching DMA Exhibit, DMA Area, and DMA Type (Required)* *Provide table in this Attachment OR on DMA Exhibit in Attachment 1a	<input type="checkbox"/> Included on DMA Exhibit in Attachment 1A <input checked="" type="checkbox"/> Included as Attachment 1B, separate from DMA Exhibit
Attachment 1C	Form I-7, Harvest and Use Feasibility Screening Checklist (Required unless the entire project will use infiltration BMPs) Refer to Appendix B.3-1 of the BMP Design Manual to complete Form I-7.	<input checked="" type="checkbox"/> Included <input type="checkbox"/> Not included because the entire project will use infiltration BMPs
Attachment 1D	Infiltration Feasibility Information. Contents of Attachment 1D depend on the infiltration condition: <input checked="" type="checkbox"/> No Infiltration Condition: <input checked="" type="checkbox"/> Infiltration Feasibility Condition <input type="checkbox"/> Letter (<i>Note: must be stamped & signed by licensed geotechnical engineer</i>) <input checked="" type="checkbox"/> Form I-8A (optional) <input type="checkbox"/> Form I-8B (optional) <input type="checkbox"/> Partial Infiltration Condition: <input type="checkbox"/> Infiltration Feasibility Condition <input type="checkbox"/> Letter (<i>Note: must be stamped & signed by licensed geotechnical engineer</i>) <input type="checkbox"/> Form I-8A <input type="checkbox"/> Form I-8B <input type="checkbox"/> Full Infiltration Condition: <input type="checkbox"/> Form I-8A <input type="checkbox"/> Form I-8B <input type="checkbox"/> Worksheet C.4-3 <input type="checkbox"/> Form I-9 Refer to Appendices C and D of the BMP Design Manual for guidance.	<input checked="" type="checkbox"/> Included <input type="checkbox"/> Not included because the entire project will use harvest and use BMPs
Attachment 1E	Pollutant Control BMP Design Worksheets/ Calculations (Required) Refer to Appendices B and E of the BMP Design Manual for structural pollutant control BMP design guidelines	<input checked="" type="checkbox"/> Included

ATTACHMENT 1a
DMA EXHIBIT

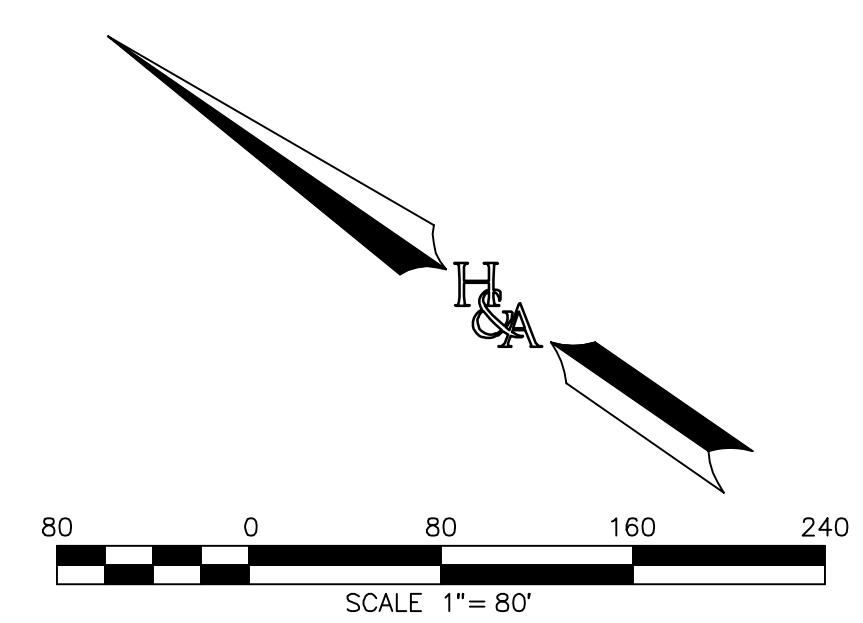
SEE SHEET 01



LEGEND:	SYMBOL:
PROJECT BOUNDARY.....	
DMA BOUNDARY.....	
DAYLIGHT.....	
PROPOSED STORM DRAIN.....	
EXISTING STORM DRAIN.....	
SUBAREA ACREAGE.....	
DMA ICON.....	
R-8 (78% IMPERVIOUSNESS).....	
R-4 (78% IMPERVIOUSNESS).....	
IMPERVIOUS - ROAD.....	
PERVIOUS - BIOFILTRATION BASIN.....	
PERVIOUS AREAS.....	
SELF MITIGATING.....	
INLET.....	
HYDROLOGIC SOIL TYPE.....	
POINT OF COMPLIANCE.....	
STRUCTURAL BMP/MWS UNIT.....	

- SITE DESIGN BMPs:**
- 1 SD-1 MAINTAIN NATURAL HYDROLOGIC FEATURES
 - 2 SD-2 CONSERVE NATURAL AREAS, SOILS, VEGETATION
 - 3 SD-3 MINIMIZE IMPERVIOUS AREAS (FOR FUTURE PHASES)
 - 4 SD-4 MINIMIZE SOIL COMPACTION (FOR FUTURE PHASES)
 - 5 SD-5 IMPERVIOUS AREA DISPERSION
 - 6 SD-7 LANDSCAPING WITH NATIVE OR DROUGHT TOLERANT SPECIES (FOR FUTURE PHASES)
- SOURCE CONTROL BMPs:**
- 1 SC-1 PREVENTION OF ILLICIT DISCHARGES TO MS4
 - 2 SC-2 STORM DRAIN STENCILING OR SIGNAGE (FOR FUTURE PHASES)
 - 3 SC-6 ADDITIONAL BMPs BASED ON POTENTIAL SOURCES OF RUNOFF POLLUTANTS
 - SC-6A ON-SITE STORM DRAIN INLETS (FOR FUTURE PHASES)
 - SC-6D NEED FOR FUTURE INDOOR & STRUCTURAL PEST CONTROL (FOR FUTURE PHASES)
 - SC-6E LANDSCAPE/OUTDOOR PESTICIDE USE (FOR FUTURE PHASES)
 - SC-6F POOLS, SPAS, PONDS, FOUNTAINS, AND OTHER WATER FEATURES (FOR FUTURE PHASES)
 - SC-6G FIRE SPRINKLER TEST WATER (FOR FUTURE PHASES)
 - SC-6P MISCELLANEOUS DRAIN OR WASH WATER (FOR FUTURE PHASES)
 - SC-6Q PLAZAS, SIDEWALKS, AND PARKING LOTS (FOR FUTURE PHASES)

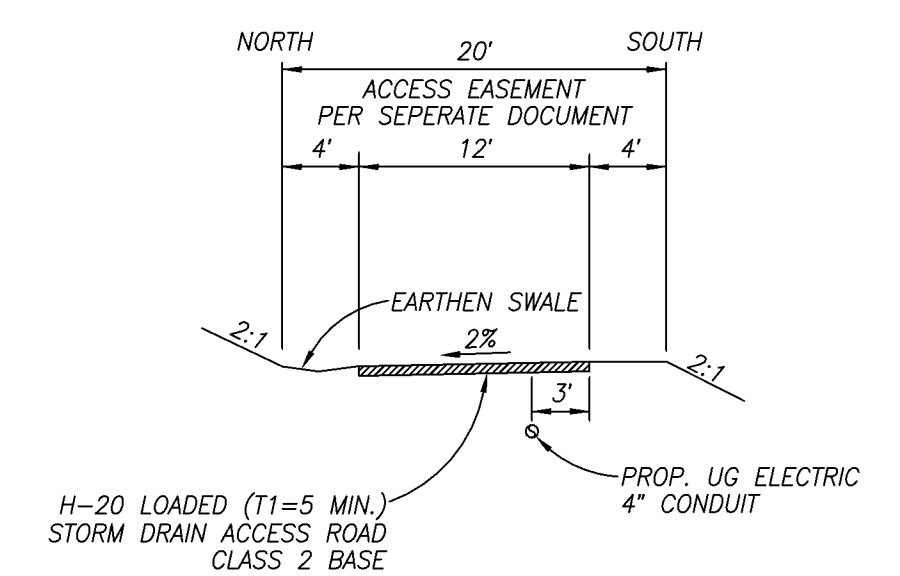
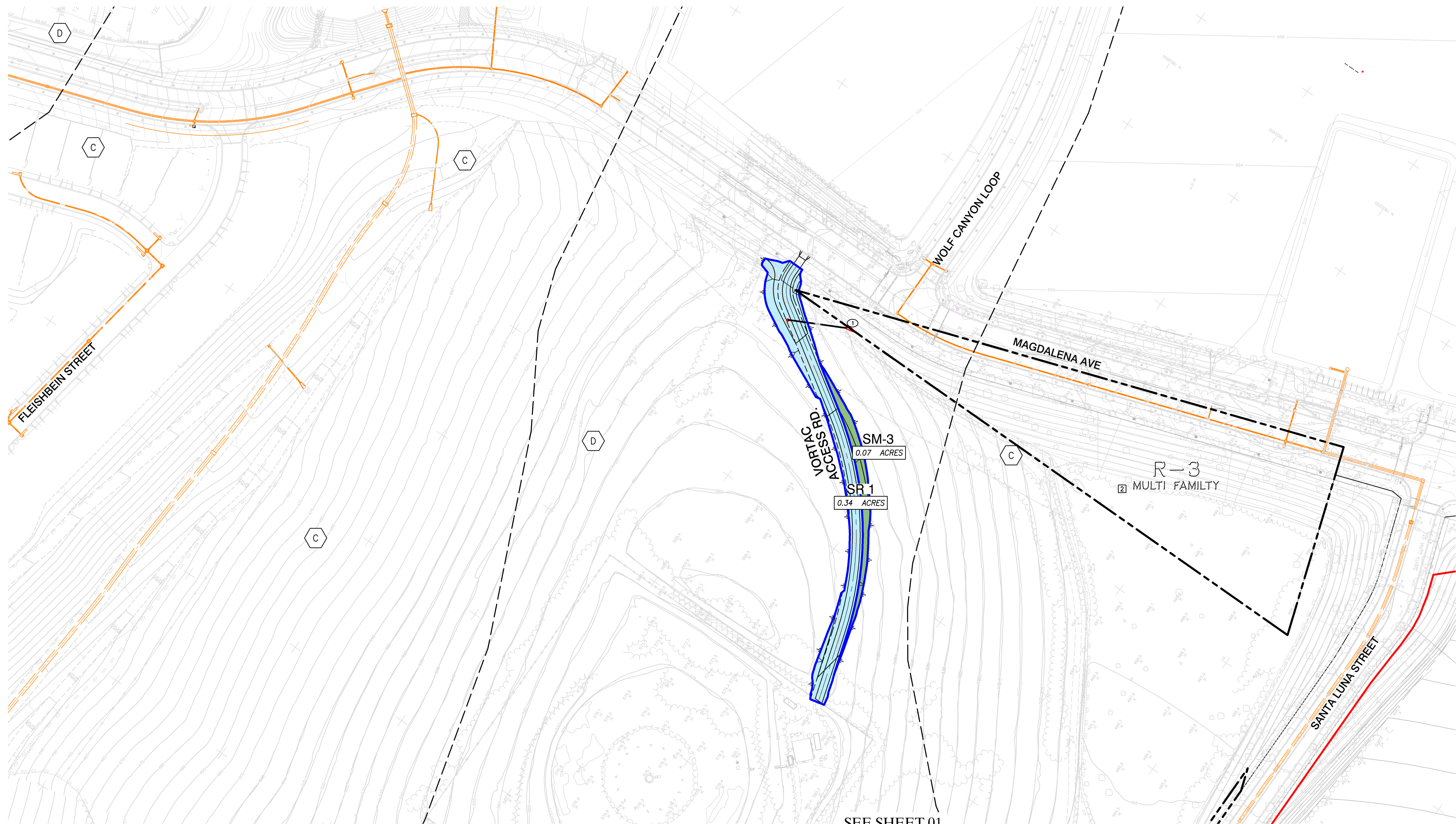
UNDERLYING SOIL GROUP : C & D
 APPROXIMATE DEPTH TO GROUNDWATER > 20'
 NO CRITICAL COARSE AREAS REQUIRE PRESERVATION



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DMA MAP
VILLAGE 7, R-3, R-4, & R-8
 CITY OF CHULA VISTA, CALIFORNIA

MAP
1
 OF
2
 M.O.# 9909-3336



VORTAC ACCESS ROAD
NOT TO SCALE
SELF-RETAINING DMA

LEGEND:	SYMBOL:
PROJECT BOUNDARY.....	
DMA BOUNDARY.....	
DAYLIGHT.....	
PROPOSED STORM DRAIN.....	
EXISTING STORM DRAIN.....	
SUBAREA ACREAGE.....	
DMA ICON.....	
R-8 (78% IMPERVIOUSNESS).....	
R-4 (78% IMPERVIOUSNESS).....	
SELF-RETAINING AREA.....	
PERVIOUS - BIOFILTRATION BASIN.....	
PERVIOUS AREAS.....	
SELF MITIGATING.....	
INLET.....	
HYDROLOGIC SOIL TYPE.....	
POINT OF COMPLIANCE.....	
STRUCTURAL BMP/ MWS UNIT.....	

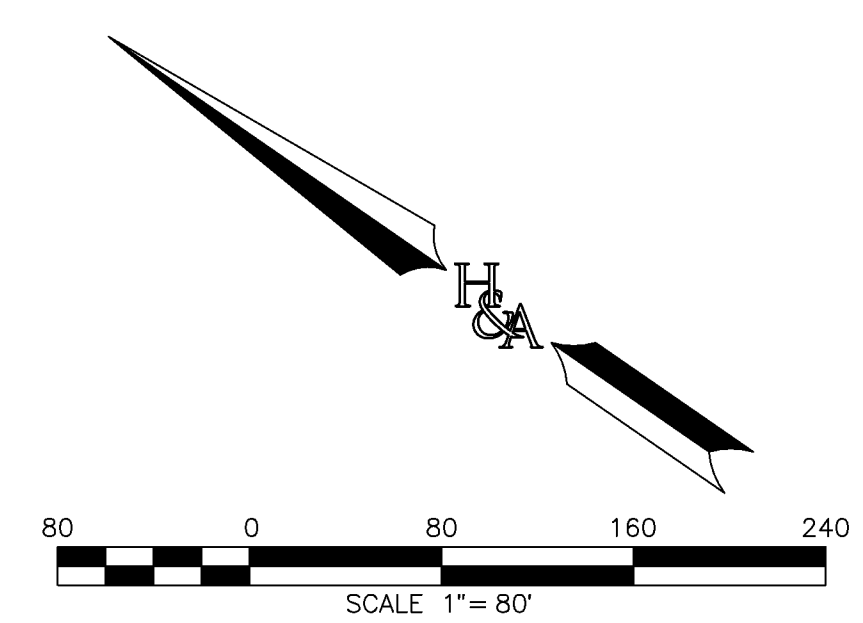
- SITE DESIGN BMPs:**
- SD-1 MAINTAIN NATURAL HYDROLOGIC FEATURES
 - SD-2 CONSERVE NATURAL AREAS, SOILS, VEGETATION
 - SD-3 MINIMIZE IMPERVIOUS AREAS (FOR FUTURE PHASES)
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- SC-6A ON-SITE STORM DRAIN INLETS (FOR FUTURE PHASES)**
- SC-6A ON-SITE STORM DRAIN INLETS (FOR FUTURE PHASES)
 - SC-6D NEED FOR FUTURE INDOOR & STRUCTURAL PEST CONTROL (FOR FUTURE PHASES)
 - SC-6E LANDSCAPE/OUTDOOR PESTICIDE USE (FOR FUTURE PHASES)
 - SC-6F POOLS, SPAS, PONDS, FOUNTAINS, AND OTHER WATER FEATURES (FOR FUTURE PHASES)
 - SC-6G FIRE SPRINKLER TEST WATER (FOR FUTURE PHASES)
 - SC-6P MISCELLANEOUS DRAIN OR WASH WATER (FOR FUTURE PHASES)
 - SC-6Q PLAZAS, SIDEWALKS, AND PARKING LOTS (FOR FUTURE PHASES)
- UNDERLYING SOIL GROUP : C & D
APPROXIMATE DEPTH TO GROUNDWATER > 20'
NO CRITICAL COARSE AREAS REQUIRE PRESERVATION

NOTES:

BOTH THE ACCESS ROAD AND R-3 ARE TRIBUTARY TO POC-2
THE ACCESS ROAD IS A SELF-RETAINING AREA PER CITY OF CHULA VISTA BMP MANUAL, CHAPTER 5, SECTION 5.2.1.

SELF-MITIGATING DMAS CONSIST OF NATURAL OR LANDSCAPED AREAS THAT DRAIN DIRECTLY OFFSITE OR TO THE PUBLIC STORM DRAIN SYSTEM. SELF-MITIGATING DMAS MUST MEET ALL THE FOLLOWING CHARACTERISTICS TO BE ELIGIBLE FOR EXCLUSION:

- VEGETATION IN THE NATURAL OR LANDSCAPED AREA IS NATIVE AND/OR NON-NATIVE/NON-INVASIVE DROUGHT TOLERANT SPECIES THAT DO NOT REQUIRE REGULAR APPLICATION OF FERTILIZERS AND PESTICIDES.
- SOILS ARE UNDISTURBED NATIVE TOPSOIL, OR DISTURBED SOILS THAT HAVE BEEN AMENDED AND AERATED TO PROMOTE WATER RETENTION CHARACTERISTICS EQUIVALENT TO UNDISTURBED NATIVE TOPSOIL.
- THE INCIDENTAL IMPERVIOUS AREAS ARE LESS THAN 5 PERCENT OF THE SELF-MITIGATING AREA.
- IMPERVIOUS AREA WITHIN THE SELF-MITIGATED AREA SHOULD NOT BE HYDRAULICALLY CONNECTED TO OTHER IMPERVIOUS AREAS UNLESS IT IS A STORM WATER CONVEYANCE SYSTEM (SUCH AS BROW DITCHES).
- THE SELF-MITIGATING AREA IS HYDRAULICALLY SEPARATE FROM DMAS THAT CONTAIN PERMANENT STORM WATER POLLUTANT CONTROL BMPs.



PREPARED BY: HUNSAKER & ASSOCIATES SAN DIEGO, INC. PLANNING 9707 Waples Street ENGINEERING San Diego, Ca 92121 SURVEYING PH(619)558-4500 - FX(619)558-1414	DMA MAP VILLAGE 7, R-3, R-4, & R-8 CITY OF CHULA VISTA, CALIFORNIA	MAP
		2
		OF
		2

ATTACHMENT 1b
TABULAR SUMMARY OF DMAs

Project Name: Otay Ranch Village 7, R-3, R-4, & R-8

Tabular Summary of DMAs							Worksheet B-1		
DMA Unique Identifier	Area (acres)	Impervious Area (acres)	% Imp	HSG	Area Weighted Runoff Coefficient	DCV (Cubic feet)	Treated by (BMP ID)	Pollutant Control Type	Drains to (POC ID)
DMA 1	7.97	6.22	78	D/C	0.72	16,650	16,650	Biofiltration	1
DMA 2	2.85	2.22	78	D/C	0.72	5,950	5,950	Biofiltration	2
SR-1	0.34	0	0	D/C	0.2	132	N/A	Self-retaining	2
Summary of DMA Information (Must match Project description and SWQMP narrative)									
No. of DMAs	Total DMA Area (acres)	Total Impervious Area (acres)	% Impervious		Area Weighted Runoff Coefficient	DCV (Cubic feet)	Total Area Treated (acres)		No. of POCs
3	11.16	8.44	76		0.71	22,732	11.16		2

Where: DMA = Drainage Management Area Imp = Imperviousness ID = identifier
HSG = Hydrologic Soil Group DCV= Design Capture Volume No. = Number
BMP = Best Management Practice POC = Point of Compliance



VILLAGE 7, R-8 R-4
DMA CALCULATIONS

	Imp. RF	Pervious RF	% Imp	DMA1	Fraction of Total	Imp Area	Pervious Area	Summation RF x A	DMA-2	Fraction of Total	Imp Area	Pervious Area	Summation RF x A	SR-1	Fraction of Total	Imp Area	Pervious Area	Summation RF x A
LANDSCAPE	0.90	0.10	0	76369	0.03	0	76369	7637	27291	0.03	0	27291	2729	7568	0.25	0	7568	757
MULTI-FAMILY	0.90	0.10	100	270761	0.97	270761	0	243685	96759	0.97	96759	0	87083	0	0.00	0	0	0
BASIN	0.90	0.10	0	0	0.00	0	0	0	0	0.00	0	0	0	0	0.00	0	0	0
SEMI-PERVIOUS ROAD	0.90	0.30	0	0	0.00	0	0	0	0	0.00	0	0	0	7432	0.75	0	7432	2230
ROAD	0.90	0.10	100	0	0.00	0	0	0	0	0.00	0	0	0	0	0.00	0	0	0
				347130	1.00	270761	76369	251322	124050	1.00	96759	27291	89812	15000	1.00	0	15000	2986
			%Imperv	78.00			Weighted C =	0.72	78.00			Weighted C =	0.72	49.55			Weighted C =	0.20

	Imp. RF	Pervious RF	% Imp	SM-1	Fraction of Total	Imp Area	Pervious Area	Summation RF x A	SM-2	Fraction of Total	Imp Area	Pervious Area	Summation RF x A	SM-3	Fraction of Total	Imp Area	Pervious Area	
				SOFT		SOFT	SOFT		SOFT		SOFT	SOFT		SOFT		SOFT	SOFT	
LANDSCAPE	0.90	0.10	0	129147	1.00	0	129147	12915	124304	1.00	0	124304.44	12430	3180	1.00	0	3180	318
				129147	1.00	0	129147	12915	124304	1.00	0	124304	12430	3180	1.00	0	3180	318
			%Imperv	0.00			Weighted C =	0.10	0.00			Weighted C =	0.10	0.00			Weighted C =	0.10

ATTACHMENT 1c

FORM I-7, HARVEST AND USE FEASIBILITY SCREENING CHECKLIST

Project Name: _____

Harvest and Use Feasibility Screening		FORM I-7 (Worksheet B.3-1)
<p>1. Is there a demand for harvested water (check all that apply) at the project site that is reliably present during the wet season?</p> <p><input checked="" type="checkbox"/> Toilet and urinal flushing</p> <p><input checked="" type="checkbox"/> Landscape irrigation</p> <p><input type="checkbox"/> Other: _____</p>		
<p>2. If there is a demand; estimate the anticipated average wet season demand over a period of 36 hours. Guidance for planning level demand calculations for toilet/urinal flushing and landscape irrigation is provided in Section B.3.2.</p> <p>[Provide a summary of calculations here]</p> <p>36 hour urinal and flushing demand = 9.3 gal/resident x 4 residents per unit x 243 units x 1.5 days = 13,559 gallons = 1812 cf</p> <p>Landscape Irrigation: 390 gals/ac x 8.44 ac = 3292 gals = 440 cf</p> <p>Total Volume = 2252 cf</p>		
<p>3. Calculate the DCV using worksheet B-2.1.</p> <p>[Provide a result here]</p> <p>DCV = 16,650 + 5,950 = 22,600</p> <p>0.25 DCV = 5,650</p>		
<p>3a. Is the 36-hour demand greater than or equal to the DCV?</p> <p>Yes <input type="checkbox"/> / No <input checked="" type="checkbox"/> </p>	<p>3b. Is the 36-hour demand greater than 0.25DCV but less than the full DCV?</p> <p>Yes <input type="checkbox"/> / No <input checked="" type="checkbox"/> </p>	<p>3c. Is the 36-hour demand less than 0.25DCV?</p> <p>Yes <input checked="" type="checkbox"/></p>
<p>Harvest and use appears to be feasible. Conduct more detailed evaluation and sizing calculations to confirm that DCV can be used at an adequate rate to meet drawdown criteria.</p>	<p>Harvest and use may be feasible. Conduct more detailed evaluation and sizing calculations to determine feasibility. Harvest and use may only be able to be used for a portion of the site, or (optionally) the storage may need to be upsized to meet long term capture targets while draining in longer than 36 hours.</p>	<p>Harvest and use is considered to be infeasible.</p>

Note: 36-hour demand calculations are for feasibility analysis only, once the feasibility analysis is complete the applicant may be allowed to use a different drawdown time provided they meet the 80 percent of average annual (long term) runoff volume performance standard.

ATTACHMENT 1d

FORM I-8, CATEGORIZATION OF INFILTRATION FEASIBILITY CONDITION

Project Name: _____

Categorization of Infiltration Feasibility Condition based on Geotechnical Conditions		Form I-8A ¹ (Worksheet C.4-1)
Part 1 - Full Infiltration Feasibility Screening Criteria		
DMA(s) Being Analyzed:		Project Phase:
DMA 1 and 2		Planning
Criteria 1: Infiltration Rate Screening		
1A	<p>Is the mapped hydrologic soil group according to the NRCS Web Soil Survey or UC Davis Soil Web Mapper Type A or B and corroborated by available site soil data²?</p> <p><input type="checkbox"/> Yes; the DMA may feasibly support full infiltration. Answer “Yes” to Criteria 1 Result or continue to Step 1B if the applicant elects to perform infiltration testing.</p> <p><input type="checkbox"/> No; the mapped soil types are A or B but is not corroborated by available site soil data (continue to Step 1B).</p> <p><input checked="" type="checkbox"/> No; the mapped soil types are C, D, or “urban/unclassified” and is corroborated by available site soil data. Answer “No” to Criteria 1 Result.</p> <p><input type="checkbox"/> No; the mapped soil types are C, D, or “urban/unclassified” but is not corroborated by available site soil data (continue to Step 1B).</p>	
1B	<p>Is the reliable infiltration rate calculated using planning phase methods from Table D.3-1²?</p> <p><input type="checkbox"/> Yes; Continue to Step 1C.</p> <p><input type="checkbox"/> No; Skip to Step 1D.</p>	
1C	<p>Is the reliable infiltration rate calculated using planning phase methods from Table D.3-1 greater than 0.5 inches per hour?</p> <p><input type="checkbox"/> Yes; the DMA may feasibly support full infiltration. Answer “Yes” to Criteria 1 Result.</p> <p><input type="checkbox"/> No; full infiltration is not required. Answer “No” to Criteria 1 Result.</p>	
1D	<p>Infiltration Testing Method. Is the selected infiltration testing method suitable during the design phase (see Appendix D.3)? Note: Alternative testing standards may be allowed with appropriate rationales and documentation.</p> <p><input type="checkbox"/> Yes; continue to Step 1E.</p> <p><input type="checkbox"/> No; select an appropriate infiltration testing method.</p>	
1E	<p>Number of Percolation/Infiltration Tests. Does the infiltration testing method performed satisfy the minimum number of tests specified in Table D.3-2?</p> <p><input type="checkbox"/> Yes; continue to Step 1F.</p> <p><input type="checkbox"/> No; conduct appropriate number of tests.</p>	

¹ This form must be completed each time there is a change to the site layout that would affect the infiltration feasibility condition. Previously completed forms shall be retained to document the evolution of the site storm water design.

² Available data includes site-specific sampling or observation of soil types or texture classes, such as obtained from borings or test pits necessary to support other design elements.



Project Name: _____

Categorization of Infiltration Feasibility Condition based on Geotechnical Conditions		Form I-8A ¹ (Worksheet C.4-1)
IF	<p>Factor of Safety. Is the suitable Factor of Safety selected for full infiltration design? See guidance in D.5; Tables D.5-1 and D.5-2; and Worksheet D.5-1 (Form I-9).</p> <p><input type="checkbox"/> Yes; continue to Step 1G. <input type="checkbox"/> No; select appropriate factor of safety.</p>	
1G	<p>Full Infiltration Feasibility. Is the average measured infiltration rate divided by the Factor of Safety greater than 0.5 inches per hour?</p> <p><input type="checkbox"/> Yes; answer “Yes” to Criteria 1 Result. <input type="checkbox"/> No; answer “No” to Criteria 1 Result.</p>	
Criteria 1 Result	<p>Is the estimated reliable infiltration rate greater than 0.5 inches per hour within the DMA where runoff can reasonably be routed to a BMP?</p> <p><input type="checkbox"/> Yes; the DMA may feasibly support full infiltration. Continue to Criteria 2. <input checked="" type="checkbox"/> No; full infiltration is not required. Skip to Part 1 Result.</p>	
<p>Summarize infiltration testing methods, testing locations, replicates, and results and summarize estimates of reliable infiltration rates according to procedures outlined in D.5. Documentation should be included in project geotechnical report.</p> <p>Geotechnical Reconnaissance report to be performed at a later date.</p>		
Criteria 2: Geologic/Geotechnical Screening		
2A	<p>If all questions in Step 2A are answered “Yes,” continue to Step 2B.</p> <p>For any “No” answer in Step 2A answer “No” to Criteria 2 and submit an “Infiltration Feasibility Condition Letter” that meets the requirements in Appendix C.1.1.</p> <p>The geologic/geotechnical analyses listed in Appendix C.2.1 do not apply to the DMA because one of the following setbacks cannot be avoided and therefore result in the DMA being in a no infiltration condition. The setbacks must be the closest horizontal radial distance from the surface edge (at the overflow elevation) of the BMP.</p>	



Otay Ranch Village 7, R-3, R-4 & R-8

Project Name: _____

Categorization of Infiltration Feasibility Condition based on Geotechnical Conditions		Form I-8A ¹ (Worksheet C.4-1)	
2A-1	Can the proposed full infiltration BMP(s) avoid areas with existing fill materials greater than 5 feet thick below the infiltrating surface?	<input type="checkbox"/> Yes	<input type="checkbox"/> No
2A-2	Can the proposed full infiltration BMP(s) avoid placement within 10 feet of existing underground utilities, structures, or retaining walls?	<input type="checkbox"/> Yes	<input type="checkbox"/> No
2A-3	Can the proposed full infiltration BMP(s) avoid placement within 50 feet of a natural slope (>25%) or within a distance of 1.5H from fill slopes where H is the height of the fill slope?	<input type="checkbox"/> Yes	<input type="checkbox"/> No
2B	When full infiltration is determined to be feasible, a geotechnical investigation report must be prepared that considers the relevant factors identified in Appendix C.2.1. If all questions in Step 2B are answered "Yes," then answer "Yes" to Criteria 2 Result. If there are "No" answers continue to Step 2C.		
2B-1	Hydroconsolidation. Analyze hydroconsolidation potential per approved ASTM standard due to a proposed full infiltration BMP. Can full infiltration BMPs be proposed within the DMA without increasing hydroconsolidation risks?	<input type="checkbox"/> Yes	<input type="checkbox"/> No
2B-2	Expansive Soils. Identify expansive soils (soils with an expansion index greater than 20) and the extent of such soils due to proposed full infiltration BMPs. Can full infiltration BMPs be proposed within the DMA without increasing expansive soil risks?	<input type="checkbox"/> Yes	<input type="checkbox"/> No
2B-3	Liquefaction. If applicable, identify mapped liquefaction areas. Evaluate liquefaction hazards in accordance with Section 6.4.2 of the City of San Diego's Guidelines for Geotechnical Reports (2011 or most recent edition). Liquefaction hazard assessment shall take into account any increase in groundwater elevation or groundwater mounding that could occur as a result of proposed infiltration or percolation facilities. Can full infiltration BMPs be proposed within the DMA without increasing liquefaction risks?	<input type="checkbox"/> Yes	<input type="checkbox"/> No
2B-4	Slope Stability. If applicable, perform a slope stability analysis in accordance with the ASCE and Southern California Earthquake Center (2002) Recommended Procedures for Implementation of DMG Special Publication 117, Guidelines for Analyzing and Mitigating Landslide Hazards in California to determine minimum slope setbacks for full infiltration BMPs. See the City of San Diego's Guidelines for Geotechnical Reports (2011) to determine which type of slope stability analysis is required. Can full infiltration BMPs be proposed within the DMA without increasing slope stability risks?	<input type="checkbox"/> Yes	<input type="checkbox"/> No
2B-5	Other Geotechnical Hazards. Identify site-specific geotechnical hazards not already mentioned (refer to Appendix C.2.1). Can full infiltration BMPs be proposed within the DMA without increasing risk of geologic or geotechnical hazards not already mentioned?	<input type="checkbox"/> Yes	<input type="checkbox"/> No

Project Name: _____

Categorization of Infiltration Feasibility Condition based on Geotechnical Conditions		Form I-8A ¹ (Worksheet C.4-1)	
2B-6	<p>Setbacks. Establish setbacks from underground utilities, structures, and/or retaining walls. Reference applicable ASTM or other recognized standard in the geotechnical report.</p> <p>Can full infiltration BMPs be proposed within the DMA using established setbacks from underground utilities, structures, and/or retaining walls?</p>	<input type="checkbox"/> Yes	<input type="checkbox"/> No
2C	<p>Mitigation Measures. Propose mitigation measures for each geologic/geotechnical hazard identified in Step 2B. Provide a discussion of geologic/geotechnical hazards that would prevent full infiltration BMPs that cannot be reasonably mitigated in the geotechnical report. See Appendix C.2.1.8 for a list of typically reasonable and typically unreasonable mitigation measures.</p> <p>Can mitigation measures be proposed to allow for full infiltration BMPs? If the question in Step 2 is answered “Yes,” then answer “Yes” to Criteria 2 Result.</p> <p>If the question in Step 2C is answered “No,” then answer “No” to Criteria 2 Result.</p>	<input type="checkbox"/> Yes	<input type="checkbox"/> No
Criteria 2 Result	<p>Can infiltration greater than 0.5 inches per hour be allowed without increasing risk of geologic or geotechnical hazards that cannot be reasonably mitigated to an acceptable level?</p>	<input type="checkbox"/> Yes	<input type="checkbox"/> No
<p>Summarize findings and basis; provide references to related reports or exhibits.</p> <p>Geotechnical Reconnaissance report to be performed at a later date.</p>			
Part 1 Result – Full Infiltration Geotechnical Screening ³		Result	
<p>If answers to both Criteria 1 and Criteria 2 are “Yes”, a full infiltration design is potentially feasible based on Geotechnical conditions only.</p> <p>If either answer to Criteria 1 or Criteria 2 is “No”, a full infiltration design is not required.</p>		<p><input type="checkbox"/> Full infiltration Condition</p> <p><input checked="" type="checkbox"/> Complete Part 2</p>	

³ To be completed using gathered site information and best professional judgement considering the definition of MEP in the MS4 Permit. Additional testing and/or studies may be required by City Engineer to substantiate findings.



Project Name: _____

Categorization of Infiltration Feasibility Condition based on Geotechnical Conditions		Form I-8A ¹ (Worksheet C.4-1)
Part 2 – Partial vs. No Infiltration Feasibility Screening Criteria		
DMA(s) Being Analyzed:		Project Phase:
DMA 1 and 2		Planning
Criteria 3 : Infiltration Rate Screening		
3A	<p>NRCS Type C, D, or “urban/unclassified”: Is the mapped hydrologic soil group according to the NRCS Web Soil Survey or UC Davis Soil Web Mapper is Type C, D, or “urban/unclassified” and corroborated by available site soil data?</p> <p><input type="checkbox"/> Yes; the site is mapped as C soils and a reliable infiltration rate of 0.15 in/hr. is used to size partial infiltration BMPS. Answer “Yes” to Criteria 3 Result.</p> <p><input checked="" type="checkbox"/> Yes; the site is mapped as D soils or “urban/unclassified” and a reliable infiltration rate of 0.05 in/hr. is used to size partial infiltration BMPS. Answer “Yes” to Criteria 3 Result.</p> <p><input type="checkbox"/> No; infiltration testing is conducted (refer to Table D.3-1), continue to Step 3B.</p>	
3B	<p>Infiltration Testing Result: Is the reliable infiltration rate (i.e. average measured infiltration rate/2) greater than 0.05 in/hr. and less than or equal to 0.5 in/hr?</p> <p><input type="checkbox"/> Yes; the site may support partial infiltration. Answer “Yes” to Criteria 3 Result.</p> <p><input type="checkbox"/> No; the reliable infiltration rate (i.e. average measured rate/2) is less than 0.05 in/hr., partial infiltration is not required. Answer “No” to Criteria 3 Result.</p>	
Criteria 3 Result	<p>Is the estimated reliable infiltration rate (i.e., average measured infiltration rate/2) greater than or equal to 0.05 inches/hour and less than or equal to 0.5 inches/hour at any location within each DMA where runoff can reasonably be routed to a BMP?</p> <p><input type="checkbox"/> Yes; Continue to Criteria 4.</p> <p><input type="checkbox"/> No: Skip to Part 2 Result.</p>	
<p>Summarize infiltration testing and/or mapping results (i.e. soil maps and series description used for infiltration rate).</p> <p>Geotechnical Reconnaissance report to be performed at a later date.</p>		

Project Name: _____

Categorization of Infiltration Feasibility Condition based on Geotechnical Conditions		Form I-8A ¹ (Worksheet C.4-1)	
Criteria 4: Geologic/Geotechnical Screening			
4A	<p>If all questions in Step 4A are answered “Yes,” continue to Step 2B.</p> <p>For any “No” answer in Step 4A answer “No” to Criteria 4 Result, and submit an “Infiltration Feasibility Condition Letter” that meets the requirements in Appendix C.1.1. The geologic/geotechnical analyses listed in Appendix C.2.1 do not apply to the DMA because one of the following setbacks cannot be avoided and therefore result in the DMA being in a no infiltration condition. The setbacks must be the closest horizontal radial distance from the surface edge (at the overflow elevation) of the BMP.</p>		
4A-1	Can the proposed partial infiltration BMP(s) avoid areas with existing fill materials greater than 5 feet thick?	<input type="checkbox"/> Yes	<input type="checkbox"/> No
4A-2	Can the proposed partial infiltration BMP(s) avoid placement within 10 feet of existing underground utilities, structures, or retaining walls?	<input type="checkbox"/> Yes	<input type="checkbox"/> No
4A-3	Can the proposed partial infiltration BMP(s) avoid placement within 50 feet of a natural slope (>25%) or within a distance of 1.5H from fill slopes where H is the height of the fill slope?	<input type="checkbox"/> Yes	<input type="checkbox"/> No
4B	<p>When full infiltration is determined to be feasible, a geotechnical investigation report must be prepared that considers the relevant factors identified in Appendix C.2.1.</p> <p>If all questions in Step 4B are answered “Yes,” then answer “Yes” to Criteria 4 Result. If there are any “No” answers continue to Step 4C.</p>		
4B-1	<p>Hydroconsolidation. Analyze hydroconsolidation potential per approved ASTM standard due to a proposed full infiltration BMP.</p> <p>Can partial infiltration BMPs be proposed within the DMA without increasing hydroconsolidation risks?</p>	<input type="checkbox"/> Yes	<input type="checkbox"/> No
4B-2	<p>Expansive Soils. Identify expansive soils (soils with an expansion index greater than 20) and the extent of such soils due to proposed full infiltration BMPs.</p> <p>Can partial infiltration BMPs be proposed within the DMA without increasing expansive soil risks?</p>	<input type="checkbox"/> Yes	<input type="checkbox"/> No
4B-3	<p>Liquefaction. If applicable, identify mapped liquefaction areas. Evaluate liquefaction hazards in accordance with Section 6.4.2 of the City of San Diego's Guidelines for Geotechnical Reports (2011). Liquefaction hazard assessment shall take into account any increase in groundwater elevation or groundwater mounding that could occur as a result of proposed infiltration or percolation facilities.</p> <p>Can partial infiltration BMPs be proposed within the DMA without increasing liquefaction risks?</p>	<input type="checkbox"/> Yes	<input type="checkbox"/> No

Project Name: _____

Categorization of Infiltration Feasibility Condition based on Geotechnical Conditions		Form I-8A ¹ (Worksheet C.4-1)	
4B-4	<p>Slope Stability. If applicable, perform a slope stability analysis in accordance with the ASCE and Southern California Earthquake Center (2002) Recommended Procedures for Implementation of DMG Special Publication 117, Guidelines for Analyzing and Mitigating Landslide Hazards in California to determine minimum slope setbacks for full infiltration BMPs. See the City of San Diego's Guidelines for Geotechnical Reports (2011) to determine which type of slope stability analysis is required.</p> <p>Can partial infiltration BMPs be proposed within the DMA without increasing slope stability risks?</p>	<input type="checkbox"/> Yes	<input type="checkbox"/> No
4B-5	<p>Other Geotechnical Hazards. Identify site-specific geotechnical hazards not already mentioned (refer to Appendix C.2.1).</p> <p>Can partial infiltration BMPs be proposed within the DMA without increasing risk of geologic or geotechnical hazards not already mentioned?</p>	<input type="checkbox"/> Yes	<input type="checkbox"/> No
4B-6	<p>Setbacks. Establish setbacks from underground utilities, structures, and/or retaining walls. Reference applicable ASTM or other recognized standard in the geotechnical report.</p> <p>Can partial infiltration BMPs be proposed within the DMA using recommended setbacks from underground utilities, structures, and/or retaining walls?</p>	<input type="checkbox"/> Yes	<input type="checkbox"/> No
4C	<p>Mitigation Measures. Propose mitigation measures for each geologic/geotechnical hazard identified in Step 4B. Provide a discussion on geologic/geotechnical hazards that would prevent partial infiltration BMPs that cannot be reasonably mitigated in the geotechnical report. See Appendix C.2.1.8 for a list of typically reasonable and typically unreasonable mitigation measures.</p> <p>Can mitigation measures be proposed to allow for partial infiltration BMPs? If the question in Step 4C is answered "Yes," then answer "Yes" to Criteria 4 Result.</p> <p>If the question in Step 4C is answered "No," then answer "No" to Criteria 4 Result.</p>	<input type="checkbox"/> Yes	<input type="checkbox"/> No
Criteria 4 Result	<p>Can infiltration of greater than or equal to 0.05 inches/hour and less than or equal to 0.5 inches/hour be allowed without increasing the risk of geologic or geotechnical hazards that cannot be reasonably mitigated to an acceptable level?</p>	<input type="checkbox"/> Yes	<input type="checkbox"/> No

Project Name: _____

Categorization of Infiltration Feasibility Condition based on Geotechnical Conditions	Form I-8A ¹ (Worksheet C.4-1)
<p>Summarize findings and basis; provide references to related reports or exhibits.</p> <p>Geotechnical Reconnaissance report to be performed at a later date.</p>	
Part 2 – Partial Infiltration Geotechnical Screening Result ⁴	Result
<p>If answers to both Criteria 3 and Criteria 4 are “Yes”, a partial infiltration design is potentially feasible based on geotechnical conditions only.</p> <p>If answers to either Criteria 3 or Criteria 4 is “No”, then infiltration of any volume is considered to be infeasible within the site.</p>	<p><input type="checkbox"/> Partial Infiltration Condition</p> <p><input checked="" type="checkbox"/> No Infiltration Condition</p>

⁴ To be completed using gathered site information and best professional judgement considering the definition of MEP in the MS4 Permit. Additional testing and/or studies may be required by City Engineer to substantiate findings.

ATTACHMENT 1e
POLLUTION CONTROL BMP DESIGN WORKSHEETS

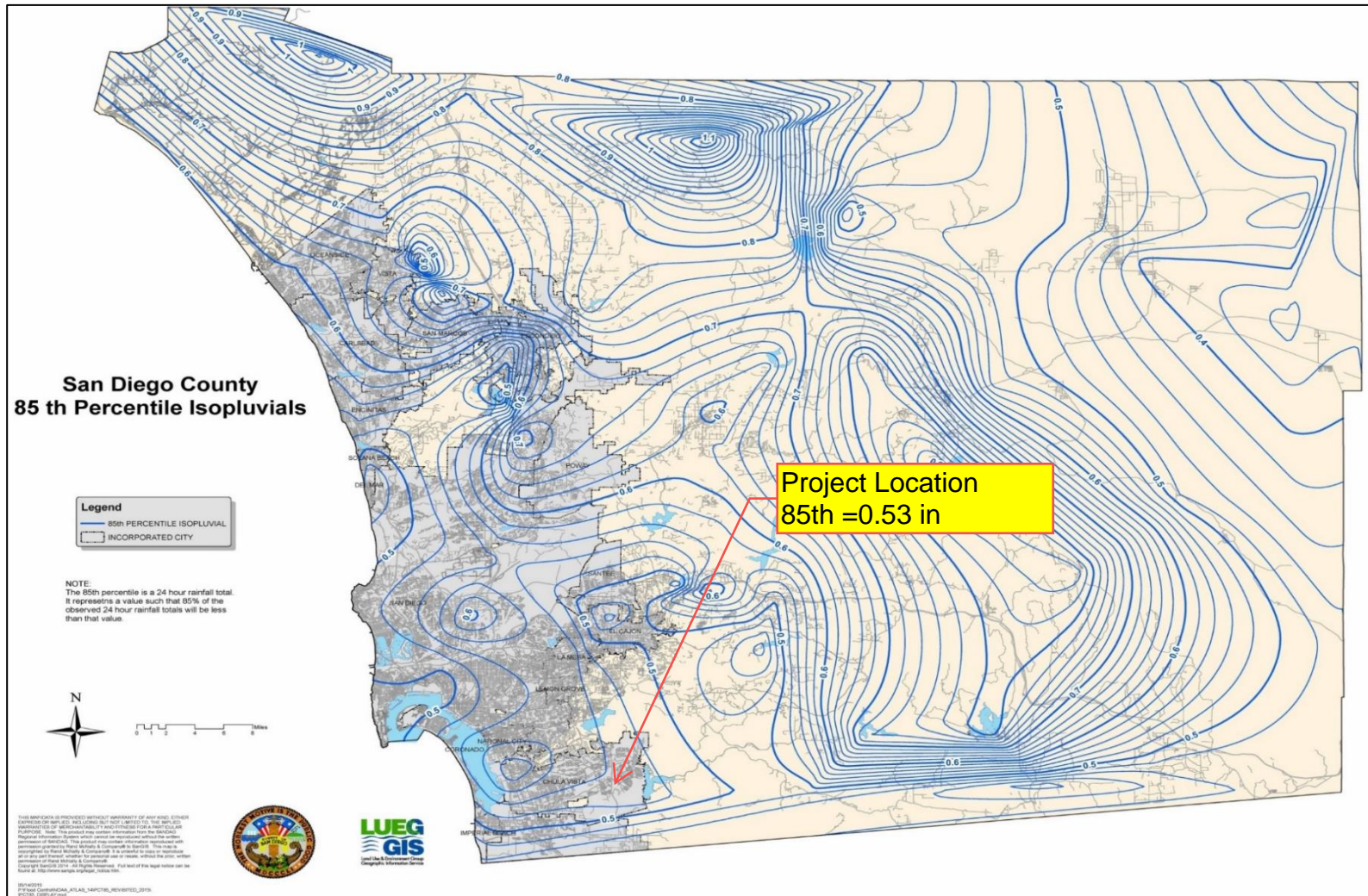


Figure B.1-1: 85th Percentile 24-hour Isopluvial Map

- (b) The retention losses from the optimized biofiltration BMP are equal to or greater than the retention losses from the conventional biofiltration BMP. This second criterion is only applicable for partial infiltration condition.

For drawdown times that are outside the range of values presented in Table B.5-5 below, the storage unit should be designed to discharge greater than 92% average annual capture to the downstream Biofiltration BMP.

Table B.5-5: Storage required for different drawdown times

Drawdown Time (hours)	Storage requirement (below the overflow elevation, or below outlet elevation that bypass the biofiltration BMP)
12	0.85 DCV
24	1.25 DCV
36	1.50 DCV
48	1.80 DCV
72	2.20 DCV
96	2.60 DCV
120	2.80 DCV

VILLAGE 7, R-8 R-4
DMA CALCULATIONS

	Imp. RF	Pervious RF	% Imp	DMA1	Fraction of Total	Imp Area	Pervious Area	Summation RF x A	DMA-2	Fraction of Total	Imp Area	Pervious Area	Summation RF x A	SR-1	Fraction of Total	Imp Area	Pervious Area	Summation RF x A
LANDSCAPE	0.90	0.10	0	76369	0.03	0	76369	7637	27291	0.03	0	27291	2729	7568	0.25	0	7568	757
MULTI-FAMILY	0.90	0.10	100	270761	0.97	270761	0	243685	96759	0.97	96759	0	87083	0	0.00	0	0	0
BASIN	0.90	0.10	0	0	0.00	0	0	0	0	0.00	0	0	0	0	0.00	0	0	0
SEMI-PERVIOUS ROAD	0.90	0.30	0	0	0.00	0	0	0	0	0.00	0	0	0	7432	0.75	0	7432	2230
ROAD	0.90	0.10	100	0	0.00	0	0	0	0	0.00	0	0	0	0	0.00	0	0	0
				347130	1.00	270761	76369	251322	124050	1.00	96759	27291	89812	15000	1.00	0	15000	2986
			%Imperv	78.00			Weighted C =	0.72	78.00			Weighted C =	0.72	49.55			Weighted C =	0.20

	Imp. RF	Pervious RF	% Imp	SM-1	Fraction of Total	Imp Area	Pervious Area	Summation RF x A	SM-2	Fraction of Total	Imp Area	Pervious Area	Summation RF x A	SM-3	Fraction of Total	Imp Area	Pervious Area	
				SOFT		SOFT	SOFT		SOFT		SOFT	SOFT		SOFT		SOFT	SOFT	
LANDSCAPE	0.90	0.10	0	129147	1.00	0	129147	12915	124304	1.00	0	124304.44	12430	3180	1.00	0	3180	318
				129147	1.00	0	129147	12915	124304	1.00	0	124304	12430	3180	1.00	0	3180	318
			%Imperv	0.00			Weighted C =	0.10	0.00			Weighted C =	0.10	0.00			Weighted C =	0.10

VILLAGE 7, R-8 R-4 DCV CALCULATIONS

DMA 1: Design Capture Volume		Worksheet B-2.1		
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.53	inches
2	Area tributary to BMP (s)	A=	7.97	acres
	Area tributary to BMP (s)	A=	347,130	s.f.
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1)	C=	0.72	unitless
4	Street trees volume reduction	TCV=	0.00	cubic-feet
5	Rain barrels volume reduction	RCV=	0.00	cubic-feet
6	Calculate DCV= (3630 x C x d x A) - TCV - RCV	DCV=	11,100	cubic-feet
		1.5 DCV=	16,650	cubic-feet

DMA 2: Design Capture Volume		Worksheet B-2.1		
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.53	inches
2	Area tributary to BMP (s)	A=	2.85	acres
	Area tributary to BMP (s)	A=	124,050	s.f.
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1)	C=	0.72	unitless
4	Street trees volume reduction	TCV=	0.00	cubic-feet
5	Rain barrels volume reduction	RCV=	0.00	cubic-feet
6	Calculate DCV= (3630 x C x d x A) - TCV - RCV	DCV=	3,967	cubic-feet
		1.5 DCV=	5,950	cubic-feet

SR-1 Design Capture Volume		Worksheet B-2.1		
1	85th percentile 24-hr storm depth from Figure B.1-1	d=	0.53	inches
2	Area tributary to BMP (s)	A=	0.34	acres
	Area tributary to BMP (s)	A=	15,000	s.f.
3	Area weighted runoff factor (estimate using Appendix B.1.1 and B.2.1)	C=	0.20	unitless
4	Street trees volume reduction	TCV=	0.00	cubic-feet
5	Rain barrels volume reduction	RCV=	0.00	cubic-feet
6	Calculate DCV= (3630 x C x d x A) - TCV - RCV	DCV=	132	cubic-feet



Project Name Otay Ranch Village 7, R-8 & R-4

BMP ID BF-3-1

Sizing Method for Volume Retention Criteria

Worksheet B.5-2

1	Area draining to the BMP	347,130	sq. ft.
2	Adjusted runoff factor for drainage area (Refer to Appendix B.1 and B.2)	0.72	
3	85 th percentile 24-hour rainfall depth	0.53	inches
4	Design capture volume [Line 1 x Line 2 x (Line 3/12)]	11100	cu. ft.
Volume Retention Requirement			
5	<p>Measured infiltration rate in the DMA</p> <p>Note:</p> <p>When mapped hydrologic soil groups are used enter 0.10 for NRCS Type D soils and for NRCS Type C soils enter 0.30</p> <p>When in no infiltration condition and the actual measured infiltration rate is unknown enter 0.0 if there are geotechnical and/or groundwater hazards identified in Appendix C or enter 0.05</p>	0	in/hr.
6	Factor of safety	2	
7	Reliable infiltration rate, for biofiltration BMP sizing [Line 5 / Line 6]	0	in/hr.
8	<p>Average annual volume reduction target (Figure B.5-2)</p> <p>When Line 7 > 0.01 in/hr. = Minimum (40, 166.9 x Line 7 + 6.62)</p> <p>When Line 7 ≤ 0.01 in/hr. = 3.5%</p>	3.5	%
9	<p>Fraction of DCV to be retained (Figure B.5-3)</p> <p>When Line 8 > 8% =</p> $0.0000013 \times \text{Line } 8^3 - 0.000057 \times \text{Line } 8^2 + 0.0086 \times \text{Line } 8 - 0.014$ <p>When Line 8 ≤ 8% = 0.023</p>	0.023	
10	Target volume retention [Line 9 x Line 4]	255	cu. ft.



Project Name Otay Ranch Village 7, R-8 & R-4

BMP ID BF-3-1

Volume Retention for No Infiltration Condition

Worksheet B.5-6

1	Area draining to the biofiltration BMP	347,130	sq. ft.
2	Adjusted runoff factor for drainage area (Refer to Appendix B.1 and B.2)	0.72	
3	Effective impervious area draining to the BMP [Line 1 x Line 2]	251322	sq. ft.
4	Required area for Evapotranspiration [Line 3 x 0.03]	7540	sq. ft.
5	Biofiltration BMP Footprint	0	sq. ft.

Landscape Area (must be identified on DS-3247)

	Identification	1	2	3	4	5
6	Landscape area that meet the requirements in SD-B and SD-F Fact Sheet (sq. ft.)					
7	Impervious area draining to the landscape area (sq. ft.)					
8	Impervious to Pervious Area ratio [Line 7/Line 6]	0.00	0.00	0.00	0.00	0.00
9	Effective Credit Area If (Line 8 >1.5, Line 6, Line 7/1.5]	0	0	0	0	0
10	Sum of Landscape area [sum of Line 9 Id's 1 to 5]				0	sq. ft.
11	Provided footprint for evapotranspiration [Line 5 + Line 10]				0	sq. ft.

Volume Retention Performance Standard

12	Is Line 11 ≥ Line 4?	No, Proceed to Line 13				
13	Fraction of the performance standard met through the BMP footprint and/or landscaping [Line 11/Line 4]				0	
14	Target Volume Retention [Line 10 from Worksheet B.5.2]				255	cu. ft.
15	Volume retention required from other site design BMPs [(1-Line 13) x Line 14]				255.3012251	cu. ft.

Site Design BMP

	Identification	Site Design Type	Credit		
16	1			cu. ft.	
	2			cu. ft.	
	3			cu. ft.	
	4			cu. ft.	
	5			cu. ft.	
	Sum of volume retention benefits from other site design BMPs (e.g. trees; rain barrels etc.). [sum of Line 16 Credits for Id's 1 to 5] Provide documentation of how the site design credit is calculated in the PDP SWQMP.			0	cu. ft.

17 Is Line 16 ≥ Line 15? Implement Additional Site Design BMPs

DMA 1: Compact Biofiltration Design Flows		Worksheet B.6-1		
1	DCV	DCV	11,100	cubic-feet
2	DCV Retained	DCV Retained	0.00	cubic-feet
3	DCV Biofiltered	DCV Biofiltered	0.00	cubic-feet
4	DCV requiring flow-thru (Line 1 - Line 2 - 0.67*Line 3)	DCV flow-thru	11,100	cubic-feet
5	Adjustment Factor (Line 4 / Line1)	AF=	1.00	unitless
6	Design rainfall intensity	i=	0.2	in/hr
7	Area tributary to BMP(s)	A=	7.97	acres
8	Area-weighted runoff factor (estimate using Appendix B.2)	C=	0.72	unitless
9	Calculate Flow Rate = AF x (C x i x A) x1.5	Q=	1.731	cfs
10	Flow Rate per 8x24 unit (High Capacity Model)	Q=	0.870	cfs
11	Number of units required		2	
12	Design Flow Rate (Per Contech)	Q=	1.740	cfs

- Adjustment factor shall be estimated considering only retention and biofiltration BMPs located upstream of Compact Biofiltration BMPs. That is, if the Compact Biofiltration BMP is upstream of the project's retention and biofiltration BMPs then the Compact Biofiltration BMP shall be sized using an adjustment factor of 1.
- 1) Volume based (e.g., dry extended detention basin) Compact Biofiltration treatment control BMPs shall be sized to the volume in Line 4 and flow based (e.g., vegetated swales) shall be sized to flow rate in Line 9. Sand filter and media filter can be designed by either volume in :ie 4 or flow rate in Line 9
 - 2) Proprietary BMPs, if used, shall provide certified treatment capacity equal to or greater than the calculated flow rate in Line 9; certified treatment capacity per unit shall be consistent with third party certifications.
 - 3) Compact Biofiltration treatment control BMPs shall be sized to filter or treat the maximum flow rate of runoff produced from a rainfall intensity of 0.2 inch of rainfall per hour, or each hour of every storm event
 - 4)



Project Name Otay Ranch Village 7, R-8 & R-4

BMP ID BF-3-2

Sizing Method for Volume Retention Criteria

Worksheet B.5-2

1	Area draining to the BMP	124,050	sq. ft.
2	Adjusted runoff factor for drainage area (Refer to Appendix B.1 and B.2)	0.72	
3	85 th percentile 24-hour rainfall depth	0.53	inches
4	Design capture volume [Line 1 x Line 2 x (Line 3/12)]	3967	cu. ft.
Volume Retention Requirement			
5	<p>Measured infiltration rate in the DMA</p> <p>Note:</p> <p>When mapped hydrologic soil groups are used enter 0.10 for NRCS Type D soils and for NRCS Type C soils enter 0.30</p> <p>When in no infiltration condition and the actual measured infiltration rate is unknown enter 0.0 if there are geotechnical and/or groundwater hazards identified in Appendix C or enter 0.05</p>	0	in/hr.
6	Factor of safety	2	
7	Reliable infiltration rate, for biofiltration BMP sizing [Line 5 / Line 6]	0	in/hr.
8	<p>Average annual volume reduction target (Figure B.5-2)</p> <p>When Line 7 > 0.01 in/hr. = Minimum (40, 166.9 x Line 7 + 6.62)</p> <p>When Line 7 ≤ 0.01 in/hr. = 3.5%</p>	3.5	%
9	<p>Fraction of DCV to be retained (Figure B.5-3)</p> <p>When Line 8 > 8% =</p> $0.0000013 \times \text{Line } 8^3 - 0.000057 \times \text{Line } 8^2 + 0.0086 \times \text{Line } 8 - 0.014$ <p>When Line 8 ≤ 8% = 0.023</p>	0.023	
10	Target volume retention [Line 9 x Line 4]	91	cu. ft.



Project Name Otay Ranch Village 7, R-8 & R-4

BMP ID BF-3-2

Volume Retention for No Infiltration Condition

Worksheet B.5-6

1	Area draining to the biofiltration BMP	124,050	sq. ft.
2	Adjusted runoff factor for drainage area (Refer to Appendix B.1 and B.2)	0.72	
3	Effective impervious area draining to the BMP [Line 1 x Line 2]	89812	sq. ft.
4	Required area for Evapotranspiration [Line 3 x 0.03]	2694	sq. ft.
5	Biofiltration BMP Footprint	0	sq. ft.

Landscape Area (must be identified on DS-3247)

Identification		1	2	3	4	5
6	Landscape area that meet the requirements in SD-B and SD-F Fact Sheet (sq. ft.)					
7	Impervious area draining to the landscape area (sq. ft.)					
8	Impervious to Pervious Area ratio [Line 7/Line 6]	0.00	0.00	0.00	0.00	0.00
9	Effective Credit Area If (Line 8 >1.5, Line 6, Line 7/1.5]	0	0	0	0	0
10	Sum of Landscape area [sum of Line 9 Id's 1 to 5]	0			sq. ft.	
11	Provided footprint for evapotranspiration [Line 5 + Line 10]	0			sq. ft.	

Volume Retention Performance Standard

12	Is Line 11 ≥ Line 4?	No, Proceed to Line 13			
13	Fraction of the performance standard met through the BMP footprint and/or landscaping [Line 11/Line 4]	0			
14	Target Volume Retention [Line 10 from Worksheet B.5.2]	91			cu. ft.
15	Volume retention required from other site design BMPs [(1-Line 13) x Line 14]	91.23391761			cu. ft.

Site Design BMP

Identification	Site Design Type	Credit	
16	1	cu. ft.	
	2	cu. ft.	
	3	cu. ft.	
	4	cu. ft.	
	5	cu. ft.	
Sum of volume retention benefits from other site design BMPs (e.g. trees; rain barrels etc.). [sum of Line 16 Credits for Id's 1 to 5] Provide documentation of how the site design credit is calculated in the PDP SWQMP.		0	cu. ft.

17 Is Line 16 ≥ Line 15? Implement Additional Site Design BMPs

DMA 2: Compact Biofiltration Design Flows		Worksheet B.6-1		
1	DCV	DCV	5,950	cubic-feet
2	DCV Retained	DCV Retained	0.00	cubic-feet
3	DCV Biofiltered	DCV Biofiltered	0.00	cubic-feet
4	DCV requiring flow-thru (Line 1 - Line 2 - 0.67*Line 3)	DCV flow-thru	5,950	cubic-feet
5	Adjustment Factor (Line 4 / Line1)	AF=	1.00	unitless
6	Design rainfall intensity	i=	0.2	in/hr
7	Area tributary to BMP(s)	A=	2.85	acres
8	Area-weighted runoff factor (estimate using Appendix B.2)	C=	0.72	unitless
9	Calculate Flow Rate = AF x (C x i x A) x1.5	Q=	0.619	cfs
10	Flow Rate per 8x24 unit	Q=	0.693	cfs
11	Number of units required		1	
12	Design Flow Rate (Per Contech)	Q=	0.693	cfs

- Adjustment factor shall be estimated considering only retention and biofiltration BMPs located upstream of Compact Biofiltration BMPs. That is, if the Compact Biofiltration BMP is upstream of the project's retention and biofiltration BMPs then the Compact Biofiltration BMP shall be sized using an adjustment factor of 1.
- 1) Volume based (e.g., dry extended detention basin) Compact Biofiltration treatment control BMPs shall be sized to the volume in Line 4 and flow based (e.g., vegetated swales) shall be sized to flow rate in Line 9. Sand filter and media filter can be designed by either volume in :ie 4 or flow rate in Line 9
 - 2) Proprietary BMPs, if used, shall provide certified treatment capacity equal to or greater than the calculated flow rate in Line 9; certified treatment capacity per unit shall be consistent with third party certifications.
 - 3) Compact Biofiltration treatment control BMPs shall be sized to filter or treat the maximum flow rate of runoff produced from a rainfall intensity of 0.2 inch of rainfall per hour, or each hour of every storm event
 - 4)

R-4 unit

R-8 unit will be deeper

MWS LINEAR 2.0 HGL SIZING CALCULATIONS



HGL HEIGHT

MWS MODEL SIZE	WETLAND PERIMETER LENGTH	LOADING RATE GPM/SF	SHALLOW MODELS																				STANDARD HEIGHT MODEL	HIGH CAPACITY MODELS								
			1.4	1.5	1.6	1.7	1.8	1.9	2.0	2.1	2.2	2.3	2.4	2.5	2.6	2.7	2.8	2.9	3.0	3.1	3.2	3.3	3.4	3.5	3.6	3.65	3.70	3.75	3.80	3.85	3.90	3.95
MWS-L-4-4	6.70	1.0	0.022	0.023	0.025	0.026	0.028	0.029	0.031	0.032	0.034	0.035	0.037	0.038	0.040	0.042	0.043	0.045	0.046	0.048	0.049	0.051	0.052	0.054	0.055	0.056	0.057	0.058	0.058	0.059	0.060	0.061
MWS-L-3-6	10.06	1.0	0.032	0.035	0.037	0.039	0.042	0.044	0.046	0.048	0.051	0.053	0.055	0.058	0.060	0.062	0.065	0.067	0.069	0.072	0.074	0.076	0.078	0.081	0.083	0.084	0.085	0.087	0.088	0.089	0.090	0.091
MWS-L-4-6	9.30	1.0	0.030	0.032	0.034	0.036	0.038	0.041	0.043	0.045	0.047	0.049	0.051	0.053	0.055	0.058	0.060	0.062	0.064	0.066	0.068	0.070	0.073	0.075	0.077	0.078	0.079	0.080	0.081	0.082	0.083	0.084
MWS-L-4-8	14.80	1.0	0.048	0.051	0.054	0.058	0.061	0.065	0.068	0.071	0.075	0.078	0.082	0.085	0.088	0.092	0.095	0.099	0.102	0.105	0.109	0.112	0.115	0.119	0.122	0.124	0.126	0.127	0.129	0.131	0.132	0.134
MWS-L-4-13	18.40	1.0	0.059	0.063	0.068	0.072	0.076	0.080	0.084	0.089	0.093	0.097	0.101	0.106	0.110	0.114	0.118	0.122	0.127	0.131	0.135	0.139	0.144	0.148	0.152	0.154	0.156	0.158	0.160	0.163	0.165	0.167
MWS-L-4-15	22.40	1.0	0.072	0.077	0.082	0.087	0.093	0.098	0.103	0.108	0.113	0.118	0.123	0.129	0.134	0.139	0.144	0.149	0.154	0.159	0.165	0.170	0.175	0.180	0.185	0.188	0.190	0.193	0.195	0.198	0.200	0.203
MWS-L-4-17	26.40	1.0	0.085	0.091	0.097	0.103	0.109	0.115	0.121	0.127	0.133	0.139	0.145	0.151	0.158	0.164	0.170	0.176	0.182	0.188	0.194	0.200	0.206	0.212	0.218	0.221	0.224	0.227	0.230	0.233	0.236	0.239
MWS-L-4-19	30.40	1.0	0.098	0.105	0.112	0.119	0.126	0.133	0.140	0.147	0.153	0.160	0.167	0.174	0.181	0.188	0.195	0.202	0.209	0.216	0.223	0.230	0.237	0.244	0.251	0.255	0.258	0.262	0.265	0.269	0.272	0.276
MWS-L-4-21	34.40	1.0	0.111	0.118	0.126	0.134	0.142	0.150	0.158	0.166	0.174	0.182	0.189	0.197	0.205	0.213	0.221	0.229	0.237	0.245	0.253	0.261	0.268	0.276	0.284	0.288	0.292	0.296	0.300	0.304	0.308	0.312
MWS-L-6-8	18.80	1.0	0.060	0.065	0.069	0.073	0.078	0.082	0.086	0.091	0.095	0.099	0.104	0.108	0.112	0.116	0.121	0.125	0.129	0.134	0.138	0.142	0.147	0.151	0.155	0.157	0.160	0.162	0.164	0.166	0.168	0.170
MWS-L-8-8	29.60	1.0	0.095	0.102	0.109	0.115	0.122	0.129	0.136	0.143	0.149	0.156	0.163	0.170	0.177	0.183	0.190	0.197	0.204	0.211	0.217	0.224	0.231	0.238	0.245	0.248	0.251	0.255	0.258	0.262	0.265	0.268
MWS-L-8-12	44.40	1.0	0.143	0.153	0.163	0.173	0.183	0.194	0.204	0.214	0.224	0.234	0.245	0.255	0.265	0.275	0.285	0.296	0.306	0.316	0.326	0.336	0.346	0.357	0.367	0.372	0.377	0.382	0.387	0.392	0.397	0.402
MWS-L-8-16	59.20	1.0	0.190	0.204	0.217	0.231	0.245	0.258	0.272	0.285	0.299	0.312	0.326	0.340	0.353	0.367	0.380	0.394	0.408	0.421	0.435	0.448	0.462	0.476	0.489	0.496	0.503	0.509	0.516	0.523	0.530	0.537
MWS-L-8-20	74.00	1.0	0.238	0.255	0.272	0.289	0.306	0.323	0.340	0.357	0.374	0.391	0.408	0.425	0.442	0.459	0.476	0.493	0.509	0.526	0.543	0.560	0.577	0.594	0.611	0.620	0.628	0.637	0.645	0.654	0.662	0.671
MWS-L-10-20 or MWS-L-8-24	88.80	1.0	0.285	0.306	0.326	0.346	0.367	0.387	0.408	0.428	0.448	0.469	0.489	0.509	0.530	0.550	0.571	0.591	0.611	0.632	0.652	0.673	0.693	0.713	0.734	0.744	0.754	0.764	0.774	0.785	0.795	0.805
4x4 media cage	14.80	1.0	0.048	0.051	0.054	0.058	0.061	0.065	0.068	0.071	0.075	0.078	0.082	0.085	0.088	0.092	0.095	0.099	0.102	0.105	0.109	0.112	0.115	0.119	0.122	0.124						

Compact (high rate) Biofiltration BMP Checklist		Form I-10
<p>Compact (high rate) biofiltration BMPs have a media filtration rate greater than 5 in/hr. and a media surface area smaller than 3% of contributing area times adjusted runoff factor. Compact biofiltration BMPs are typically proprietary BMPs that may qualify as biofiltration.</p> <p>A compact biofiltration BMP may satisfy the pollutant control requirements for a DMA onsite in some cases. This depends on the characteristics of the DMA and the performance certification/data of the BMP. If the pollutant control requirements for a DMA are met onsite, then the DMA is not required to participate in an offsite storm water alternative compliance program to meet its</p> <p>An applicant using a compact biofiltration BMP to meet the pollutant control requirements onsite must complete Section 1 of this form and include it in the PDP SWQMP. A separate form must be completed for each DMA. In instances where the City Engineer does not agree with the applicant's determination, Section 2 of this form will be completed by the City and returned to the applicant.</p>		
Section 1: Biofiltration Criteria Checklist (Appendix F)		
<p>Refer to Part 1 of the Storm Water Standards to complete this section. When separate forms/worksheets are referenced below, the applicant must also complete these separate forms/worksheets (as applicable) and include in the PDP SWQMP. The criteria numbers below correspond to the criteria numbers in Appendix F.</p>		
Criteria	Answer	Progression
<p>Criteria 1 and 3:</p> <p>What is the infiltration condition of the DMA?</p> <p>Refer to Section 5.4.2 and Appendix C of the BMP Design Manual (Part 1 of Storm Water Standards) for guidance.</p> <p>Applicant must complete and include the following in the PDP SWQMP submittal to support the feasibility determination:</p> <ul style="list-style-type: none"> Infiltration Feasibility Condition Letter; or Worksheet C.4-1: Form I-8A and Worksheet C.4-2: Form I-8B. <p>Applicant must complete and include all applicable sizing worksheets in the SWQMP submittal</p>	<input type="radio"/> Full Infiltration Condition	<p>Stop. Compact biofiltration BMP is not allowed.</p>
	<input type="radio"/> Partial Infiltration Condition	<p>Compact biofiltration BMP is only allowed, if the target volume retention is met onsite (Refer to Table B.5-1 in Appendix B.5). Use Worksheet B.5-2 in Appendix B.5 to estimate the target volume retention (Note: retention in this context means reduction).</p> <p>If the required volume reduction is achieved proceed to Criteria 2.</p> <p>If the required volume reduction is not achieved, compact biofiltration BMP is not allowed. Stop.</p>
	<input checked="" type="radio"/> No Infiltration Condition	<p>Compact biofiltration BMP is allowed if volume retention criteria in Table B.5-1 in Appendix B.5 for the no infiltration condition is met. Compliance with this criterion must be documented in the PDP SWQMP.</p> <p>If the criteria in Table B.5-1 is met proceed to Criteria 2.</p> <p>If the criteria in Table B.5-1 is not met, compact biofiltration BMP is not allowed. Stop.</p>



Provide basis for Criteria 1 and 3:

Feasibility Analysis:

Summarize findings and include either infiltration feasibility condition letter or Worksheet C.4-1: Form I-8A and Worksheet C.4-2: Form I-8B in the PDP SWQMP submittal.

If Partial Infiltration Condition:

Provide documentation that target volume retention is met (include Worksheet B.5-2 in the PDP SWQMP submittal). Worksheet B.5-7 in Appendix B.5 can be used to estimate volume retention benefits from landscape areas.

If No Infiltration Condition:

Provide documentation that the volume retention performance standard is met (include Worksheet B.5-2 in the PDP SWQMP submittal) in the PDP SWQMP submittal. Worksheet B.5-6 in Appendix B.5 can be used to document that the performance standard is met.

Criteria	Answer	Progression
<p><u>Criteria 2:</u></p> <p>Is the compact biofiltration BMP sized to meet the performance standard from the MS4 Permit?</p> <p>Refer to Appendix B.5 and Appendix F.2 of the BMP Design Manual (Part 1 of Storm Water Standards) for guidance.</p>	<input checked="" type="radio"/> Meets Flow based Criteria	<p>Use guidance from Appendix F.2.2 to size the compact biofiltration BMP to meet the flow based criteria. Include the calculations in the PDP SWQMP.</p> <p>Use parameters for sizing consistent with manufacturer guidelines and conditions of its third party certifications (i.e. a BMP certified at a loading rate of 1 gpm/sq. ft. cannot be designed using a loading rate of 1.5 gpm/sq. ft.)</p> <p>Proceed to Criteria 4.</p>
	<input type="radio"/> Meets Volume based Criteria	<p>Provide documentation that the compact biofiltration BMP has a total static (i.e. non-routed) storage volume, including pore-spaces and pre-filter detention volume (Refer to Appendix B.5 for a schematic) of at least 0.75 times the portion of the DCV not reliably retained onsite.</p> <p>Proceed to Criteria 4.</p>
	<input type="radio"/> Does not Meet either criteria	<p>Stop. Compact biofiltration BMP is not allowed.</p>



Provide basis for Criteria 2:

Provide documentation that the BMP meets the numeric criteria and is designed consistent with the manufacturer guidelines and conditions of its third-party certification (i.e., loading rate, etc., as applicable).

Criteria	Answer	Progression
<p>Criteria 4:</p> <p>Does the compact biofiltration BMP meet the pollutant treatment performance standard for the projects most significant pollutants of concern?</p> <p>Refer to Appendix B.6 and Appendix F.1 of the BMP Design Manual (Part 1 of Storm Water Standards) for guidance.</p>	<input checked="" type="radio"/> Yes, meets the TAPE certification.	<p>Provide documentation that the compact BMP has an appropriate TAPE certification for the projects most significant pollutants of concern.</p> <p>Proceed to Criteria 5.</p>
	<input type="radio"/> Yes, through other third-party documentation	<p>Acceptance of third-party documentation is at the discretion of the City Engineer. The City engineer will consider, (a) the data submitted; (b) representativeness of the data submitted; and (c) consistency of the BMP performance claims with pollutant control objectives in Table F.1-2 and Table F.1-1 while making this determination. If a compact biofiltration BMP is not accepted, a written explanation/ reason will be provided in Section 2.</p> <p>Proceed to Criteria 5.</p>
	<input type="radio"/> No	<p>Stop. Compact biofiltration BMP is not allowed.</p>

Provide basis for Criteria 4:

Provide documentation that identifies the projects most significant pollutants of concern and TAPE certification or other third party documentation that shows that the compact biofiltration BMP meets the pollutant treatment performance standard for the projects most significant pollutants of concern.



Compact (high rate) Biofiltration BMP Checklist		Form I-10
Criteria	Answer	Progression
Criteria 5: Is the compact biofiltration BMP designed to promote appropriate biological activity to support and maintain treatment process? Refer to Appendix F of the BMP Design Manual (Part 1 of Storm Water Standards) for guidance.	<input checked="" type="radio"/> Yes	Provide documentation that the compact biofiltration BMP support appropriate biological activity. Refer to Appendix F for guidance. Proceed to Criteria 6.
	<input type="radio"/> No	Stop. Compact biofiltration BMP is not allowed.
Provide basis for Criteria 5: Provide documentation that appropriate biological activity is supported by the compact biofiltration BMP to maintain treatment process.		
Criteria	Answer	Progression
Criteria 6: Is the compact biofiltration BMP designed with a hydraulic loading rate to prevent erosion, scour and channeling within the BMP?	<input checked="" type="radio"/> Yes	Provide documentation that the compact biofiltration BMP is used in a manner consistent with manufacturer guidelines and conditions of its third-party certification. Proceed to Criteria 7.
	<input type="radio"/> No	Stop. Compact biofiltration BMP is not allowed.
Provide basis for Criteria 6: Provide documentation that the BMP meets the numeric criteria and is designed consistent with the manufacturer guidelines and conditions of its third-party certification (i.e., maximum tributary area, maximum inflow velocities, etc., as applicable).		



Compact (high rate) Biofiltration BMP Checklist		Form I-10
Criteria	Answer	Progression
<p>Criteria 7: Is the compact biofiltration BMP maintenance plan consistent with manufacturer guidelines and conditions of its third-party certification (i.e., maintenance activities, frequencies)?</p>	<input checked="" type="radio"/> Yes, and the compact BMP is privately owned, operated and not in the public right of way.	<p>Submit a maintenance agreement that will also include a statement that the BMP will be maintained in accordance with manufacturer guidelines and conditions of third-party certification.</p> <p>Stop. The compact biofiltration BMP meets the required criteria.</p>
	<input type="radio"/> Yes, and the BMP is either owned or operated by the City or in the public right of way.	<p>Approval is at the discretion of the City Engineer. The city engineer will consider maintenance requirements, cost of maintenance activities, relevant previous local experience with operation and maintenance of the BMP type, ability to continue to operate the system in event that the vending company is no longer operating as a business or other relevant factors while making the determination.</p> <p>Stop. Consult the City Engineer for a determination.</p>
	<input type="radio"/> No	<p>Stop. Compact biofiltration BMP is not allowed.</p>
<p>Provide basis for Criteria 7:</p> <p>Include copy of manufacturer guidelines and conditions of third-party certification in the maintenance agreement. PDP SWQMP must include a statement that the compact BMP will be maintained in accordance with manufacturer guidelines and conditions of third-party certification.</p>		



Compact (high rate) Biofiltration BMP Checklist		Form I-10
Section 2: Verification (For City Use Only)		
Is the proposed compact BMP accepted by the City Engineer for onsite pollutant control compliance for the DMA?	<input type="radio"/> Yes <input type="radio"/> No, See explanation below	
Explanation/reason if the compact BMP is not accepted by the City for onsite pollutant control compliance:		





August 2021

**GENERAL USE LEVEL DESIGNATION FOR BASIC (TSS)
ENHANCED AND PHOSPHORUS TREATMENT**

For

MWS-Linear Modular Wetland

Ecology's Decision

Based on Modular Wetland Systems, Inc, application submissions, including the Technical Evaluation Report, dated April 1, 2014, Ecology hereby issues the following use level designation:

1. General Use Level Designation (GULD) for the MWS-Linear Modular Wetland Stormwater Treatment System for Basic, Phosphorus, and Enhanced treatment
 - Sized at a hydraulic loading rate of:
 - 1 gallon per minute (gpm) per square foot (sq ft) of Wetland Cell Surface Area
 - Prefilter box (approved at either 22 inches or 33 inches tall)
 - 3.0 gpm/sq ft of prefilter box surface area for moderate pollutant loading rates (low to medium density residential basins).
 - 2.1 gpm/sq ft of prefilter box surface area for high pollutant loading rates (commercial and industrial basins).
2. Ecology approves the MWS – Linear Modular Wetland Stormwater Treatment System units for Basic, Phosphorus, and Enhanced treatment at the hydraulic loading rate listed above. Designers shall calculate the water quality design flow rates using the following procedures:
 - Western Washington: For treatment installed upstream of detention or retention, the water quality design flow rate is the peak 15-minute water quality treatment design flow rate as calculated using the latest version of the Western Washington Hydrology Model or other Ecology- approved continuous runoff model.

- Eastern Washington: For treatment installed upstream of detention or retention, the water quality design flow rate is the peak 15-minute water quality treatment design flow rate as calculated using one of the three methods described in Chapter 2.2.5 of the Stormwater Management Manual for Eastern Washington (SWMMEW) or local manual.
 - Entire State: For treatment installed downstream of detention, the water quality treatment design flow rate is the full 2-year release rate of the detention facility.
3. These use level designations have no expiration date but may be amended or revoked by Ecology, and are subject to the conditions specified below.

Ecology's Conditions of Use

Applicants shall comply with the following conditions:

- 1) Design, assemble, install, operate, and maintain the MWS – Linear Modular Wetland Stormwater Treatment System units, in accordance with Modular Wetland Systems, Inc. applicable manuals and documents and the Ecology Decision.
- 2) Each site plan must undergo Modular Wetland Systems, Inc. review and approval before site installation. This ensures that site grading and slope are appropriate for use of a MWS – Linear Modular Wetland Stormwater Treatment System unit.
- 3) MSW – Linear Modular Wetland Stormwater Treatment System media shall conform to the specifications submitted to and approved by Ecology.
- 4) The applicant tested the MWS – Linear Modular Wetland Stormwater Treatment System with an external bypass weir. This weir limited the depth of water flowing through the media, and therefore the active treatment area, to below the root zone of the plants. This GULD applies to MWS – Linear Modular Wetland Stormwater Treatment Systems whether plants are included in the final product or not.
- 5) Maintenance: The required maintenance interval for stormwater treatment devices is often dependent upon the degree of pollutant loading from a particular drainage basin. Therefore, Ecology does not endorse or recommend a “one size fits all” maintenance cycle for a particular model/size of stormwater treatment technology.
 - Typically, Modular Wetland Systems, Inc. designs MWS – Linear Modular Wetland systems for a target prefilter media life of 6 to 12 months.
 - Indications of the need for maintenance include effluent flow decreasing to below the design flow rate or decrease in treatment below required levels.
 - Owners/operators must inspect MWS – Linear Modular Wetland systems for a minimum of twelve months from the start of post-construction operation to determine site-specific maintenance schedules and requirements. You must conduct inspections monthly during the wet season, and every other month during the dry season (According to the SWMMWW, the wet season in western Washington is October 1 to April

30. According to the SWMMEW, the wet season in eastern Washington is October 1 to June 30). After the first year of operation, owners/operators must conduct inspections based on the findings during the first year of inspections.

- Conduct inspections by qualified personnel, follow manufacturer's guidelines, and use methods capable fo determining either a decrease in treated effluent flowrate and/or a decrease in pollutant removal ability.
- When inspections are performed, the following findings typically serve as maintenance triggers:
 - Standing water remains in the vault between rain events, or
 - Bypass occurs during storms smaller than the design storm.
 - If excessive floatables (trash and debris) are present (but no standing water or excessive sedimentation), perform a minor maintenance consisting of gross solids removal, not prefilter media replacement.
 - Additional data collection will be used to create a correlation between pretreatment chamber sediment depth and pre-filter clogging (see *Issues to be Addressed by the Company* section below)

6) Discharges from the MWS – Linear Modular Wetland Stormwater Treatment System units shall not cause or contribute to water quality standards violations in receiving waters.

Applicant: Modular Wetland Systems, Inc.

Applicant's Address: 5796 Armada Drive, Suite 250
Carlsbad, CA 92008

Application Documents:

Original Application for Conditional Use Level Designation, Modular Wetland System, Linear Stormwater Filtration System Modular Wetland Systems, Inc., January 2011

Quality Assurance Project Plan: Modular Wetland System – Linear Treatment System Performance Monitoring Project, draft, January 2011

Revised Application for Conditional Use Level Designation, Modular Wetland System, Linear Stormwater Filtration System Modular Wetland Systems, Inc., May 2011

Memorandum: Modular Wetland System-Linear GULD Application Supplementary Data, April 2014

Applicant's Use Level Request:

- General Use Level Designation as a Basic, Enhanced, and Phosphorus treatment device in accordance with Ecology's Guidance for Evaluating Emerging Stormwater Treatment Technologies Technology Assessment Protocol – Ecology (TAPE) January 2011 Revision.

Applicant's Performance Claims:

- The MWS – Linear Modular wetland is capable of removing a minimum of 80-percent of TSS from stormwater with influent concentrations between 100 and 200 mg/L.
- The MWS – Linear Modular wetland is capable of removing a minimum of 50-percent of total phosphorus from stormwater with influent concentrations between 0.1 and 0.5 mg/L.
- The MWS – Linear Modular wetland is capable of removing a minimum 30-percent of dissolved copper from stormwater with influent concentrations between 0.005 and 0.020 mg/L.
- The MWS – Linear Modular wetland is capable of removing a minimum 60-percent of dissolved zinc from stormwater with influent concentrations between 0.02 and 0.30 mg/L.

Ecology's Recommendations:

- Modular Wetland System, Inc. has shown Ecology, through laboratory and field-testing, that the MWS – Linear Modular Wetland Stormwater Treatment System filter system is capable of attaining Ecology's Basic, Phosphorus, and Enhanced treatment goals.

Findings of Fact:

Laboratory Testing

The MWS-Linear Modular wetland has the:

- Capability to remove 99 percent of total suspended solids (using Sil-Co-Sil 106) in a quarter-scale model with influent concentrations of 270 mg/L.
- Capability to remove 91 percent of total suspended solids (using Sil-Co-Sil 106) in laboratory conditions with influent concentrations of 84.6 mg/L at a flow rate of 3.0 gpm per square foot of media.
- Capability to remove 93 percent of dissolved Copper in a quarter-scale model with influent concentrations of 0.757 mg/L.
- Capability to remove 79 percent of dissolved Copper in laboratory conditions with influent concentrations of 0.567 mg/L at a flow rate of 3.0 gpm per square foot of media.

- Capability to remove 80.5-percent of dissolved Zinc in a quarter-scale model with influent concentrations of 0.95 mg/L at a flow rate of 3.0 gpm per square foot of media.
- Capability to remove 78-percent of dissolved Zinc in laboratory conditions with influent concentrations of 0.75 mg/L at a flow rate of 3.0 gpm per square foot of media.

Field Testing

- Modular Wetland Systems, Inc. conducted monitoring of an MWS-Linear (Model # MWS-L-4-13) from April 2012 through May 2013, at a transportation maintenance facility in Portland, Oregon. The manufacturer collected flow-weighted composite samples of the system's influent and effluent during 28 separate storm events. The system treated approximately 75 percent of the runoff from 53.5 inches of rainfall during the monitoring period. The applicant sized the system at 1 gpm/sq ft. (wetland media) and 3gpm/sq ft. (prefilter).
- Influent TSS concentrations for qualifying sampled storm events ranged from 20 to 339 mg/L. Average TSS removal for influent concentrations greater than 100 mg/L (n=7) averaged 85 percent. For influent concentrations in the range of 20-100 mg/L (n=18), the upper 95 percent confidence interval about the mean effluent concentration was 12.8 mg/L.
- Total phosphorus removal for 17 events with influent TP concentrations in the range of 0.1 to 0.5 mg/L averaged 65 percent. A bootstrap estimate of the lower 95 percent confidence limit (LCL95) of the mean total phosphorus reduction was 58 percent.
- The lower 95 percent confidence limit of the mean percent removal was 60.5 percent for dissolved zinc for influent concentrations in the range of 0.02 to 0.3 mg/L (n=11). The lower 95 percent confidence limit of the mean percent removal was 32.5 percent for dissolved copper for influent concentrations in the range of 0.005 to 0.02 mg/L (n=14) at flow rates up to 28 gpm (design flow rate 41 gpm). Laboratory test data augmented the data set, showing dissolved copper removal at the design flow rate of 41 gpm (93 percent reduction in influent dissolved copper of 0.757 mg/L).

Issues to be addressed by the Company:

1. Modular Wetland Systems, Inc. should collect maintenance and inspection data for the first year on all installations in the Northwest in order to assess standard maintenance requirements for various land uses in the region. Modular Wetland Systems, Inc. should use these data to establish required maintenance cycles.
2. Modular Wetland Systems, Inc. should collect pre-treatment chamber sediment depth data for the first year of operation for all installations in the Northwest. Modular Wetland Systems, Inc. will use these data to create a correlation between sediment depth and pre-filter clogging.

Technology Description:

Download at <http://www.modularwetlands.com/>

Contact Information:

Applicant: Zach Kent
 BioClean A Forterra Company
 5796 Armada Drive, Suite 250
 Carlsbad, CA 92008
 zach.kent@forterrabp.com

Applicant website: <http://www.modularwetlands.com/>

Ecology web link: <http://www.ecy.wa.gov/programs/wg/stormwater/newtech/index.html>

Ecology: Douglas C. Howie,
 P.E. Department of
 Ecology Water
 Quality Program
 (360) 870-0983
douglas.howie@ecy.wa.gov

Revision History

Date	Revision
June 2011	Original use-level-designation document
September 2012	Revised dates for TER and expiration
January 2013	Modified Design Storm Description, added Revision Table, added maintenance discussion, modified format in accordance with Ecology standard
December 2013	Updated name of Applicant
April 2014	Approved GULD designation for Basic, Phosphorus, and Enhanced treatment
December 2015	Updated GULD to document the acceptance of MWS – Linear Modular Wetland installations with or without the inclusion of plants
July 2017	Revised Manufacturer Contact Information (name, address, and email)
December 2019	Revised Manufacturer Contact Address
July 2021	Added additional prefilter sized at 33 inches
August 2021	Changed “Prefilter” to “Prefilter box”

ATTACHMENT 2

Backup for PDP Hydromodification Control Measures

- Mark this box if this attachment is empty because the project is exempt from PDP hydromodification management requirements.

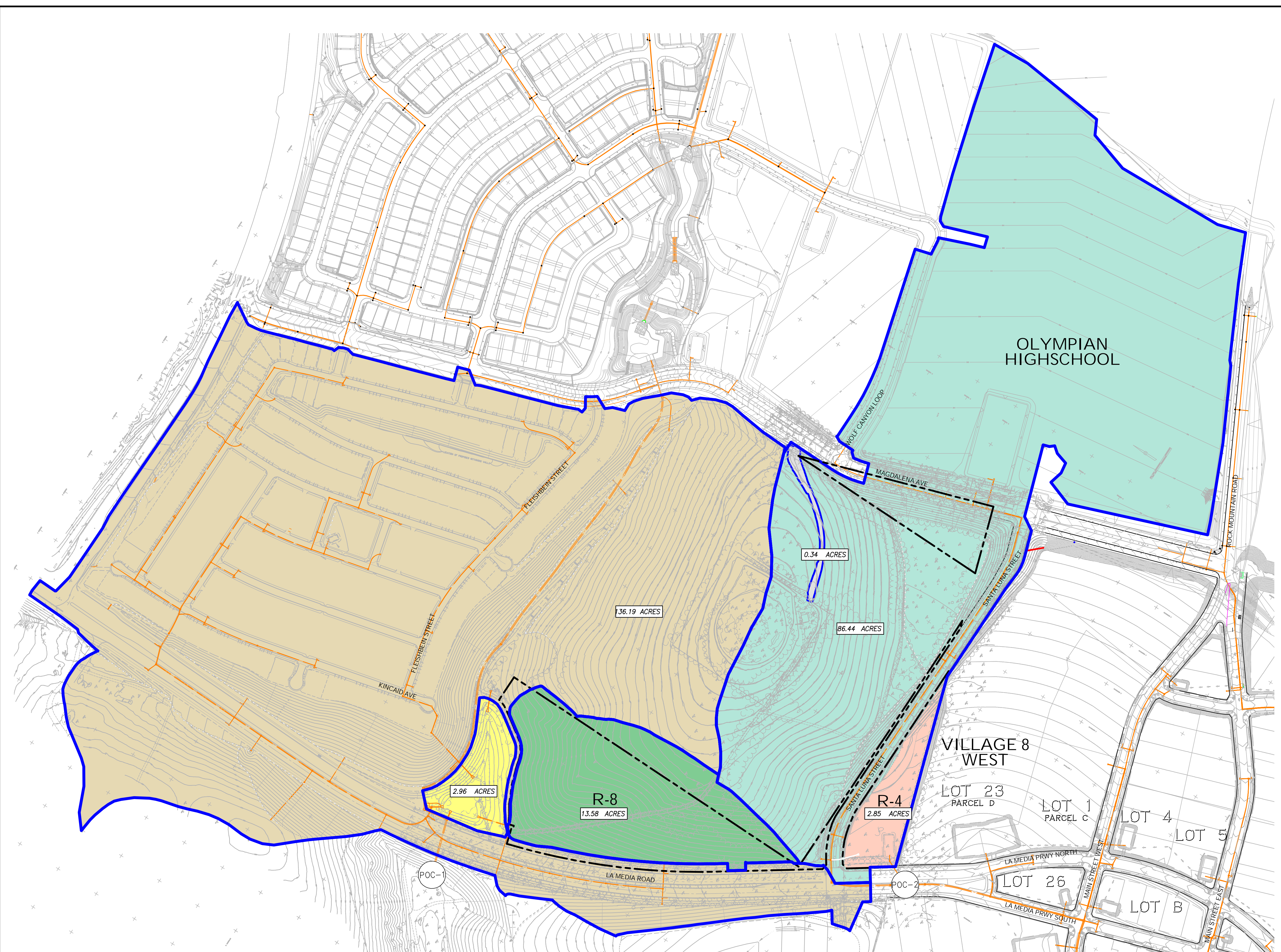
Indicate which Items are Included

Attachment Sequence	Contents	Checklist
Attachment 2A	Hydromodification Management Exhibit (Required)	<input checked="" type="checkbox"/> Included See Hydromodification Management Exhibit Checklist.
Attachment 2B	Management of Critical Coarse Sediment Yield Areas (WMAA Exhibit is required, additional analyses are optional) See Section 6.2 of the BMP Design Manual.	<input checked="" type="checkbox"/> Exhibit showing project drainage boundaries marked on WMAA Critical Coarse Sediment Yield Area Map (Required) Optional analyses for Critical Coarse Sediment Yield Area Determination <ul style="list-style-type: none"> <input type="checkbox"/> 6.2.1 Verification of Geomorphic Landscape Units Onsite <input type="checkbox"/> 6.2.2 Downstream Systems Sensitivity to Coarse Sediment <input type="checkbox"/> 6.2.3 Optional Additional Analysis of Potential Critical Coarse Sediment Yield Areas Onsite
Attachment 2C	Geomorphic Assessment of Receiving Channels (Optional) See Section 6.3.4 of the BMP Design Manual.	<input checked="" type="checkbox"/> Not performed <input type="checkbox"/> Included <input type="checkbox"/> Submitted as separate stand-alone document
Attachment 2D	Flow Control Facility Design and Structural BMP Drawdown Calculations (Required) Overflow Design Summary for each Structural BMP See Chapter 6 and Appendix G of the BMP Design Manual	<input checked="" type="checkbox"/> Included <input type="checkbox"/> Submitted as separate stand-alone document

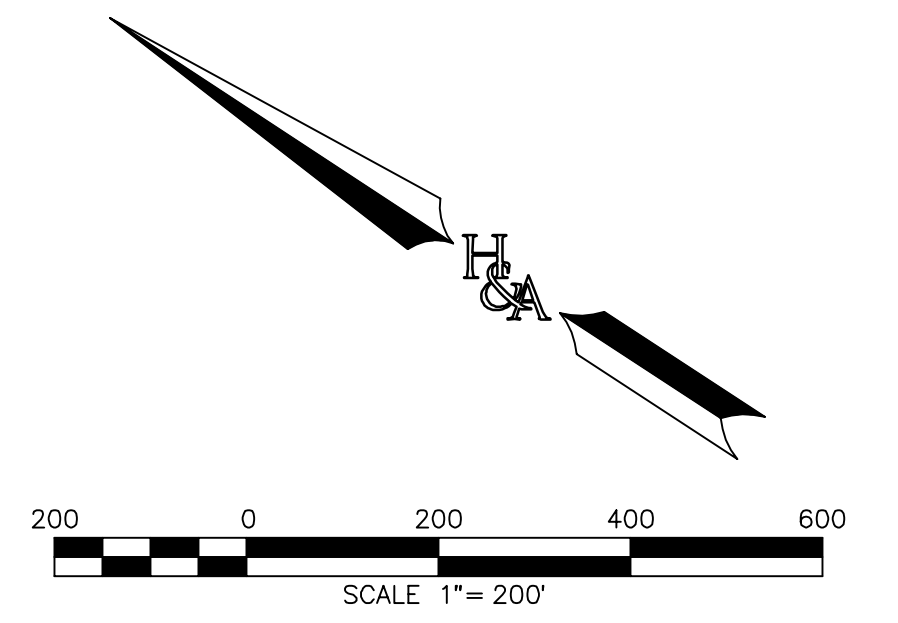
Use this checklist to ensure the required information has been included on the Hydromodification Management Exhibit:


The Hydromodification Management Exhibit must identify:

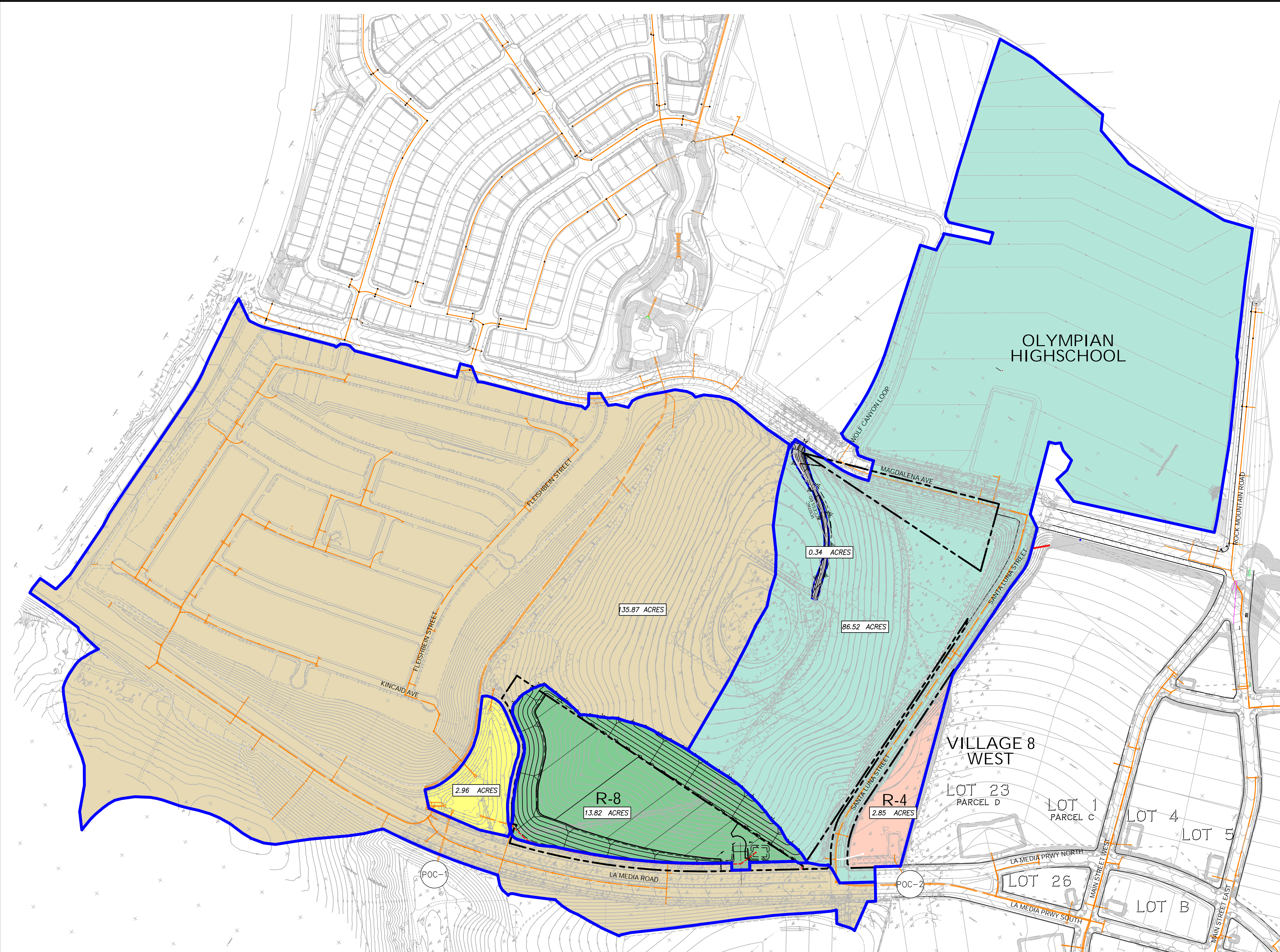
- Underlying hydrologic soil group
- Approximate depth to groundwater
- Existing natural hydrologic features (watercourses, seeps, springs, wetlands)
- Critical coarse sediment yield areas to be protected
- Existing topography
- Existing and proposed site drainage network and connections to drainage offsite
- Proposed grading
- Proposed impervious features
- Proposed design features and surface treatments used to minimize imperviousness
- Point(s) of Compliance (POC) for Hydromodification Management Hydromodification Management, with a POC at each point of discharge
- Existing and proposed drainage boundary and drainage area to each POC (when necessary, create separate exhibits for pre-development and post-project conditions)
- Structural BMPs for hydromodification management (identify location, type of BMP, cross-section and size/detail)



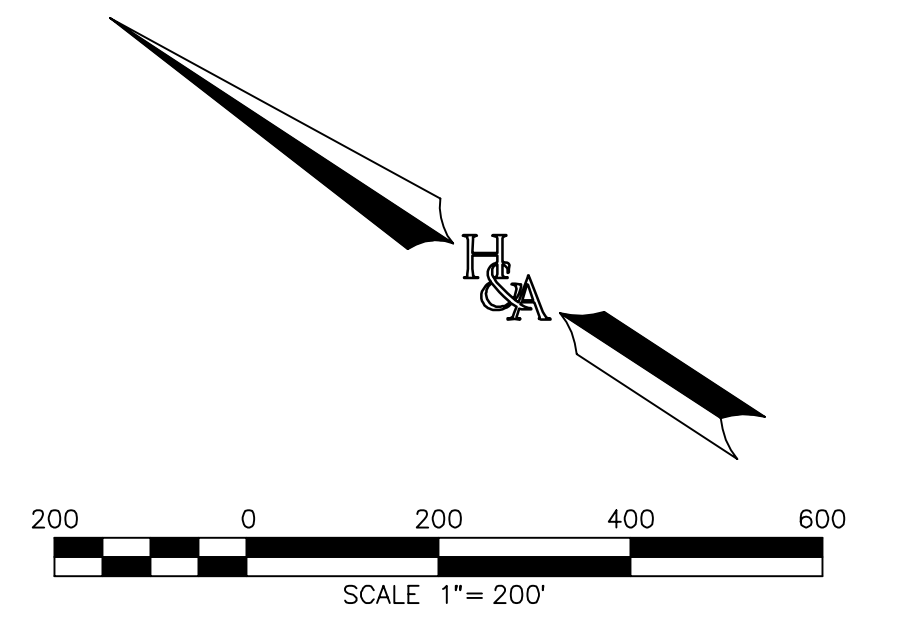
LEGEND:	SYMBOL:
PROJECT BOUNDARY.....	--- --
DMA BOUNDARY.....	— — — —
PROPOSED STORM DRAIN.....	— (S) —
EXISTING STORM DRAIN.....	— (E) —
SUBAREA ACREAGE.....	00.00 ACRES
DMA ICON.....	DMA 1
AREA TRIBUTARY TO POC 1.....	[Light Blue Box]
OTAY RANCH VILLAGE 7 R-8.....	[Light Green Box]
AREA TRIBUTARY TO POC 2.....	[Light Green Box]
VORTAC SITE ACCESS ROAD.....	[Grey Box]
OTAY RANCH VILLAGE 7 R-4.....	[Light Blue Box]
EXISTING DETENTION BASIN.....	[Yellow Box]
HYDROLOGIC SOIL TYPE.....	[D in Circle]
POINT OF COMPLIANCE.....	[X in Circle]



PREPARED BY:  HUNSAKER & ASSOCIATES SAN DIEGO, INC. PLANNING 9707 Waples Street ENGINEERING San Diego, Ca 92121 SURVEYING PH(658)558-4500 FX(658)558-1414	EXISTING CONDIION HYDROMODIFICATION MAP VILLAGE 7, R-3, R-4, & R-8 CITY OF CHULA VISTA, CALIFORNIA	MAP
		1
		OF
		2



LEGEND:	SYMBOL:
PROJECT BOUNDARY.....	--- --
DMA BOUNDARY.....	— — — —
PROPOSED STORM DRAIN.....	— (S) —
EXISTING STORM DRAIN.....	— (S) —
SUBAREA ACREAGE.....	00.00 ACRES
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AREA TRIBUTARY TO POC 1.....	[Light Green Box]
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AREA TRIBUTARY TO POC 2.....	[Light Yellow Box]
VORTAC SITE ACCESS ROAD.....	[Grey Box]
OTAY RANCH VILLAGE 7 R-4.....	[Light Orange Box]
EXISTING DETENTION BASIN.....	[Yellow Box]
HYDROLOGIC SOIL TYPE.....	(D)
POINT OF COMPLIANCE.....	(X)



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**DEVELOPED CONDIIION
 HYDROMODIFICATION MAP
 VILLAGE 7, R-3, R-4, & R-8**
 CITY OF CHULA VISTA, CALIFORNIA

MAP
 2
 OF
 2
 M.O.F. 8909-1336



ATTACHMENT 2a
HYDROMODIFICATION MANAGEMENT EXHIBITS

ATTACHMENT 2b
MANAGEMENT OF CRITICAL COARSE SEDIMENT YIELD AREAS

Otay Ranch Village 7 TM Conceptual Grading

Potential Critical Course Sidement Yield Areas Exhibit

Legend

-  1601 Otay Village 7 South
-  PCCSYA



ATTACHMENT 2c
GEOMORPHIC ASSESSMENT OF RECEIVING CHANNELS

NOT PERFORMED FOR THIS PROJECT

ATTACHMENT 2d
FLOW CONTROL FACILITY DESIGN

Hydromodification Management Plan (HMP) -Flow Control Facility Design

This HMP report has been prepared in place of Attachment 2a of the *Priority Development Project (PDP) SWQMP for Otay Village 7, R-8 & R-4 (April 2023)* and has been prepared to be submitted concurrently with the SWQMP.

SECTIONS

1. Introduction/Summary
2. Flow Duration Curve Analysis
3. Elevation vs. Discharge, Elevation vs Area Curves, Drawdown, SWMM Input Calculations
4. Basin Details
5. SWMM Input Data (Existing and Proposed Models)
6. Hydromodification Watershed Maps

I. INTRODUCTION/SUMMARY

This HMP report summarizes the approach used to model the proposed Otay Ranch Village 7 R-8 & R-4 sites within City of Chula Vista, California using the Environmental Protection Agency (EPA) Storm Water Management Model 5.1 (SWMM).

SWMM models were prepared for pre- and post- developed conditions at the project designated Point of Compliance (POC) in order to demonstrate that for flow rates ranging from pre-development (0.1Q2) to Q10 runoff event, the post-development discharge rates and durations, don't exceed the pre-development (not pre-project) rates and durations by more than 10% at the discharge point of the existing detention basin located immediately north of the project site. As a result, the stormwater discharge from the site will not cause any altered flow regimes or excessive downstream erosion in the receiving waters.

Therefore, no additional BMPs need to be provided for flow control to meet the current HMP requirements from the Regional Water Quality Control Board (RWQCB).

The runoff from the proposed project will be collected in inlets and will be conveyed through the onsite storm drain system. This system will tie into existing drainage facilities which have anticipated these onsite flows and will ultimately reach the existing BMP basin located immediately north of the project site. Once runoff routes through the respective basin outlet structure, it exits the basin via one 36-in RCP outlet pipe. See Section 6 of this Report for HMP-related maps.

Following are brief description of the POCs:

POC 1:

POC 1 is located at the discharge location from the riser that manages detention in the existing detention basin north of lot R-8 and adjacent to La Media Road.

This POC encompasses the area of R-8, the existing detention basin, the existing Otay Ranch Village 7 development, a portion of La Media Road and surrounding non-developed areas (Portion of the Vortac site), for a total of 152.73 acres in existing and 152.65 in proposed conditions. The difference in area is due to the R-8 development that cause minor changes to the drainage patterns.

POC 2:

POC 2 is located at near the Village 8 West development boundary. A clean out where the flows from R-4 commingle with the offsite flows before entering Village 8 West.

This POC encompasses the area of the proposed Vortac site access road, a portion of the Vortac site, Santa Luna Street, a portion of Magdalena Avenue, the Olympian High school, and lot R-4. POC 2 has a total area of 89.63 acres in the existing condition and 89.71 in the proposed conditions. The difference reason is stated above as the area lost on POC 1 is gained on POC 2.

SWMM MODEL DEVELOPMENT

POC 1:

The following will outline the typical model development. Two (2) SWMM models are prepared for the POC; one represents existing conditions and another represents proposed condition.

The existing condition analysis was prepared by defining the area to the respective POC. Similarly, the proposed condition analysis was prepared by defining the areas to the respective POC.

The Otay Ranch Village 7 R-8 site was modeled with 0% imperviousness in the pre-development condition, and 48% imperviousness in the post-development condition.

The existing detention basin was modeled with 1.5% imperviousness for both, post and pre-development condition.

The rest of the northern and eastern portions corresponding to existing development of Otay Ranch Village 7 that also discharge to the existing detention basin was modeled with 47% imperviousness in pre and post-development conditions. This was calculated as weighted percentage of imperviousness between the existing development of Otay Ranch Village 7 and the additional surrounding non-developed areas. For the existing developed area of Otay Ranch Village 7 (108.43 ac), a 60% of imperviousness was considered per the "Preliminary Water Quality Technical Report for Otay Ranch Village 7 Neighborhoods R-2" dated May, 2004. For the rest of the existing non-developed surrounding areas, a 0% of imperviousness was considered.

For all SWMM models, flow duration curves were prepared to demonstrate that the existing basin footprint/volume and outlet structure will be sufficient to meet the current HMP requirements.

The inputs required to develop SWMM models include rainfall, watershed characteristics, and BMP configurations. The Lower Otay Reservoir Rain Gage from the Project Clean Water website was used for this study since it is the most representative of the project site precipitation.

Evaporation for the site was modeled using average monthly values from the San Diego County hourly dataset. The site was modeled with hydrologic group D soil. Other SWMM inputs for the subareas are discussed in the following sections of this document where the selection of parameters is further explained in detail.

Existing Basin Discussion: Flow control at the basin is not achieved with the existing outlet and structure configuration (one orifice at the bottom of an 8' x 8' x 10.9' concrete riser structure), therefore, a new orifice and height configuration is proposed for the existing outlet riser structure to achieve flow control at the existing basin. The size, number and location of the orifices along the height of the outlet structure for existing and proposed conditions are presented in the Tables below.

Table 1 Existing Outlet Structure & Orifice Characteristics

Bottom orifice diameter:	30 in
Number:	1
Cg-low:	0.61
invert elev:	0.00 ft
Emergency weir:	
Invert:	10.90 ft
Weir Length (ft)	32.00 ft
Riser Box LxW	8'X8'

Table 2 Proposed Modified Outlet Structure & Orifice Characteristics

1 Orifice Stag diameter:	10 in
Number:	1
Cg-low:	0.61
invert elev:	0.00 ft
2 Orifice Stag diameter:	12 in
number of orif:	1
Cg-middle:	0.61
invert elev:	1.50 ft
3 Orifice Stag diameter:	18 in
Number:	2
Cg-low:	0.61
invert elev:	3.00 ft
4 Orifice Stag diameter:	24 in
Number:	2
Cg-low:	0.61
invert elev:	8.00 ft
Emergency weir:	
Invert:	11.90 ft
Area	64 sq ft
Weir Length	32
Riser Box LxW	8'x8'

POC 2:

The following will outline the typical model development. Two (3) SWMM models are prepared for the POC; one represents existing conditions, another one represents the mass graded condition with the access road developed, and the last one represents developed condition for both the access road and lot R-4.

The existing condition analysis was prepared by defining the area to the respective POC. Similarly, the proposed condition analysis was prepared by defining the areas to the respective POC.

The Otay Ranch Village 7 R-4 site was modeled with 0% imperviousness in the pre-development condition, 0% imperviousness in the mass graded condition, and 78% in the post-development condition.

The Vortac access road off Magdalena Avenue was modeled with 0% imperviousness in the pre-development condition, and 49.5% in the mass graded and post-development condition.

For all SWMM models, flow duration curves were prepared to demonstrate that the mass graded condition does not need any flood control structures to meet HMP requirements, while the post-development condition will need an underground storage unit.

The inputs required to develop SWMM models include rainfall, watershed characteristics, and BMP configurations. The Lower Otay Reservoir Rain Gage from the Project Clean Water website was used for this study since it is the most representative of the project site precipitation.

Evaporation for the site was modeled using average monthly values from the San Diego County hourly dataset. The site was modeled with hydrologic group D soil. Other SWMM inputs for the subareas are discussed in the following sections of this document where the selection of parameters is further explained in detail.

Table 3 Proposed Outlet structure & Orifice Characteristics

Bottom orifice diameter:	1.922 in	Emergency weir:	
Number:	1	Invert:	4.65
Cg-low:	0.61	Weir Length (ft)	5.75 ft
invert elev:	0.00 ft	Riser Box LxW	3'x1.92' ft
Middle orifice diameter:	2.5 in		
Number:	2		
Cg-low:	0.61		
invert elev:	2.70 ft		
Top orifice diameter:	3 in		
Number:	4		
Cg-low:	0.61		
invert elev:	4.0 ft		

FLOW DURATION CURVE COMPARISON

The Flow Duration Curves (FDC) for the site was calculated at the POC by exporting the hourly runoff time series results from SWMM to a spreadsheet. The FDC for the POC was compared between 10% of the existing condition Q₂ ((based on accepting an assumption of high susceptibility for downstream channel erosion as required if no soils tests are completed)

Up to the existing condition Q₁₀. The Q₂ and Q₁₀ were determined using a partial duration statistical analysis of the runoff time series in an Excel spreadsheet. As the SWMM Model is a statistical analysis based on the Weibull Plotting Position Method, the Weibull Method was also used within the spreadsheet to ensure that the results were similar to those obtained by the SWMM Model.

The range between 10% of Q₂ and Q₁₀ was divided into 100 equal time intervals; the number of hours that each flow rate was exceeded was counted from the hourly series. Additionally, the intermediate peaks with a return period "i" were obtained (Q_i with i=3 to 9). For the purpose of the plot, the values were presented as a percentage of time that exceeded for each flow rate.

Section 6 of this HMP Study provides detailed drainage exhibit for the post-developed conditions. As shown in Figures 1 and 2 in the succeeding section, the FDC for the proposed condition for the basin is within 110% of the curve for the existing condition. The additional runoff volume generated from developing the site will be released to the downstream storm drain at a flow rate below the 10% Q₂ lower threshold. Additionally, the Proposed Project will not increase peak flow rates between the Q₂ and the Q₁₀, as shown in the graphics in Figure 3 and also in the Tables included in Section 1. Similar FDC comparison curves also were generated for the POC and included in the following Section 1 of this report.

SUMMARY & CONCLUSION

A summary of pre and post development conditions draining to the POC is shown in the table 2 below. The Existing Basin was recognized in the pre and post conditions.

Table 4 POC Area Summary

	Existing (AC)	Proposed (AC)
POC 1	152.73	152.65
POC 2	89.63	89.71
Total	242.36	242.36

For both SWMM models, flow duration curves were prepared to demonstrate that the existing basin storage and footprints and the proposed modified outlet structure will be sufficient to meet the current HMP requirements for POC 1. Similarly the three SWMM models, for POC 2 had flow duration curves performed to demonstrate that there is no need for flood control structures in the mass graded conditions, and that an underground storage unit with an outlet structure will help meet HMP requirements in the post-developed condition.

II. FLOW DURATION CURVE ANALYSIS

SECTION 1 - Flow Duration Curve Analysis, Plot & Table

The FDCs shall not exceed the existing conditions by more than 10%, neither in peak flow nor duration.

Figure 1 on the following page illustrates that the FDCs in post-development conditions, is not exceeding the existing FDC with more than 10%. The FDC table following the curve shows that if the interval 0.10Q2 – Q 10 is divided in 100 sub-intervals, then: a) the post development divided by pre-development durations are never larger than 110% (the permit allows up to 110%); and, b) there are no more than 3 intervals in the range 101%-110%, which would imply an excess over 10% of the length of the curve (the permit allows less than 10% of excesses measured as 101-110%). Consequently, the design passes the hydromodification test.

It is important to note that the FDCs can be expressed in the “x” axis as a percentage of time, hours per year, total number of hours, or any other similar time variable. As those variables only differ by multiplying a constant, their plot in logarithmic scale will appear exactly the same and compliance can be observed regardless of the variable selected. The selection of a logarithmic scale in lieu of the normal scale is preferred, as differences between the pre-development and post-development curves can be depicted more clearly in the entire range of analysis. Both graphics are presented for reference in Table 4 and Figure 1 below.

For the “y” axis, the peak flow value is the variable of choice. As an additional analysis performed by H&A, not only the range of analysis is clearly depicted (10% of Q 2 to Q 10) but also all intermediate flows are shown (30% of Q 2, 50% of Q 2, Q 2, Q 3, Q 4, Q 5, Q 6, Q 7, Q 8 and Q 9) in order to demonstrate compliance at any range $Q_x - Q_{x+1}$. It must be pointed out that one of the limitations of both the SWMM and SDHM models is that the intermediate analysis is not performed (to obtain Q_i from $i = 2$ to 10). H&A performed the analysis using the Cunnane Plotting Position Method (the preferred method in the HMP permit) from the “n” largest independent peak flows obtained from the continuous time series.

Otay Village 7, R-8, DMA Calculations
Pre-Developed Condition

POC	Location (POC ID)	% Imperviousness	Total Area	Pervious Area	Impervious Area
1	Otay Ranch Village 7 R-8	0.0%	13.58	13.58	0.00
1	Otay Ranch Village Ranch 7	47.0%	136.19	72.18	64.01
1	Detention Bain	1.5%	2.96	2.91	0.04
1	Total Ex Basin	41.9%	152.73	88.68	64.05

Otay Village 7, R-8 - DMA Calculations
Post-Developed Condition

POC	Location (POC ID)	% Imperviousness	Total Area	Pervious Area	Impervious Area
1-Direct	Otay Ranch Village 7 R-8	48.0%	13.82	7.19	6.63
1-Direct	Otay Ranch Village Ranch 7	47.0%	135.87	72.01	63.86
1-Direct	Detention Bain	1.5%	2.96	2.91	0.04
1-Total	Total Ex Basin	46.2%	152.65	82.11	70.54

Watershed Parameters

POC 2 -EX

L=	3385	ft
A=	89.63	ac
% Impervious	64.9%	
W=	1190	ft
US Elev=	578	ft
DS Elev=	463	ft
S=	3.40%	

POC 2 -PR-UNMITIGATED (Access road developed)

L=	3385	ft
A=	89.63	ac
% Impervious	65.1%	
W=	1190	ft
US Elev=	578	ft
DS Elev=	463	ft
S=	3.40%	

POC 2 -PR-MITIGATED (R-4, R-8 & Access road developed)

Village 7

L=	3385	ft
A=	86.52	ac
% Impervious	67.3%	
W=	1190	ft
US Elev=	578	ft
DS Elev=	463	ft
S=	3.40%	

DMA 3

L=	725	ft
A=	0.34	ac
% Impervious	49.5%	
W=	675	ft
US Elev=	561	ft
DS Elev=	540	ft
S=	2.90%	

DMA 2

L=	800	ft
A=	2.85	ac
% Impervious	78.0%	
W=	155	ft
US Elev=	470	ft
DS Elev=	460	ft
S=	1.25%	

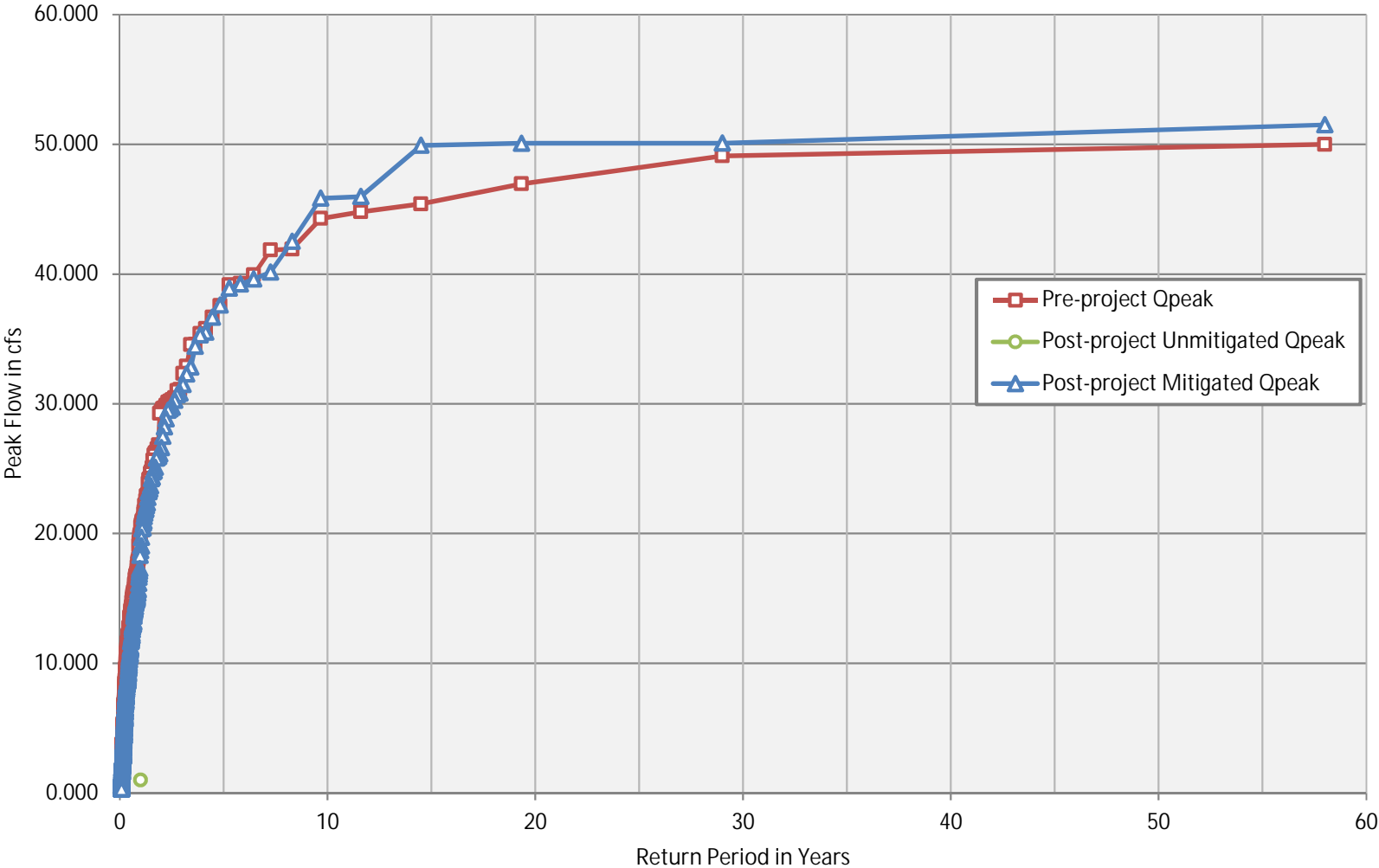
Flow Duration Curve Analysis, Plot & Table

Figure 1 Peak Flow Frequency Curves - POC1

Peak Flow Frequency Summary

Return Period	Pre-project Q _{peak} (cfs)	Post-project - Mitigated Q (cfs)	Post-project - Mitigated Reduction Q (cfs)
LF = 0.1xQ ₂	2.907	2.641	0.266
2-year	29.068	26.411	2.657
5-year	37.878	37.640	0.238
10-year	43.871	45.209	-1.338

Peak Flow Frequency Curves



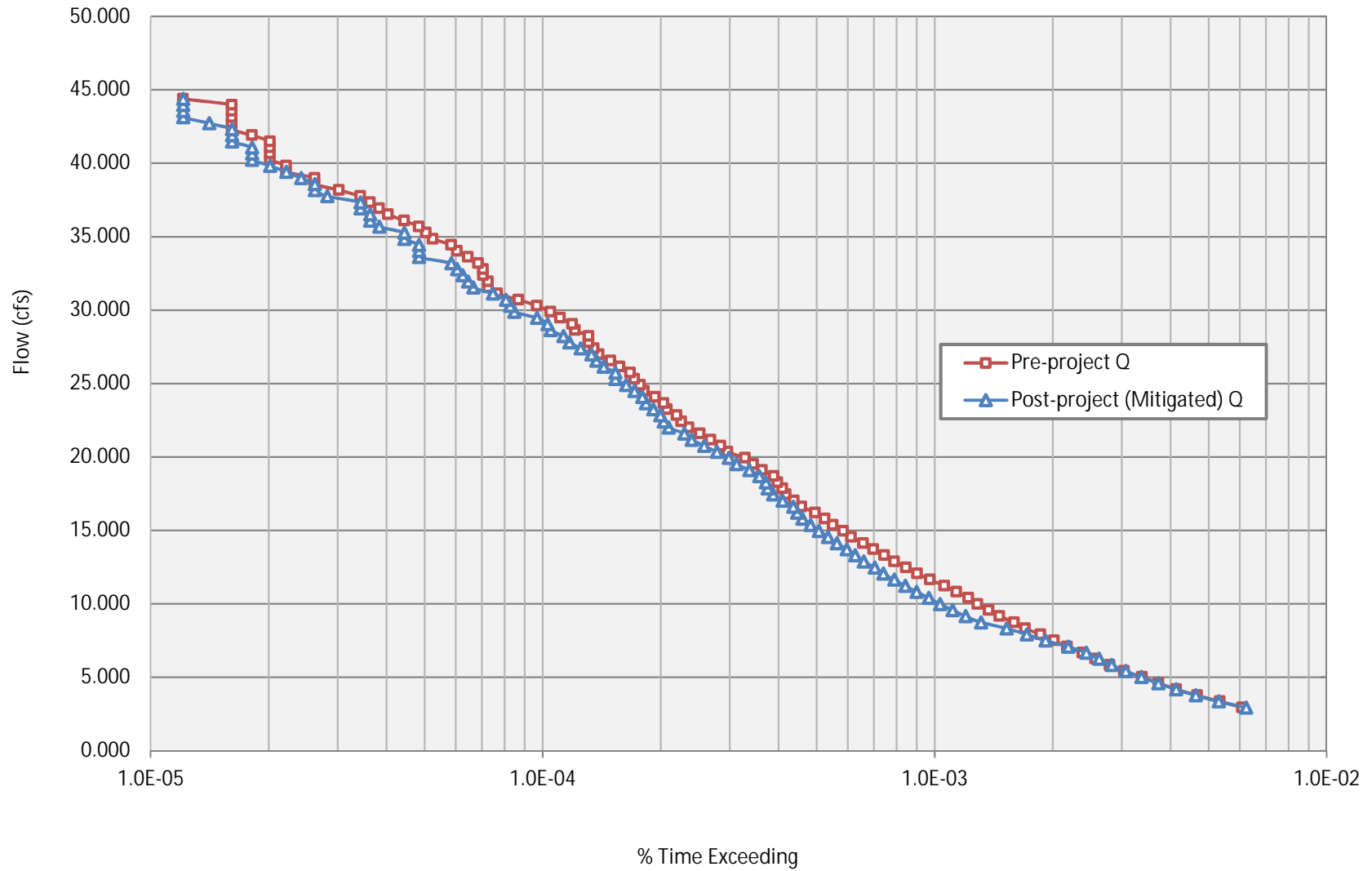
Low-flow Threshold: **10%**
 0.1xQ2 (Pre): 2.941 cfs
 Q10 (Pre): 44.379 cfs
 Ordinate #: 100
 Incremental Q (Pre): 0.41439 cfs
 Total Hourly Data: **495743** hours

The proposed BMP: **PASSED**

Interval	Pre-project Flow (cfs)	Pre-project Hours	Pre-project % Time Exceeding	Post-project Hours	Post-project % Time Exceeding	Percentage	Pass/Fail
0	2.941	3017	6.09E-03	3088	6.23E-03	102%	Pass
1	3.355	2649	5.34E-03	2629	5.30E-03	99%	Pass
2	3.769	2319	4.68E-03	2299	4.64E-03	99%	Pass
3	4.184	2052	4.14E-03	2048	4.13E-03	100%	Pass
4	4.598	1847	3.73E-03	1844	3.72E-03	100%	Pass
5	5.012	1678	3.38E-03	1669	3.37E-03	99%	Pass
6	5.427	1510	3.05E-03	1523	3.07E-03	101%	Pass
7	5.841	1387	2.80E-03	1403	2.83E-03	101%	Pass
8	6.256	1274	2.57E-03	1302	2.63E-03	102%	Pass
9	6.670	1183	2.39E-03	1209	2.44E-03	102%	Pass
10	7.084	1080	2.18E-03	1087	2.19E-03	101%	Pass
11	7.499	1001	2.02E-03	951	1.92E-03	95%	Pass
12	7.913	924	1.86E-03	852	1.72E-03	92%	Pass
13	8.328	844	1.70E-03	757	1.53E-03	90%	Pass
14	8.742	791	1.60E-03	651	1.31E-03	82%	Pass
15	9.156	726	1.46E-03	595	1.20E-03	82%	Pass
16	9.571	682	1.38E-03	551	1.11E-03	81%	Pass
17	9.985	638	1.29E-03	512	1.03E-03	80%	Pass
18	10.399	605	1.22E-03	479	9.66E-04	79%	Pass
19	10.814	564	1.14E-03	446	9.00E-04	79%	Pass
20	11.228	525	1.06E-03	417	8.41E-04	79%	Pass
21	11.643	483	9.74E-04	391	7.89E-04	81%	Pass
22	12.057	448	9.04E-04	367	7.40E-04	82%	Pass
23	12.471	419	8.45E-04	349	7.04E-04	83%	Pass
24	12.886	391	7.89E-04	327	6.60E-04	84%	Pass
25	13.300	369	7.44E-04	311	6.27E-04	84%	Pass
26	13.715	346	6.98E-04	296	5.97E-04	86%	Pass
27	14.129	326	6.58E-04	279	5.63E-04	86%	Pass
28	14.543	304	6.13E-04	265	5.35E-04	87%	Pass
29	14.958	290	5.85E-04	251	5.06E-04	87%	Pass
30	15.372	273	5.51E-04	239	4.82E-04	88%	Pass
31	15.786	260	5.24E-04	228	4.60E-04	88%	Pass
32	16.201	246	4.96E-04	221	4.46E-04	90%	Pass
33	16.615	227	4.58E-04	216	4.36E-04	95%	Pass
34	17.030	217	4.38E-04	203	4.09E-04	94%	Pass
35	17.444	207	4.18E-04	192	3.87E-04	93%	Pass
36	17.858	203	4.09E-04	186	3.75E-04	92%	Pass
37	18.273	197	3.97E-04	184	3.71E-04	93%	Pass
38	18.687	193	3.89E-04	177	3.57E-04	92%	Pass
39	19.102	180	3.63E-04	167	3.37E-04	93%	Pass
40	19.516	171	3.45E-04	155	3.13E-04	91%	Pass
41	19.930	163	3.29E-04	148	2.99E-04	91%	Pass
42	20.345	147	2.97E-04	138	2.78E-04	94%	Pass
43	20.759	141	2.84E-04	128	2.58E-04	91%	Pass
44	21.174	133	2.68E-04	119	2.40E-04	89%	Pass
45	21.588	125	2.52E-04	114	2.30E-04	91%	Pass
46	22.002	117	2.36E-04	104	2.10E-04	89%	Pass
47	22.417	112	2.26E-04	101	2.04E-04	90%	Pass
48	22.831	109	2.20E-04	99	2.00E-04	91%	Pass
49	23.245	103	2.08E-04	95	1.92E-04	92%	Pass
50	23.660	101	2.04E-04	91	1.84E-04	90%	Pass

Interval	Pre-project Flow (cfs)	Pre-project Hours	Pre-project % Time Exceeding	Post-project Hours	Post-project % Time Exceeding	Percentage	Pass/Fail
51	24.074	96	1.94E-04	89	1.80E-04	93%	Pass
52	24.489	90	1.82E-04	85	1.71E-04	94%	Pass
53	24.903	88	1.78E-04	81	1.63E-04	92%	Pass
54	25.317	85	1.71E-04	76	1.53E-04	89%	Pass
55	25.732	83	1.67E-04	76	1.53E-04	92%	Pass
56	26.146	78	1.57E-04	71	1.43E-04	91%	Pass
57	26.561	74	1.49E-04	68	1.37E-04	92%	Pass
58	26.975	69	1.39E-04	66	1.33E-04	96%	Pass
59	27.389	67	1.35E-04	62	1.25E-04	93%	Pass
60	27.804	65	1.31E-04	58	1.17E-04	89%	Pass
61	28.218	65	1.31E-04	56	1.13E-04	86%	Pass
62	28.632	60	1.21E-04	52	1.05E-04	87%	Pass
63	29.047	59	1.19E-04	51	1.03E-04	86%	Pass
64	29.461	55	1.11E-04	48	9.68E-05	87%	Pass
65	29.876	52	1.05E-04	42	8.47E-05	81%	Pass
66	30.290	48	9.68E-05	41	8.27E-05	85%	Pass
67	30.704	43	8.67E-05	40	8.07E-05	93%	Pass
68	31.119	38	7.67E-05	37	7.46E-05	97%	Pass
69	31.533	36	7.26E-05	33	6.66E-05	92%	Pass
70	31.948	36	7.26E-05	32	6.45E-05	89%	Pass
71	32.362	35	7.06E-05	31	6.25E-05	89%	Pass
72	32.776	35	7.06E-05	30	6.05E-05	86%	Pass
73	33.191	34	6.86E-05	29	5.85E-05	85%	Pass
74	33.605	32	6.45E-05	24	4.84E-05	75%	Pass
75	34.020	30	6.05E-05	24	4.84E-05	80%	Pass
76	34.434	29	5.85E-05	24	4.84E-05	83%	Pass
77	34.848	26	5.24E-05	22	4.44E-05	85%	Pass
78	35.263	25	5.04E-05	22	4.44E-05	88%	Pass
79	35.677	24	4.84E-05	19	3.83E-05	79%	Pass
80	36.091	22	4.44E-05	18	3.63E-05	82%	Pass
81	36.506	20	4.03E-05	18	3.63E-05	90%	Pass
82	36.920	19	3.83E-05	17	3.43E-05	89%	Pass
83	37.335	18	3.63E-05	17	3.43E-05	94%	Pass
84	37.749	17	3.43E-05	14	2.82E-05	82%	Pass
85	38.163	15	3.03E-05	13	2.62E-05	87%	Pass
86	38.578	13	2.62E-05	13	2.62E-05	100%	Pass
87	38.992	13	2.62E-05	12	2.42E-05	92%	Pass
88	39.407	11	2.22E-05	11	2.22E-05	100%	Pass
89	39.821	11	2.22E-05	10	2.02E-05	91%	Pass
90	40.235	10	2.02E-05	9	1.82E-05	90%	Pass
91	40.650	10	2.02E-05	9	1.82E-05	90%	Pass
92	41.064	10	2.02E-05	9	1.82E-05	90%	Pass
93	41.478	10	2.02E-05	8	1.61E-05	80%	Pass
94	41.893	9	1.82E-05	8	1.61E-05	89%	Pass
95	42.307	8	1.61E-05	8	1.61E-05	100%	Pass
96	42.722	8	1.61E-05	7	1.41E-05	88%	Pass
97	43.136	8	1.61E-05	6	1.21E-05	75%	Pass
98	43.550	8	1.61E-05	6	1.21E-05	75%	Pass
99	43.965	8	1.61E-05	6	1.21E-05	75%	Pass
100	44.379	6	1.21E-05	6	1.21E-05	100%	Pass

Flow Duration Curve [Pre vs. Post (Mitigated)]



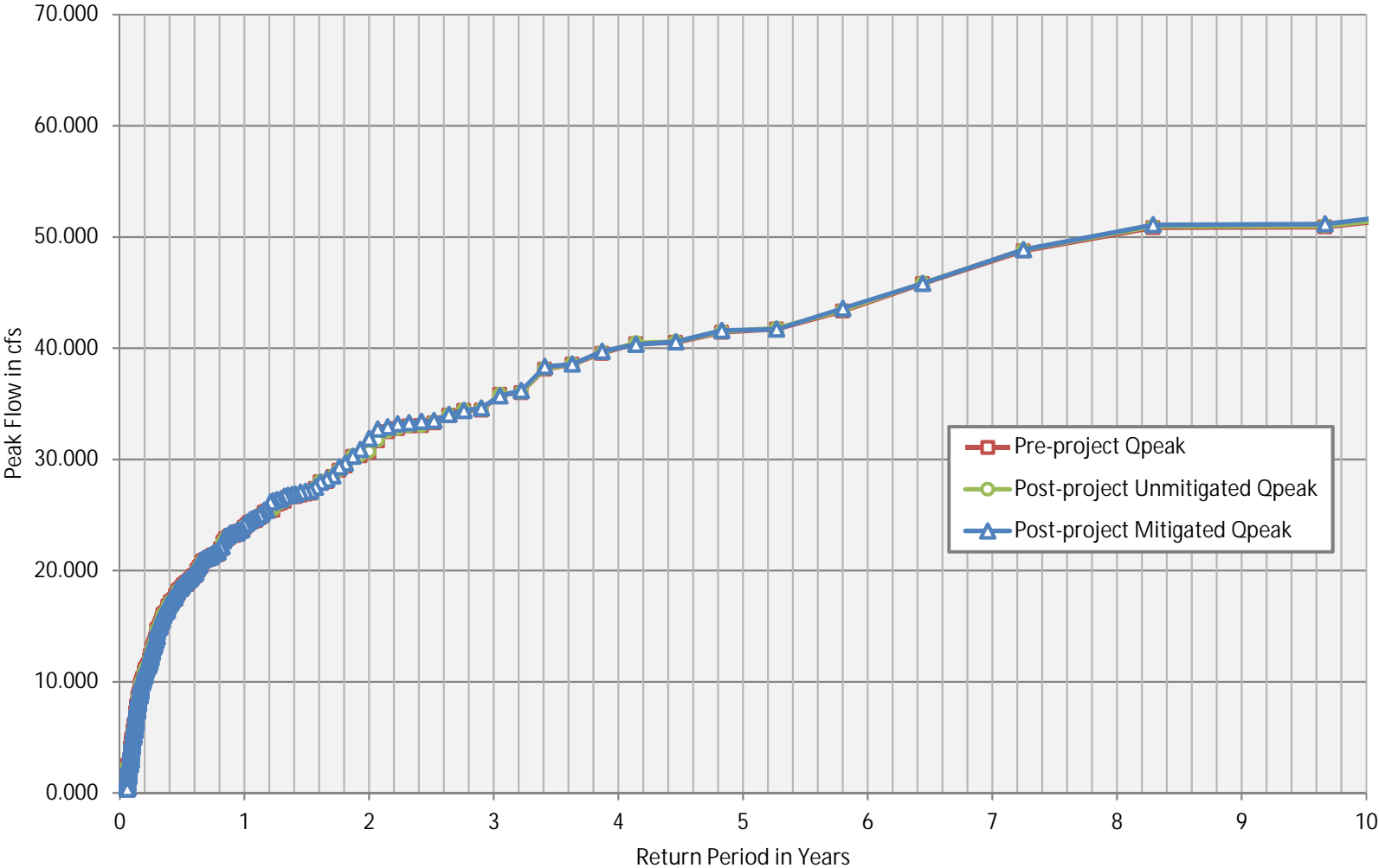
Flow Duration Curve Analysis, Plot & Table

Figure 2 Peak Flow Frequency Curves – POC2

Peak Flow Frequency Summary (POC 2)

Return Period	Pre-project Qpeak (cfs)	Post-project - Unmitigated Q (R-4 mass graded) (cfs)	Post-project - Mitigated Q (R-4 developed) (cfs)	Reduction Q (cfs)
LF = 0.5xQ2	3.063	3.071	3.187	-0.124
2-year	30.632	30.706	31.867	-1.235
5-year	41.536	41.616	41.637	-0.101
10-year	51.299	51.385	51.617	-0.317

Peak Flow Frequency Curves



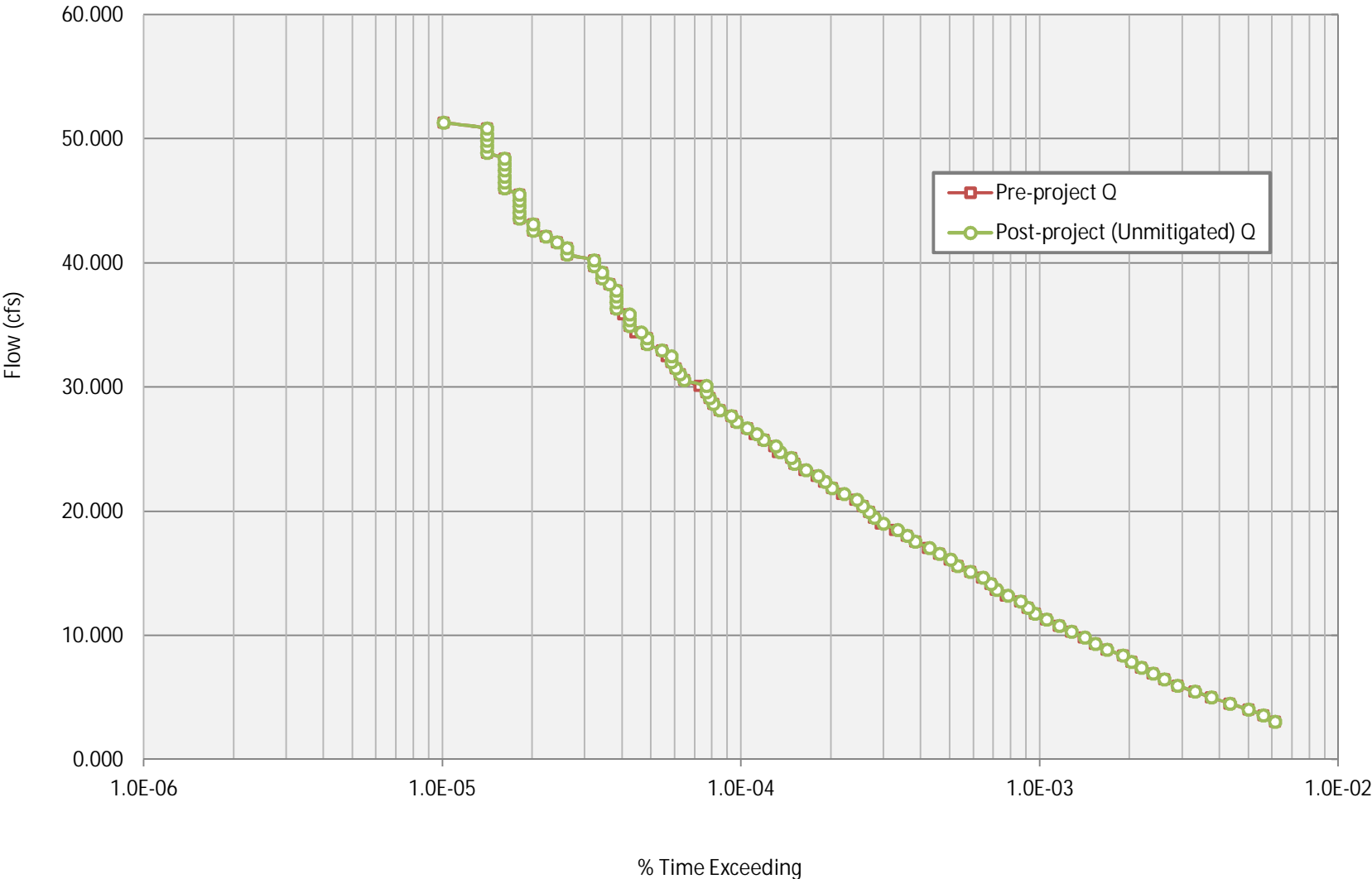
Lower Flow Threshold: 10%
 0.1xQ2 (Pre): 3.063 cfs
 Q10 (Pre): 51.299 cfs
 # of Ordinates: 100
 Incremental Q (Pre): 0.48236 cfs
 Total Hourly Data: 495743 hours

POC 2: PASSED

Interval	Pre-project Flow (cfs)	Pre-project Hours	Pre-project % Time Exceeding	Post-project Hours	Post-project % Time Exceeding	Percentage	Pass/Fail
0	3.063	3045	6.14E-03	3047	6.15E-03	100%	Pass
1	3.546	2779	5.61E-03	2783	5.61E-03	100%	Pass
2	4.028	2480	5.00E-03	2483	5.01E-03	100%	Pass
3	4.510	2144	4.32E-03	2152	4.34E-03	100%	Pass
4	4.993	1860	3.75E-03	1865	3.76E-03	100%	Pass
5	5.475	1637	3.30E-03	1645	3.32E-03	100%	Pass
6	5.957	1436	2.90E-03	1438	2.90E-03	100%	Pass
7	6.440	1297	2.62E-03	1299	2.62E-03	100%	Pass
8	6.922	1187	2.39E-03	1191	2.40E-03	100%	Pass
9	7.404	1084	2.19E-03	1088	2.19E-03	100%	Pass
10	7.887	1005	2.03E-03	1006	2.03E-03	100%	Pass
11	8.369	941	1.90E-03	942	1.90E-03	100%	Pass
12	8.852	831	1.68E-03	833	1.68E-03	100%	Pass
13	9.334	758	1.53E-03	762	1.54E-03	101%	Pass
14	9.816	698	1.41E-03	702	1.42E-03	101%	Pass
15	10.299	631	1.27E-03	633	1.28E-03	100%	Pass
16	10.781	576	1.16E-03	578	1.17E-03	100%	Pass
17	11.263	522	1.05E-03	524	1.06E-03	100%	Pass
18	11.746	478	9.64E-04	479	9.66E-04	100%	Pass
19	12.228	452	9.12E-04	454	9.16E-04	100%	Pass
20	12.710	426	8.59E-04	428	8.63E-04	100%	Pass
21	13.193	382	7.71E-04	388	7.83E-04	102%	Pass
22	13.675	354	7.14E-04	356	7.18E-04	101%	Pass
23	14.157	340	6.86E-04	341	6.88E-04	100%	Pass
24	14.640	318	6.41E-04	320	6.45E-04	101%	Pass
25	15.122	291	5.87E-04	291	5.87E-04	100%	Pass
26	15.605	264	5.33E-04	264	5.33E-04	100%	Pass
27	16.087	248	5.00E-04	250	5.04E-04	101%	Pass
28	16.569	228	4.60E-04	229	4.62E-04	100%	Pass
29	17.052	210	4.24E-04	212	4.28E-04	101%	Pass
30	17.534	190	3.83E-04	190	3.83E-04	100%	Pass
31	18.016	178	3.59E-04	179	3.61E-04	101%	Pass
32	18.499	163	3.29E-04	166	3.35E-04	102%	Pass
33	18.981	146	2.95E-04	149	3.01E-04	102%	Pass
34	19.463	138	2.78E-04	139	2.80E-04	101%	Pass
35	19.946	133	2.68E-04	133	2.68E-04	100%	Pass
36	20.428	127	2.56E-04	127	2.56E-04	100%	Pass
37	20.911	120	2.42E-04	121	2.44E-04	101%	Pass
38	21.393	108	2.18E-04	110	2.22E-04	102%	Pass
39	21.875	100	2.02E-04	100	2.02E-04	100%	Pass
40	22.358	94	1.90E-04	95	1.92E-04	101%	Pass
41	22.840	89	1.80E-04	90	1.82E-04	101%	Pass
42	23.322	81	1.63E-04	82	1.65E-04	101%	Pass
43	23.805	75	1.51E-04	75	1.51E-04	100%	Pass
44	24.287	73	1.47E-04	73	1.47E-04	100%	Pass
45	24.769	66	1.33E-04	67	1.35E-04	102%	Pass
46	25.252	64	1.29E-04	65	1.31E-04	102%	Pass
47	25.734	59	1.19E-04	59	1.19E-04	100%	Pass
48	26.216	55	1.11E-04	56	1.13E-04	102%	Pass
49	26.699	52	1.05E-04	52	1.05E-04	100%	Pass

50	27.181	48	9.68E-05	48	9.68E-05	100%	Pass
51	27.664	46	9.28E-05	46	9.28E-05	100%	Pass
52	28.146	42	8.47E-05	42	8.47E-05	100%	Pass
53	28.628	40	8.07E-05	40	8.07E-05	100%	Pass
54	29.111	39	7.87E-05	39	7.87E-05	100%	Pass
55	29.593	38	7.67E-05	38	7.67E-05	100%	Pass
56	30.075	36	7.26E-05	38	7.67E-05	106%	Pass
57	30.558	32	6.45E-05	32	6.45E-05	100%	Pass
58	31.040	31	6.25E-05	31	6.25E-05	100%	Pass
59	31.522	30	6.05E-05	30	6.05E-05	100%	Pass
60	32.005	29	5.85E-05	29	5.85E-05	100%	Pass
61	32.487	28	5.65E-05	29	5.85E-05	104%	Pass
62	32.970	27	5.45E-05	27	5.45E-05	100%	Pass
63	33.452	24	4.84E-05	24	4.84E-05	100%	Pass
64	33.934	24	4.84E-05	24	4.84E-05	100%	Pass
65	34.417	22	4.44E-05	23	4.64E-05	105%	Pass
66	34.899	21	4.24E-05	21	4.24E-05	100%	Pass
67	35.381	21	4.24E-05	21	4.24E-05	100%	Pass
68	35.864	20	4.03E-05	21	4.24E-05	105%	Pass
69	36.346	19	3.83E-05	19	3.83E-05	100%	Pass
70	36.828	19	3.83E-05	19	3.83E-05	100%	Pass
71	37.311	19	3.83E-05	19	3.83E-05	100%	Pass
72	37.793	19	3.83E-05	19	3.83E-05	100%	Pass
73	38.275	18	3.63E-05	18	3.63E-05	100%	Pass
74	38.758	17	3.43E-05	17	3.43E-05	100%	Pass
75	39.240	17	3.43E-05	17	3.43E-05	100%	Pass
76	39.723	16	3.23E-05	16	3.23E-05	100%	Pass
77	40.205	16	3.23E-05	16	3.23E-05	100%	Pass
78	40.687	13	2.62E-05	13	2.62E-05	100%	Pass
79	41.170	13	2.62E-05	13	2.62E-05	100%	Pass
80	41.652	12	2.42E-05	12	2.42E-05	100%	Pass
81	42.134	11	2.22E-05	11	2.22E-05	100%	Pass
82	42.617	10	2.02E-05	10	2.02E-05	100%	Pass
83	43.099	10	2.02E-05	10	2.02E-05	100%	Pass
84	43.581	9	1.82E-05	9	1.82E-05	100%	Pass
85	44.064	9	1.82E-05	9	1.82E-05	100%	Pass
86	44.546	9	1.82E-05	9	1.82E-05	100%	Pass
87	45.029	9	1.82E-05	9	1.82E-05	100%	Pass
88	45.511	9	1.82E-05	9	1.82E-05	100%	Pass
89	45.993	8	1.61E-05	8	1.61E-05	100%	Pass
90	46.476	8	1.61E-05	8	1.61E-05	100%	Pass
91	46.958	8	1.61E-05	8	1.61E-05	100%	Pass
92	47.440	8	1.61E-05	8	1.61E-05	100%	Pass
93	47.923	8	1.61E-05	8	1.61E-05	100%	Pass
94	48.405	8	1.61E-05	8	1.61E-05	100%	Pass
95	48.887	7	1.41E-05	7	1.41E-05	100%	Pass
96	49.370	7	1.41E-05	7	1.41E-05	100%	Pass
97	49.852	7	1.41E-05	7	1.41E-05	100%	Pass
98	50.334	7	1.41E-05	7	1.41E-05	100%	Pass
99	50.817	7	1.41E-05	7	1.41E-05	100%	Pass
100	51.299	5	1.01E-05	5	1.01E-05	100%	Pass

Flow Duration Curve [Pre vs. Post (Unmitigated)]



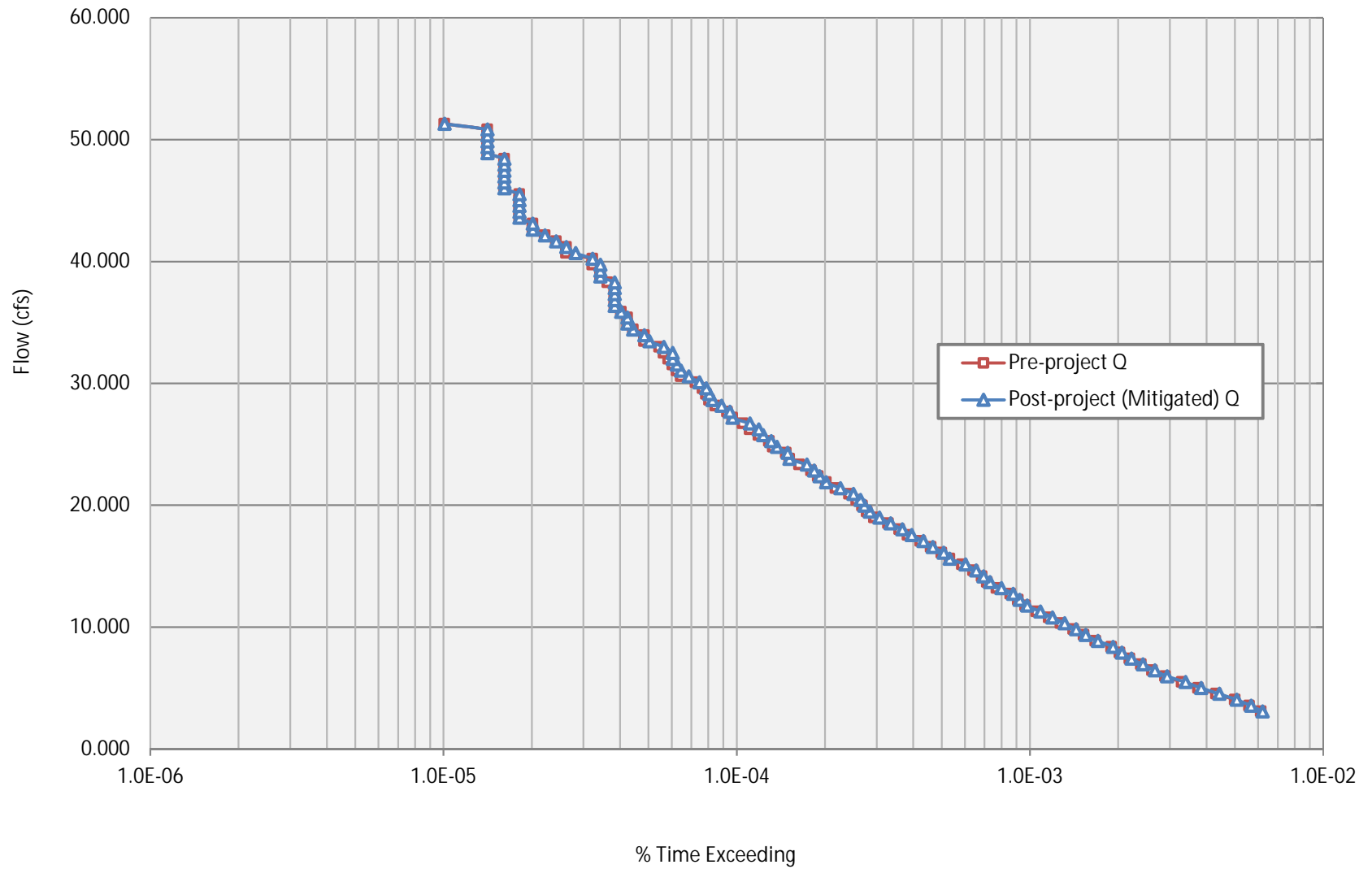
Low-flow Threshold: 10%
 0.1xQ2 (Pre): 3.063 cfs
 Q10 (Pre): 51.299 cfs
 Ordinate #: 100
 Incremental Q (Pre): 0.48236 cfs
 Total Hourly Data: 495743 hours

The proposed BMP: PASSED

Interval	Pre-project Flow (cfs)	Pre-project Hours	Pre-project % Time Exceeding	Post-project Hours	Post-project % Time Exceeding	Percentage	Pass/Fail
0	3.063	3045	6.14E-03	3082	6.22E-03	101%	Pass
1	3.546	2779	5.61E-03	2814	5.68E-03	101%	Pass
2	4.028	2480	5.00E-03	2518	5.08E-03	102%	Pass
3	4.510	2144	4.32E-03	2194	4.43E-03	102%	Pass
4	4.993	1860	3.75E-03	1901	3.83E-03	102%	Pass
5	5.475	1637	3.30E-03	1682	3.39E-03	103%	Pass
6	5.957	1436	2.90E-03	1456	2.94E-03	101%	Pass
7	6.440	1297	2.62E-03	1324	2.67E-03	102%	Pass
8	6.922	1187	2.39E-03	1204	2.43E-03	101%	Pass
9	7.404	1084	2.19E-03	1101	2.22E-03	102%	Pass
10	7.887	1005	2.03E-03	1021	2.06E-03	102%	Pass
11	8.369	941	1.90E-03	953	1.92E-03	101%	Pass
12	8.852	831	1.68E-03	845	1.70E-03	102%	Pass
13	9.334	758	1.53E-03	768	1.55E-03	101%	Pass
14	9.816	698	1.41E-03	712	1.44E-03	102%	Pass
15	10.299	631	1.27E-03	651	1.31E-03	103%	Pass
16	10.781	576	1.16E-03	591	1.19E-03	103%	Pass
17	11.263	522	1.05E-03	539	1.09E-03	103%	Pass
18	11.746	478	9.64E-04	485	9.78E-04	101%	Pass
19	12.228	452	9.12E-04	457	9.22E-04	101%	Pass
20	12.710	426	8.59E-04	435	8.77E-04	102%	Pass
21	13.193	382	7.71E-04	396	7.99E-04	104%	Pass
22	13.675	354	7.14E-04	362	7.30E-04	102%	Pass
23	14.157	340	6.86E-04	344	6.94E-04	101%	Pass
24	14.640	318	6.41E-04	325	6.56E-04	102%	Pass
25	15.122	291	5.87E-04	299	6.03E-04	103%	Pass
26	15.605	264	5.33E-04	264	5.33E-04	100%	Pass
27	16.087	248	5.00E-04	252	5.08E-04	102%	Pass
28	16.569	228	4.60E-04	231	4.66E-04	101%	Pass
29	17.052	210	4.24E-04	215	4.34E-04	102%	Pass
30	17.534	190	3.83E-04	196	3.95E-04	103%	Pass
31	18.016	178	3.59E-04	182	3.67E-04	102%	Pass
32	18.499	163	3.29E-04	166	3.35E-04	102%	Pass
33	18.981	146	2.95E-04	152	3.07E-04	104%	Pass
34	19.463	138	2.78E-04	141	2.84E-04	102%	Pass
35	19.946	133	2.68E-04	135	2.72E-04	102%	Pass
36	20.428	127	2.56E-04	131	2.64E-04	103%	Pass
37	20.911	120	2.42E-04	124	2.50E-04	103%	Pass
38	21.393	108	2.18E-04	112	2.26E-04	104%	Pass
39	21.875	100	2.02E-04	100	2.02E-04	100%	Pass
40	22.358	94	1.90E-04	95	1.92E-04	101%	Pass
41	22.840	89	1.80E-04	91	1.84E-04	102%	Pass
42	23.322	81	1.63E-04	86	1.73E-04	106%	Pass
43	23.805	75	1.51E-04	75	1.51E-04	100%	Pass
44	24.287	73	1.47E-04	74	1.49E-04	101%	Pass
45	24.769	66	1.33E-04	68	1.37E-04	103%	Pass
46	25.252	64	1.29E-04	65	1.31E-04	102%	Pass
47	25.734	59	1.19E-04	61	1.23E-04	103%	Pass
48	26.216	55	1.11E-04	59	1.19E-04	107%	Pass
49	26.699	52	1.05E-04	55	1.11E-04	106%	Pass
50	27.181	48	9.68E-05	48	9.68E-05	100%	Pass

Interval	Pre-project Flow (cfs)	Pre-project Hours	Pre-project % Time Exceeding	Post-project Hours	Post-project % Time Exceeding	Percentage	Pass/Fail
51	27.664	46	9.28E-05	47	9.48E-05	102%	Pass
52	28.146	42	8.47E-05	44	8.88E-05	105%	Pass
53	28.628	40	8.07E-05	41	8.27E-05	103%	Pass
54	29.111	39	7.87E-05	40	8.07E-05	103%	Pass
55	29.593	38	7.67E-05	39	7.87E-05	103%	Pass
56	30.075	36	7.26E-05	37	7.46E-05	103%	Pass
57	30.558	32	6.45E-05	34	6.86E-05	106%	Pass
58	31.040	31	6.25E-05	32	6.45E-05	103%	Pass
59	31.522	30	6.05E-05	31	6.25E-05	103%	Pass
60	32.005	29	5.85E-05	30	6.05E-05	103%	Pass
61	32.487	28	5.65E-05	30	6.05E-05	107%	Pass
62	32.970	27	5.45E-05	28	5.65E-05	104%	Pass
63	33.452	24	4.84E-05	25	5.04E-05	104%	Pass
64	33.934	24	4.84E-05	24	4.84E-05	100%	Pass
65	34.417	22	4.44E-05	22	4.44E-05	100%	Pass
66	34.899	21	4.24E-05	21	4.24E-05	100%	Pass
67	35.381	21	4.24E-05	21	4.24E-05	100%	Pass
68	35.864	20	4.03E-05	20	4.03E-05	100%	Pass
69	36.346	19	3.83E-05	19	3.83E-05	100%	Pass
70	36.828	19	3.83E-05	19	3.83E-05	100%	Pass
71	37.311	19	3.83E-05	19	3.83E-05	100%	Pass
72	37.793	19	3.83E-05	19	3.83E-05	100%	Pass
73	38.275	18	3.63E-05	19	3.83E-05	106%	Pass
74	38.758	17	3.43E-05	17	3.43E-05	100%	Pass
75	39.240	17	3.43E-05	17	3.43E-05	100%	Pass
76	39.723	16	3.23E-05	17	3.43E-05	106%	Pass
77	40.205	16	3.23E-05	16	3.23E-05	100%	Pass
78	40.687	13	2.62E-05	14	2.82E-05	108%	Pass
79	41.170	13	2.62E-05	13	2.62E-05	100%	Pass
80	41.652	12	2.42E-05	12	2.42E-05	100%	Pass
81	42.134	11	2.22E-05	11	2.22E-05	100%	Pass
82	42.617	10	2.02E-05	10	2.02E-05	100%	Pass
83	43.099	10	2.02E-05	10	2.02E-05	100%	Pass
84	43.581	9	1.82E-05	9	1.82E-05	100%	Pass
85	44.064	9	1.82E-05	9	1.82E-05	100%	Pass
86	44.546	9	1.82E-05	9	1.82E-05	100%	Pass
87	45.029	9	1.82E-05	9	1.82E-05	100%	Pass
88	45.511	9	1.82E-05	9	1.82E-05	100%	Pass
89	45.993	8	1.61E-05	8	1.61E-05	100%	Pass
90	46.476	8	1.61E-05	8	1.61E-05	100%	Pass
91	46.958	8	1.61E-05	8	1.61E-05	100%	Pass
92	47.440	8	1.61E-05	8	1.61E-05	100%	Pass
93	47.923	8	1.61E-05	8	1.61E-05	100%	Pass
94	48.405	8	1.61E-05	8	1.61E-05	100%	Pass
95	48.887	7	1.41E-05	7	1.41E-05	100%	Pass
96	49.370	7	1.41E-05	7	1.41E-05	100%	Pass
97	49.852	7	1.41E-05	7	1.41E-05	100%	Pass
98	50.334	7	1.41E-05	7	1.41E-05	100%	Pass
99	50.817	7	1.41E-05	7	1.41E-05	100%	Pass
100	51.299	5	1.01E-05	5	1.01E-05	100%	Pass

Flow Duration Curve [Pre vs. Post (Mitigated)]



III. ELEVATION VS DISCHARGE, ELEVATION VS AREA, DRAWDOWN, SWMM INPUT PARAMETER CALCULATIONS

Elevation vs. Area

For the flow diverted each respective receiving detention basin, a pond is used to route the hydrographs. The elevation vs area curve in the model is calculated in Excel and imported into the model.

Elevation vs Discharge

The total discharge peak flow data is imported from an Excel spreadsheet that calculated the elevation vs discharge of the outlet system for the basin.

A detail of the orifice and riser structure is provided in Section 3 of this attachment. The stage-storage and stage-discharge calculations have been provided on the following pages.

POC 1

Otay Village 7 R-8, Ex Basin

Discharge vs Elevation Table

1 Orifice Stag diameter: 30 "			2 Orifice Stag diameter: 12 "			3 Orifice Stag diameter: 18 "			4 Orifice Stag diameter: 24 "			Emergency w 8X8								
Number: 1			number of orif: 0			Number: 0			Number: 0			Invert: 10.90 ft								
Cg-low: 0.61			Cg-middle: 0.61			Cg-low: 0.61			Cg-low: 0.61			Area: 64.00 sq ft								
Invert elev: 0.00 ft			Invert elev: 1.50 ft			Invert elev: 3.00 ft			Invert elev: 8.00 ft			Circumferenc: 8' x 8'								
h	H/D-1	H/D-2	H/D-3	H/D-4	H/D-peak	Qlow-orif	Qlow-weir	Qtot-1	Qmid-orif	Qmid-weir	Qtot-2	Qtop-orif	Qtop-weir	Qtot-3	Qtop-orif	Qtop-weir	Qtot-4	Q emergency	Qtot	Qtot w UD
(ft)						(cfs)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.05	0.02	0.00	0.00	0.00	0.00	0.00	0.02	0.02	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.015	0.015
0.10	0.04	0.00	0.00	0.00	0.00	0.00	0.06	0.06	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.057	0.057
0.15	0.06	0.00	0.00	0.00	0.00	0.00	0.13	0.13	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.125	0.125
0.20	0.08	0.00	0.00	0.00	0.00	0.00	0.22	0.22	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.219	0.219
0.25	0.10	0.00	0.00	0.00	0.00	0.00	0.34	0.34	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.339	0.339
0.30	0.12	0.00	0.00	0.00	0.00	0.00	0.48	0.48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.484	0.484
0.35	0.14	0.00	0.00	0.00	0.00	0.00	0.65	0.65	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.654	0.654
0.40	0.16	0.00	0.00	0.00	0.00	0.00	0.85	0.85	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.849	0.849
0.45	0.18	0.00	0.00	0.00	0.00	0.00	1.07	1.07	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.069	1.069
0.50	0.20	0.00	0.00	0.00	0.00	0.00	1.31	1.31	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.312	1.312
0.55	0.22	0.00	0.00	0.00	0.00	0.00	1.58	1.58	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.579	1.579
0.60	0.24	0.00	0.00	0.00	0.00	0.00	1.87	1.87	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.869	1.869
0.65	0.26	0.00	0.00	0.00	0.00	0.00	2.18	2.18	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.182	2.182
0.70	0.28	0.00	0.00	0.00	0.00	0.00	2.52	2.52	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.518	2.518
0.75	0.30	0.00	0.00	0.00	0.00	0.00	2.88	2.88	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.876	2.876
0.80	0.32	0.00	0.00	0.00	0.00	0.00	3.26	3.26	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	3.255	3.255
0.85	0.34	0.00	0.00	0.00	0.00	0.00	3.66	3.66	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	3.656	3.656
0.90	0.36	0.00	0.00	0.00	0.00	0.00	4.08	4.08	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	4.078	4.078
0.95	0.38	0.00	0.00	0.00	0.00	0.00	4.52	4.52	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	4.520	4.520
1.00	0.40	0.00	0.00	0.00	0.00	0.00	4.98	4.98	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	4.982	4.982
1.05	0.42	0.00	0.00	0.00	0.00	0.00	5.46	5.46	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	5.463	5.463
1.10	0.44	0.00	0.00	0.00	0.00	0.00	5.96	5.96	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	5.964	5.964
1.15	0.46	0.00	0.00	0.00	0.00	0.00	6.48	6.48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	6.483	6.483
1.20	0.48	0.00	0.00	0.00	0.00	0.00	7.02	7.02	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	7.020	7.020
1.25	0.50	0.00	0.00	0.00	0.00	0.00	7.57	7.57	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	7.575	7.575
1.30	0.52	0.00	0.00	0.00	0.00	0.00	8.15	8.15	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	8.147	8.147
1.35	0.54	0.00	0.00	0.00	0.00	0.00	8.74	8.74	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	8.735	8.735
1.40	0.56	0.00	0.00	0.00	0.00	0.00	9.34	9.34	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	9.340	9.340
1.45	0.58	0.00	0.00	0.00	0.00	0.00	9.96	9.96	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	9.960	9.960
1.50	0.60	0.00	0.00	0.00	0.00	0.00	10.60	10.60	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	10.596	10.596
1.55	0.62	0.05	0.00	0.00	0.00	0.00	13.16	11.25	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	11.246	11.246
1.60	0.64	0.10	0.00	0.00	0.00	0.00	14.22	11.91	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	11.910	11.910
1.65	0.66	0.15	0.00	0.00	0.00	0.00	15.20	12.59	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	12.588	12.588
1.70	0.68	0.20	0.00	0.00	0.00	0.00	16.12	13.28	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	13.279	13.279
1.75	0.70	0.25	0.00	0.00	0.00	0.00	16.99	13.98	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	13.982	13.982
1.80	0.72	0.30	0.00	0.00	0.00	0.00	17.82	14.70	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	14.698	14.698
1.85	0.74	0.35	0.00	0.00	0.00	0.00	18.61	15.42	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	15.424	15.424
1.90	0.76	0.40	0.00	0.00	0.00	0.00	19.37	16.16	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	16.162	16.162
1.95	0.78	0.45	0.00	0.00	0.00	0.00	20.10	16.91	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	16.910	16.910
2.00	0.80	0.50	0.00	0.00	0.00	0.00	20.81	17.67	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	17.668	17.668
2.05	0.82	0.55	0.00	0.00	0.00	0.00	21.49	18.44	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	18.435	18.435
2.10	0.84	0.60	0.00	0.00	0.00	0.00	22.15	19.21	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	19.211	19.211
2.15	0.86	0.65	0.00	0.00	0.00	0.00	22.80	20.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	19.995	19.995
2.20	0.88	0.70	0.00	0.00	0.00	0.00	23.42	20.79	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	20.787	20.787
2.25	0.90	0.75	0.00	0.00	0.00	0.00	24.03	21.59	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	21.586	21.586
2.30	0.92	0.80	0.00	0.00	0.00	0.00	24.62	22.39	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	22.391	22.391
2.35	0.94	0.85	0.00	0.00	0.00	0.00	25.20	23.20	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	23.203	23.203
2.40	0.96	0.90	0.00	0.00	0.00	0.00	25.77	24.02	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	24.020	24.020
2.45	0.98	0.95	0.00	0.00	0.00	0.00	26.32	24.84	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	24.842	24.842
2.50	1.00	1.00	0.00	0.00	0.00	0.00	26.87	25.67	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	25.668	25.668
2.55	1.02	1.05	0.00	0.00	0.00	0.00	27.40	26.50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	26.498	26.498
2.60	1.04	1.10	0.00	0.00	0.00	0.00	27.92	27.33	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	27.331	27.331
2.65	1.06	1.15	0.00	0.00	0.00	0.00	28.43	28.17	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	28.167	28.167
2.70	1.08	1.20	0.00	0.00	0.00	0.00	28.94	29.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	29.005	29.005
2.75	1.10	1.25	0.00	0.00	0.00	0.00	29.43	29.85	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	29.430	29.430
2.80	1.12	1.30	0.00	0.00	0.00	0.00	29.92	30.69	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	29.916	29.916
2.85	1.14	1.35	0.00	0.00	0.00	0.00	30.40	31.53	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	30.395	30.395
2.90	1.16	1.40																		

0.20	0.08	0.00	0.00	0.00	0.00	0.00	0.22	0.22	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.219	0.219
0.25	0.10	0.00	0.00	0.00	0.00	0.00	0.34	0.34	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.339	0.339
5.80	2.32	4.30	1.87	0.00	0.00	51.26	67.21	51.26	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	51.256	51.256
5.85	2.34	4.35	1.90	0.00	0.00	51.54	67.43	51.54	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	51.537	51.537
5.90	2.36	4.40	1.93	0.00	0.00	51.82	67.63	51.82	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	51.817	51.817
5.95	2.38	4.45	1.97	0.00	0.00	52.09	67.82	52.09	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	52.094	52.094
6.00	2.40	4.50	2.00	0.00	0.00	52.37	67.99	52.37	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	52.371	52.371
6.05	2.42	4.55	2.03	0.00	0.00	52.65	68.16	52.65	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	52.646	52.646
6.10	2.44	4.60	2.07	0.00	0.00	52.92	68.31	52.92	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	52.919	52.919
6.15	2.46	4.65	2.10	0.00	0.00	53.19	68.45	53.19	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	53.191	53.191
6.20	2.48	4.70	2.13	0.00	0.00	53.46	68.58	53.46	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	53.462	53.462
6.25	2.50	4.75	2.17	0.00	0.00	53.73	68.70	53.73	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	53.731	53.731
6.30	2.52	4.80	2.20	0.00	0.00	54.00	68.80	54.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	53.999	53.999
6.35	2.54	4.85	2.23	0.00	0.00	54.27	68.90	54.27	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	54.266	54.266
6.40	2.56	4.90	2.27	0.00	0.00	54.53	68.99	54.53	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	54.531	54.531
6.45	2.58	4.95	2.30	0.00	0.00	54.80	69.06	54.80	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	54.795	54.795
6.50	2.60	5.00	2.33	0.00	0.00	55.06	69.13	55.06	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	55.058	55.058
6.55	2.62	5.05	2.37	0.00	0.00	55.32	69.19	55.32	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	55.320	55.320
6.60	2.64	5.10	2.40	0.00	0.00	55.58	69.25	55.58	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	55.580	55.580
6.65	2.66	5.15	2.43	0.00	0.00	55.84	69.29	55.84	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	55.839	55.839
6.70	2.68	5.20	2.47	0.00	0.00	56.10	69.34	56.10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	56.097	56.097
6.75	2.70	5.25	2.50	0.00	0.00	56.35	69.37	56.35	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	56.354	56.354
6.80	2.72	5.30	2.53	0.00	0.00	56.61	69.40	56.61	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	56.609	56.609
6.85	2.74	5.35	2.57	0.00	0.00	56.86	69.43	56.86	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	56.864	56.864
6.90	2.76	5.40	2.60	0.00	0.00	57.12	69.45	57.12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	57.117	57.117
6.95	2.78	5.45	2.63	0.00	0.00	57.37	69.47	57.37	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	57.369	57.369
7.00	2.80	5.50	2.67	0.00	0.00	57.62	69.49	57.62	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	57.620	57.620
7.05	2.82	5.55	2.70	0.00	0.00	57.87	69.50	57.87	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	57.870	57.870
7.10	2.84	5.60	2.73	0.00	0.00	58.12	69.52	58.12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	58.119	58.119
7.15	2.86	5.65	2.77	0.00	0.00	58.37	69.54	58.37	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	58.367	58.367
7.20	2.88	5.70	2.80	0.00	0.00	58.61	69.55	58.61	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	58.614	58.614
7.25	2.90	5.75	2.83	0.00	0.00	58.86	69.58	58.86	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	58.860	58.860
7.30	2.92	5.80	2.87	0.00	0.00	59.10	69.60	59.10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	59.104	59.104
7.35	2.94	5.85	2.90	0.00	0.00	59.35	69.63	59.35	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	59.348	59.348
7.40	2.96	5.90	2.93	0.00	0.00	59.59	69.66	59.59	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	59.591	59.591
7.45	2.98	5.95	2.97	0.00	0.00	59.83	69.71	59.83	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	59.833	59.833
7.50	3.00	6.00	3.00	0.00	0.00	60.07	69.76	60.07	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	60.073	60.073
7.55	3.02	6.05	3.03	0.00	0.00	60.31	69.81	60.31	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	60.313	60.313
7.60	3.04	6.10	3.07	0.00	0.00	60.55	69.88	60.55	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	60.552	60.552
7.65	3.06	6.15	3.10	0.00	0.00	60.79	69.96	60.79	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	60.790	60.790
7.70	3.08	6.20	3.13	0.00	0.00	61.03	70.05	61.03	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	61.027	61.027
7.75	3.10	6.25	3.17	0.00	0.00	61.26	70.16	61.26	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	61.263	61.263
7.80	3.12	6.30	3.20	0.00	0.00	61.50	70.28	61.50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	61.498	61.498
7.85	3.14	6.35	3.23	0.00	0.00	61.73	70.42	61.73	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	61.733	61.733
7.90	3.16	6.40	3.27	0.00	0.00	61.97	70.57	61.97	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	61.966	61.966
7.95	3.18	6.45	3.30	0.00	0.00	62.20	70.75	62.20	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	62.199	62.199
8.00	3.20	6.50	3.33	0.00	0.00	62.43	70.94	62.43	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	62.430	62.430
8.05	3.22	6.55	3.37	0.02	0.00	62.66	71.16	62.66	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	62.661	62.661
8.10	3.24	6.60	3.40	0.05	0.00	62.89	71.40	62.89	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	62.891	62.891
8.15	3.26	6.65	3.43	0.07	0.00	63.12	71.67	63.12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	63.120	63.120
8.20	3.28	6.70	3.47	0.10	0.00	63.35	71.96	63.35	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	63.348	63.348
8.25	3.30	6.75	3.50	0.12	0.00	63.58	72.28	63.58	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	63.576	63.576
8.30	3.32	6.80	3.53	0.15	0.00	63.80	72.63	63.80	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	63.802	63.802
8.35	3.34	6.85	3.57	0.17	0.00	64.03	73.01	64.03	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	64.028	64.028
8.40	3.36	6.90	3.60	0.20	0.00	64.25	73.43	64.25	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	64.253	64.253
8.45	3.38	6.95	3.63	0.22	0.00	64.48	73.88	64.48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	64.478	64.478
8.50	3.40	7.00	3.67	0.25	0.00	64.70	74.36	64.70	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	64.701	64.701
8.55	3.42	7.05	3.70	0.27	0.00	64.92	74.89	64.92	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	64.924	64.924
8.60	3.44	7.10	3.73	0.30	0.00	65.15	75.46	65.15	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	65.146	65.146
8.65	3.46	7.15	3.77	0.32	0.00	65.37	76.07	65.37	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	65.367	65.367
8.70	3.48	7.20	3.80	0.35	0.00	65.59	76.72	65.59	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	65.587	65.587
8.75	3.50	7.25	3.83	0.37	0.00	65.81	77.42	65.81	0.00											

0.20	0.08	0.00	0.00	0.00	0.00	0.00	0.00	0.22	0.22	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.219	0.219	
0.25	0.10	0.00	0.00	0.00	0.00	0.00	0.00	0.34	0.34	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.339	0.339	
11.95	4.78	10.45	5.97	1.98	0.20	78.60	343.00	78.60	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	229.30	307.904	307.904
12.00	4.80	10.50	6.00	2.00	0.21	78.79	353.10	78.79	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	245.87	324.660	324.660
12.05	4.82	10.55	6.03	2.03	0.22	78.97	363.49	78.97	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	262.83	341.796	341.796
12.10	4.84	10.60	6.07	2.05	0.23	79.15	374.16	79.15	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	280.15	359.305	359.305
12.15	4.86	10.65	6.10	2.08	0.23	79.33	385.12	79.33	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	297.84	377.178	377.178
12.20	4.88	10.70	6.13	2.10	0.24	79.52	396.37	79.52	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	315.89	395.408	395.408
12.25	4.90	10.75	6.17	2.13	0.25	79.70	407.93	79.70	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	334.29	413.988	413.988
12.30	4.92	10.80	6.20	2.15	0.26	79.88	419.79	79.88	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	353.03	432.911	432.911
12.35	4.94	10.85	6.23	2.18	0.27	80.06	431.97	80.06	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	372.11	452.172	452.172
12.40	4.96	10.90	6.27	2.20	0.28	80.24	444.46	80.24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	383.71	463.944	463.944
12.45	4.98	10.95	6.30	2.23	0.29	80.42	457.28	80.42	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	390.05	470.466	470.466
12.50	5.00	11.00	6.33	2.25	0.30	80.60	470.43	80.60	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	396.29	476.887	476.887
12.55	5.02	11.05	6.37	2.28	0.31	80.78	483.91	80.78	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	402.43	483.210	483.210
12.60	5.04	11.10	6.40	2.30	0.32	80.95	497.73	80.95	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	408.49	489.440	489.440
12.65	5.06	11.15	6.43	2.33	0.33	81.13	511.90	81.13	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	414.45	495.582	495.582
12.70	5.08	11.20	6.47	2.35	0.34	81.31	526.42	81.31	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	420.33	501.639	501.639
12.75	5.10	11.25	6.50	2.38	0.35	81.49	541.30	81.49	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	426.13	507.614	507.614
12.80	5.12	11.30	6.53	2.40	0.36	81.66	556.55	81.66	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	431.85	513.511	513.511
12.85	5.14	11.35	6.57	2.43	0.37	81.84	572.17	81.84	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	437.49	519.333	519.333
12.90	5.16	11.40	6.60	2.45	0.38	82.02	588.16	82.02	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	443.07	525.083	525.083
12.95	5.18	11.45	6.63	2.48	0.38	82.19	604.54	82.19	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	448.57	530.763	530.763
13.00	5.20	11.50	6.67	2.50	0.39	82.37	621.31	82.37	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	454.01	536.375	536.375
13.05	5.22	11.55	6.70	2.53	0.40	82.54	638.48	82.54	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	459.38	541.924	541.924
13.10	5.24	11.60	6.73	2.55	0.41	82.72	656.05	82.72	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	464.69	547.409	547.409
13.15	5.26	11.65	6.77	2.58	0.42	82.89	674.04	82.89	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	469.94	552.834	552.834
13.20	5.28	11.70	6.80	2.60	0.43	83.07	692.44	83.07	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	475.13	558.201	558.201
13.25	5.30	11.75	6.83	2.63	0.44	83.24	711.26	83.24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	480.27	563.512	563.512
13.30	5.32	11.80	6.87	2.65	0.45	83.41	730.51	83.41	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	485.35	568.767	568.767
13.35	5.34	11.85	6.90	2.68	0.46	83.59	750.20	83.59	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	490.38	573.970	573.970
13.40	5.36	11.90	6.93	2.70	0.47	83.76	770.34	83.76	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	495.36	579.121	579.121
13.45	5.38	11.95	6.97	2.73	0.48	83.93	790.92	83.93	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	500.29	584.222	584.222
13.50	5.40	12.00	7.00	2.75	0.49	84.10	811.97	84.10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	505.17	589.275	589.275
13.55	5.42	12.05	7.03	2.78	0.50	84.27	833.48	84.27	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	510.01	594.281	594.281
13.60	5.44	12.10	7.07	2.80	0.51	84.45	855.46	84.45	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	514.80	599.241	599.241
13.65	5.46	12.15	7.10	2.83	0.52	84.62	877.93	84.62	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	519.54	604.156	604.156
13.70	5.48	12.20	7.13	2.85	0.53	84.79	900.88	84.79	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	524.24	609.029	609.029
13.75	5.50	12.25	7.17	2.88	0.53	84.96	924.33	84.96	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	528.90	613.859	613.859
13.80	5.52	12.30	7.20	2.90	0.54	85.13	948.27	85.13	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	533.52	618.648	618.648
13.85	5.54	12.35	7.23	2.93	0.55	85.30	972.73	85.30	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	538.10	623.397	623.397
13.90	5.56	12.40	7.27	2.95	0.56	85.46	997.71	85.46	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	542.64	628.107	628.107

Otay Ranch Village 7 R-8
/ Stage Storage

Ex Detention Bain	
Depth (ft)	Area (sqft)
0.00	17535
0.05	17647
0.10	17758
0.15	17870
0.20	17982
0.25	18093
0.30	18205
0.35	18317
0.40	18428
0.45	18540
0.50	18651
0.55	18763
0.60	18875
0.65	18986
0.70	19098
0.75	19210
0.80	19321
0.85	19433
0.90	19545
0.95	19656
1.00	19768
1.05	19873
1.10	19979
1.15	20085
1.20	20190
1.25	20296
1.30	20401
1.35	20507
1.40	20613
1.45	20718
1.50	20824
1.55	20929
1.60	21035
1.65	21140
1.70	21246
1.75	21352
1.80	21457
1.85	21563
1.90	21668
1.95	21774
2.00	21880
2.05	21991

Depth (ft)	Area (sqft)
2.10	22102
2.15	22213
2.20	22325
2.25	22436
2.30	22547
2.35	22659
2.40	22770
2.45	22881
2.50	22993
2.55	23104
2.60	23215
2.65	23326
2.70	23438
2.75	23549
2.80	23660
2.85	23772
2.90	23883
2.95	23994
3.00	24106
3.05	24215
3.10	24325
3.15	24435
3.20	24545
3.25	24655
3.30	24765
3.35	24875
3.40	24985
3.45	25095
3.50	25205
3.55	25315
3.60	25425
3.65	25535
3.70	25645
3.75	25755
3.80	25865
3.85	25975
3.90	26085
3.95	26195
4.00	26305
4.05	26420
4.10	26536
4.15	26652
4.20	26768
4.25	26883
4.30	26999

Depth (ft)	Area (sqft)
4.35	27115
4.40	27230
4.45	27346
4.50	27462
4.55	27578
4.60	27693
4.65	27809
4.70	27925
4.75	28040
4.80	28156
4.85	28272
4.90	28388
4.95	28503
5.00	28619
5.05	28739
5.10	28860
5.15	28980
5.20	29101
5.25	29221
5.30	29342
5.35	29462
5.40	29582
5.45	29703
5.50	29823
5.55	29944
5.60	30064
5.65	30184
5.70	30305
5.75	30425
5.80	30546
5.85	30666
5.90	30787
5.95	30907
6.00	31027
6.05	31160
6.10	31292
6.15	31424
6.20	31557
6.25	31689
6.30	31821
6.35	31954
6.40	32086
6.45	32219
6.50	32351
6.55	32483

Depth (ft)	Area (sqft)
6.60	32616
6.65	32748
6.70	32880
6.75	33013
6.80	33145
6.85	33277
6.90	33410
6.95	33542
7.00	33674
7.05	33810
7.10	33945
7.15	34081
7.20	34216
7.25	34352
7.30	34487
7.35	34623
7.40	34758
7.45	34894
7.50	35029
7.55	35165
7.60	35300
7.65	35436
7.70	35571
7.75	35707
7.80	35842
7.85	35977
7.90	36113
7.95	36248
8.00	36384
8.05	36523
8.10	36662
8.15	36801
8.20	36941
8.25	37080
8.30	37219
8.35	37358
8.40	37497
8.45	37636
8.50	37776
8.55	37915
8.60	38054
8.65	38193
8.70	38332
8.75	38471
8.80	38611

Depth (ft)	Area (sqft)
8.85	38750
8.90	38889
8.95	39028
9.00	39167
9.05	39311
9.10	39455
9.15	39599
9.20	39743
9.25	39887
9.30	40031
9.35	40175
9.40	40319
9.45	40463
9.50	40607
9.55	40751
9.60	40895
9.65	41039
9.70	41183
9.75	41327
9.80	41471
9.85	41615
9.90	41759
9.95	41903
10.00	42047
10.05	42200
10.10	42353
10.15	42506
10.20	42659
10.25	42812
10.30	42965
10.35	43118
10.40	43271
10.45	43424
10.50	43577
10.55	43730
10.60	43883
10.65	44036
10.70	44189
10.75	44342
10.80	44495
10.85	44648
10.90	44801
10.95	44954
11.00	45107
11.05	45270

Depth (ft)	Area (sqft)
11.10	45432
11.15	45595
11.20	45758
11.25	45920
11.30	46083
11.35	46246
11.40	46408
11.45	46571
11.50	46734
11.55	46896
11.60	47059
11.65	47222
11.70	47384
11.75	47547
11.80	47710
11.85	47872
11.90	48035
11.95	48198
12.00	48360
12.05	48537
12.10	48713
12.15	48889
12.20	49066
12.25	49242
12.30	49419
12.35	49595
12.40	49772
12.45	49948
12.50	50124
12.55	50301
12.60	50477
12.65	50654
12.70	50830
12.75	51007
12.80	51183
12.85	51359
12.90	51536
12.95	51712
13.00	51889
13.05	52065
13.10	52242
13.15	52418
13.20	52594
13.25	52771
13.30	52947

Depth (ft)	Area (sqft)
13.35	53124
13.40	53300
13.45	53477
13.50	53653
13.55	53829
13.60	54006
13.65	54182
13.70	54359
13.75	54535
13.80	54712
13.85	54888
13.90	55064

Ex Basin		Q _{Sub Drain} =	0.000	cfs
Elevation	Q _{AVG} (CFS)	DV (CF)	DT (HR)	Total T
0.00	0.000	879.5	31.891	48.99
0.05	0.015	885.1	6.784	17.10
0.10	0.057	890.7	2.712	10.31
0.15	0.125	896.3	1.445	7.60
0.20	0.219	901.9	0.897	6.16
0.25	0.339	907.5	0.612	5.26
0.30	0.484	913.0	0.446	4.65
0.35	0.654	918.6	0.339	4.20
0.40	0.849	924.2	0.268	3.86
0.45	1.069	929.8	0.217	3.59
0.50	1.312	935.4	0.180	3.38
0.55	1.579	940.9	0.152	3.20
0.60	1.869	946.5	0.130	3.05
0.65	2.182	952.1	0.113	2.92
0.70	2.518	957.7	0.099	2.80
0.75	2.876	963.3	0.087	2.71
0.80	3.255	968.9	0.078	2.62
0.85	3.656	974.4	0.070	2.54
0.90	4.078	980.0	0.063	2.47
0.95	4.520	985.6	0.058	2.41
1.00	4.982	991.0	0.053	2.35
1.05	5.463	996.3	0.048	2.30
1.10	5.964	1001.6	0.045	2.25
1.15	6.483	1006.9	0.041	2.20
1.20	7.020	1012.1	0.039	2.16
1.25	7.575	1017.4	0.036	2.12
1.30	8.147	1022.7	0.034	2.09
1.35	8.735	1028.0	0.032	2.05
1.40	9.340	1033.3	0.030	2.02
1.45	9.960	1038.5	0.028	1.99
1.50	10.596	1043.8	0.027	1.96
1.55	11.246	1049.1	0.025	1.94
1.60	11.910	1054.4	0.024	1.91
1.65	12.588	1059.7	0.023	1.89
1.70	13.279	1064.9	0.022	1.87
1.75	13.982	1070.2	0.021	1.84
1.80	14.698	1075.5	0.020	1.82
1.85	15.424	1080.8	0.019	1.80
1.90	16.162	1086.1	0.018	1.78
1.95	16.910	1091.3	0.018	1.77
2.00	17.668	1096.8	0.017	1.75
2.05	18.435	1102.3	0.016	1.73
2.10	19.211	1107.9	0.016	1.72
2.15	19.995	1113.5	0.015	1.70
2.20	20.787	1119.0	0.015	1.68
2.25	21.586	1124.6	0.014	1.67
2.30	22.391	1130.1	0.014	1.66
2.35	23.203	1135.7	0.013	1.64
2.40	24.020	1141.3	0.013	1.63
2.45	24.842	1146.8	0.013	1.62
2.50	25.668	1152.4	0.012	1.60

Ex Basin		$Q_{\text{Sub Drain}} =$	0.000	cfs
Elevation	Q_{AVG} (CFS)	DV (CF)	DT (HR)	Total T
2.55	26.498	1158.0	0.012	1.59
2.60	27.331	1163.5	0.012	1.58
2.65	28.167	1169.1	0.011	1.57
2.70	29.005	1174.7	0.011	1.56
2.75	29.430	1180.2	0.011	1.54
2.80	29.916	1185.8	0.011	1.53
2.85	30.395	1191.4	0.011	1.52
2.90	30.866	1196.9	0.011	1.51
2.95	31.330	1202.5	0.011	1.50
3.00	31.788	1208.0	0.010	1.49
3.05	32.239	1213.5	0.010	1.48
3.10	32.683	1219.0	0.010	1.47
3.15	33.122	1224.5	0.010	1.46
3.20	33.555	1230.0	0.010	1.45
3.25	33.983	1235.5	0.010	1.44
3.30	34.405	1241.0	0.010	1.43
3.35	34.822	1246.5	0.010	1.42
3.40	35.234	1252.0	0.010	1.41
3.45	35.641	1257.5	0.010	1.40
3.50	36.044	1263.0	0.010	1.39
3.55	36.442	1268.5	0.010	1.38
3.60	36.836	1274.0	0.010	1.37
3.65	37.226	1279.5	0.009	1.36
3.70	37.612	1285.0	0.009	1.35
3.75	37.994	1290.5	0.009	1.34
3.80	38.372	1296.0	0.009	1.33
3.85	38.746	1301.5	0.009	1.32
3.90	39.117	1307.0	0.009	1.31
3.95	39.484	1312.5	0.009	1.30
4.00	39.848	1318.1	0.009	1.30
4.05	40.209	1323.9	0.009	1.29
4.10	40.566	1329.7	0.009	1.28
4.15	40.921	1335.5	0.009	1.27
4.20	41.272	1341.3	0.009	1.26
4.25	41.620	1347.1	0.009	1.25
4.30	41.966	1352.8	0.009	1.24
4.35	42.308	1358.6	0.009	1.23
4.40	42.648	1364.4	0.009	1.22
4.45	42.985	1370.2	0.009	1.21
4.50	43.320	1376.0	0.009	1.21
4.55	43.652	1381.8	0.009	1.20
4.60	43.981	1387.6	0.009	1.19
4.65	44.308	1393.3	0.009	1.18
4.70	44.633	1399.1	0.009	1.17
4.75	44.955	1404.9	0.009	1.16
4.80	45.275	1410.7	0.009	1.15
4.85	45.593	1416.5	0.009	1.14
4.90	45.908	1422.3	0.009	1.14
4.95	46.221	1428.1	0.009	1.13
5.00	46.533	1434.0	0.009	1.12
5.05	46.842	1440.0	0.009	1.11

Ex Basin		$Q_{\text{Sub Drain}} =$	0.000	cfs
Elevation	Q_{AVG} (CFS)	DV (CF)	DT (HR)	Total T
5.10	47.149	1446.0	0.008	1.10
5.15	47.454	1452.0	0.008	1.09
5.20	47.757	1458.0	0.008	1.09
5.25	48.059	1464.1	0.008	1.08
5.30	48.358	1470.1	0.008	1.07
5.35	48.656	1476.1	0.008	1.06
5.40	48.952	1482.1	0.008	1.05
5.45	49.246	1488.2	0.008	1.04
5.50	49.538	1494.2	0.008	1.03
5.55	49.828	1500.2	0.008	1.03
5.60	50.117	1506.2	0.008	1.02
5.65	50.404	1512.2	0.008	1.01
5.70	50.690	1518.3	0.008	1.00
5.75	50.974	1524.3	0.008	0.99
5.80	51.256	1530.3	0.008	0.98
5.85	51.537	1536.3	0.008	0.98
5.90	51.817	1542.3	0.008	0.97
5.95	52.094	1548.4	0.008	0.96
6.00	52.371	1554.7	0.008	0.95
6.05	52.646	1561.3	0.008	0.94
6.10	52.919	1567.9	0.008	0.94
6.15	53.191	1574.5	0.008	0.93
6.20	53.462	1581.1	0.008	0.92
6.25	53.731	1587.8	0.008	0.91
6.30	53.999	1594.4	0.008	0.90
6.35	54.266	1601.0	0.008	0.89
6.40	54.531	1607.6	0.008	0.89
6.45	54.795	1614.2	0.008	0.88
6.50	55.058	1620.9	0.008	0.87
6.55	55.320	1627.5	0.008	0.86
6.60	55.580	1634.1	0.008	0.85
6.65	55.839	1640.7	0.008	0.85
6.70	56.097	1647.3	0.008	0.84
6.75	56.354	1653.9	0.008	0.83
6.80	56.609	1660.6	0.008	0.82
6.85	56.864	1667.2	0.008	0.81
6.90	57.117	1673.8	0.008	0.80
6.95	57.369	1680.4	0.008	0.80
7.00	57.620	1687.1	0.008	0.79
7.05	57.870	1693.9	0.008	0.78
7.10	58.119	1700.7	0.008	0.77
7.15	58.367	1707.4	0.008	0.76
7.20	58.614	1714.2	0.008	0.76
7.25	58.860	1721.0	0.008	0.75
7.30	59.104	1727.7	0.008	0.74
7.35	59.348	1734.5	0.008	0.73
7.40	59.591	1741.3	0.008	0.72
7.45	59.833	1748.1	0.008	0.72
7.50	60.073	1754.8	0.008	0.71
7.55	60.313	1761.6	0.008	0.70
7.60	60.552	1768.4	0.008	0.69

Ex Basin		$Q_{\text{Sub Drain}} =$	0.000	cfs
Elevation	Q_{AVG} (CFS)	DV (CF)	DT (HR)	Total T
7.65	60.790	1775.2	0.008	0.68
7.70	61.027	1781.9	0.008	0.67
7.75	61.263	1788.7	0.008	0.67
7.80	61.498	1795.5	0.008	0.66
7.85	61.733	1802.3	0.008	0.65
7.90	61.966	1809.0	0.008	0.64
7.95	62.199	1815.8	0.008	0.63
8.00	62.430	1822.7	0.008	0.63
8.05	62.661	1829.6	0.008	0.62
8.10	62.891	1836.6	0.008	0.61
8.15	63.120	1843.6	0.008	0.60
8.20	63.348	1850.5	0.008	0.59
8.25	63.576	1857.5	0.008	0.59
8.30	63.802	1864.4	0.008	0.58
8.35	64.028	1871.4	0.008	0.57
8.40	64.253	1878.3	0.008	0.56
8.45	64.478	1885.3	0.008	0.55
8.50	64.701	1892.3	0.008	0.55
8.55	64.924	1899.2	0.008	0.54
8.60	65.146	1906.2	0.008	0.53
8.65	65.367	1913.1	0.008	0.52
8.70	65.587	1920.1	0.008	0.51
8.75	65.807	1927.1	0.008	0.50
8.80	66.026	1934.0	0.008	0.50
8.85	66.244	1941.0	0.008	0.49
8.90	66.462	1947.9	0.008	0.48
8.95	66.679	1954.9	0.008	0.47
9.00	66.895	1962.0	0.008	0.46
9.05	67.110	1969.2	0.008	0.46
9.10	67.325	1976.4	0.008	0.45
9.15	67.539	1983.6	0.008	0.44
9.20	67.753	1990.8	0.008	0.43
9.25	67.965	1998.0	0.008	0.42
9.30	68.177	2005.2	0.008	0.42
9.35	68.389	2012.4	0.008	0.41
9.40	68.600	2019.6	0.008	0.40
9.45	68.810	2026.8	0.008	0.39
9.50	69.019	2034.0	0.008	0.38
9.55	69.228	2041.2	0.008	0.37
9.60	69.436	2048.4	0.008	0.37
9.65	69.644	2055.6	0.008	0.36
9.70	69.851	2062.8	0.008	0.35
9.75	70.057	2070.0	0.008	0.34
9.80	70.263	2077.2	0.008	0.33
9.85	70.468	2084.4	0.008	0.33
9.90	70.673	2091.6	0.008	0.32
9.95	70.877	2098.8	0.008	0.31
10.00	71.080	2106.2	0.008	0.30
10.05	71.283	2113.8	0.008	0.29
10.10	71.485	2121.5	0.008	0.28
10.15	71.687	2129.1	0.008	0.28

<u>Ex Basin</u>		$Q_{Sub\ Drain} =$	0.000	cfs
Elevation	Q_{AVG} (CFS)	DV (CF)	DT (HR)	Total T
10.20	71.888	2136.8	0.008	0.27
10.25	72.088	2144.4	0.008	0.26
10.30	72.288	2152.1	0.008	0.25
10.35	72.488	2159.7	0.008	0.24
10.40	72.686	2167.4	0.008	0.23
10.45	72.885	2175.0	0.008	0.23
10.50	73.083	2182.7	0.008	0.22
10.55	73.280	2190.3	0.008	0.21
10.60	73.477	2198.0	0.008	0.20
10.65	73.673	2205.6	0.008	0.19
10.70	73.868	2213.3	0.008	0.19
10.75	74.064	2220.9	0.008	0.18
10.80	74.258	2228.6	0.008	0.17
10.85	74.452	2236.2	0.008	0.16
10.90	74.646	2243.9	0.008	0.15
10.95	77.222	2251.5	0.008	0.14
11.00	81.771	2259.4	0.007	0.14
11.05	87.605	2267.6	0.007	0.13
11.10	94.478	2275.7	0.006	0.12
11.15	102.247	2283.8	0.006	0.11
11.20	110.817	2292.0	0.006	0.11
11.25	120.117	2300.1	0.005	0.10
11.30	130.093	2308.2	0.005	0.10
11.35	140.701	2316.4	0.004	0.09
11.40	151.905	2324.5	0.004	0.09
11.45	163.673	2332.6	0.004	0.09
11.50	175.981	2340.7	0.004	0.08
11.55	188.804	2348.9	0.003	0.08
11.60	202.122	2357.0	0.003	0.07
11.65	215.918	2365.1	0.003	0.07
11.70	230.175	2373.3	0.003	0.07
11.75	244.878	2381.4	0.003	0.07
11.80	260.014	2389.5	0.002	0.06
11.85	275.572	2397.7	0.002	0.06
11.90	291.538	2405.8	0.002	0.06
11.95	307.904	2413.9	0.002	0.06
12.00	324.660	2422.4	0.002	0.05
12.05	341.796	2431.2	0.002	0.05
12.10	359.305	2440.1	0.002	0.05
12.15	377.178	2448.9	0.002	0.05
12.20	395.408	2457.7	0.002	0.05
12.25	413.988	2466.5	0.002	0.04
12.30	432.911	2475.3	0.002	0.04
12.35	452.172	2484.2	0.002	0.04
12.40	463.944	2493.0	0.001	0.04
12.45	470.466	2501.8	0.001	0.04
12.50	476.887	2510.6	0.001	0.04
12.55	483.210	2519.5	0.001	0.04
12.60	489.440	2528.3	0.001	0.03
12.65	495.582	2537.1	0.001	0.03
12.70	501.639	2545.9	0.001	0.03

Ex Basin		$Q_{\text{Sub Drain}} =$	0.000	cfs
Elevation	Q_{AVG} (CFS)	DV (CF)	DT (HR)	Total T
12.75	507.614	2554.7	0.001	0.03
12.80	513.511	2563.6	0.001	0.03
12.85	519.333	2572.4	0.001	0.03
12.90	525.083	2581.2	0.001	0.03
12.95	530.763	2590.0	0.001	0.02
13.00	536.375	2598.8	0.001	0.02
13.05	541.924	2607.7	0.001	0.02
13.10	547.409	2616.5	0.001	0.02
13.15	552.834	2625.3	0.001	0.02
13.20	558.201	2634.1	0.001	0.02
13.25	563.512	2643.0	0.001	0.02
13.30	568.767	2651.8	0.001	0.02
13.35	573.970	2660.6	0.001	0.01
13.40	579.121	2669.4	0.001	0.01
13.45	584.222	2678.2	0.001	0.01
13.50	589.275	2687.1	0.001	0.01
13.55	594.281	2695.9	0.001	0.01
13.60	599.241	2704.7	0.001	0.01
13.65	604.156	2713.5	0.001	0.01
13.70	609.029	2722.3	0.001	0.00
13.75	613.859	2731.2	0.001	0.00
13.80	618.648	2740.0	0.001	0.00
13.85	623.397	2748.8	0.001	0.00
13.90	628.107	0.0	0.000	0.00

Otay Village 7 R-8, Ex Basin
Discharge vs Elevation Table

1 Orifice Stag diameter: 10"	2 Orifice Stag diameter: 12"	3 Orifice Stag diameter: 18"	4 Orifice Stag diameter: 18"	Emergency v	8X8
Number: 1	Number of orif: 1	Number: 2	Number: 3	Invert: 11.50 ft	
Cg-low: 0.61	Cg-middle: 0.61	Cg-low: 0.61	Cg-low: 0.61	Area: 64.00 sq ft	8' x 8'
Invert elev: 0.00 ft	Invert elev: 1.50 ft	Invert elev: 3.00 ft	Invert elev: 5.50 ft	Circumferen:	32 ft

Elev

	h (ft)	H/D-1	H/D-2	H/D-3	H/D-4	H/D-peak	Olow-orif (cfs)	Olow-weir (cfs)	Otot-1 (cfs)	Omid-orif (cfs)	Omid-weir (cfs)	Otot-2 (cfs)	Otop-orif (cfs)	Otop-weir (cfs)	Otot-3 (cfs)	Otop-orif (cfs)	Otop-weir (cfs)	Otot-4 (cfs)	O emergency (cfs)	Otot (cfs)
390	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.000
390.05	0.05	0.06	0.00	0.00	0.00	0.00	0.00	0.01	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.008
390.10	0.10	0.12	0.00	0.00	0.00	0.00	0.00	0.03	0.03	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.031
390.15	0.15	0.18	0.00	0.00	0.00	0.00	0.00	0.07	0.07	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.069
390.20	0.20	0.24	0.00	0.00	0.00	0.00	0.00	0.12	0.12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.120
390.25	0.25	0.30	0.00	0.00	0.00	0.00	0.00	0.18	0.18	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.184
390.30	0.30	0.36	0.00	0.00	0.00	0.00	0.00	0.26	0.26	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.262
390.35	0.35	0.42	0.00	0.00	0.00	0.00	0.00	0.35	0.35	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.350
390.40	0.40	0.48	0.00	0.00	0.00	0.00	0.00	0.45	0.45	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.450
390.45	0.45	0.54	0.00	0.00	0.00	0.00	0.49	0.56	0.56	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.560
390.50	0.50	0.60	0.00	0.00	0.00	0.00	0.77	0.68	0.68	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.680
390.55	0.55	0.66	0.00	0.00	0.00	0.00	0.97	0.81	0.81	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.908
390.60	0.60	0.72	0.00	0.00	0.00	0.00	1.14	0.94	0.94	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.943
390.65	0.65	0.78	0.00	0.00	0.00	0.00	1.29	1.08	1.08	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.085
390.70	0.70	0.84	0.00	0.00	0.00	0.00	1.42	1.23	1.23	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.232
390.75	0.75	0.90	0.00	0.00	0.00	0.00	1.54	1.38	1.38	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.385
390.80	0.80	0.96	0.00	0.00	0.00	0.00	1.86	1.54	1.54	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.541
390.85	0.85	1.02	0.00	0.00	0.00	0.00	1.76	1.70	1.70	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.700
390.90	0.90	1.08	0.00	0.00	0.00	0.00	1.86	1.86	1.86	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.861
390.95	0.95	1.14	0.00	0.00	0.00	0.00	1.95	2.02	1.95	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.950
391.00	1.00	1.20	0.00	0.00	0.00	0.00	2.04	2.18	2.04	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.039
391.05	1.05	1.26	0.00	0.00	0.00	0.00	2.12	2.35	2.12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.125
391.10	1.10	1.32	0.00	0.00	0.00	0.00	2.21	2.50	2.21	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.207
391.15	1.15	1.38	0.00	0.00	0.00	0.00	2.29	2.66	2.29	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.286
391.20	1.20	1.44	0.00	0.00	0.00	0.00	2.36	2.81	2.36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.363
391.25	1.25	1.50	0.00	0.00	0.00	0.00	2.44	2.96	2.44	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.437
391.30	1.30	1.56	0.00	0.00	0.00	0.00	2.51	3.11	2.51	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.509
391.35	1.35	1.62	0.00	0.00	0.00	0.00	2.58	3.25	2.58	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.579
391.40	1.40	1.68	0.00	0.00	0.00	0.00	2.65	3.38	2.65	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.648
391.45	1.45	1.74	0.00	0.00	0.00	0.00	2.71	3.51	2.71	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.714
391.50	1.50	1.80	0.00	0.00	0.00	0.00	2.78	3.63	2.78	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.779
391.55	1.55	1.86	0.05	0.00	0.00	0.00	2.84	3.74	2.84	0.00	0.01	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.851
391.60	1.60	1.92	0.10	0.00	0.00	0.00	2.90	3.84	2.90	0.00	0.03	0.03	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.939
391.65	1.65	1.98	0.15	0.00	0.00	0.00	2.97	3.93	2.97	0.00	0.08	0.08	0.00	0.00	0.00	0.00	0.00	0.00	0.00	3.041
391.70	1.70	2.04	0.20	0.00	0.00	0.00	3.02	4.02	3.02	0.00	0.13	0.13	0.00	0.00	0.00	0.00	0.00	0.00	0.00	3.157
391.75	1.75	2.10	0.25	0.00	0.00	0.00	3.08	4.10	3.08	0.00	0.20	0.20	0.00	0.00	0.00	0.00	0.00	0.00	0.00	3.288
391.80	1.80	2.16	0.30	0.00	0.00	0.00	3.14	4.17	3.14	0.00	0.29	0.29	0.00	0.00	0.00	0.00	0.00	0.00	0.00	3.431
391.85	1.85	2.22	0.35	0.00	0.00	0.00	3.20	4.23	3.20	0.00	0.39	0.39	0.00	0.00	0.00	0.00	0.00	0.00	0.00	3.588
391.90	1.90	2.28	0.40	0.00	0.00	0.00	3.25	4.28	3.25	0.00	0.50	0.50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	3.756
391.95	1.95	2.34	0.45	0.00	0.00	0.00	3.31	4.33	3.31	0.00	0.63	0.63	0.00	0.00	0.00	0.00	0.00	0.00	0.00	3.936
392.00	2.00	2.40	0.50	0.00	0.00	0.00	3.36	4.36	3.36	0.00	0.77	0.77	0.00	0.00	0.00	0.00	0.00	0.00	0.00	4.126
392.05	2.05	2.46	0.55	0.00	0.00	0.00	3.41	4.39	3.41	0.86	0.91	0.91	0.00	0.00	0.00	0.00	0.00	0.00	0.00	4.327
392.10	2.10	2.52	0.60	0.00	0.00	0.00	3.46	4.41	3.46	1.22	1.07	1.07	0.00	0.00	0.00	0.00	0.00	0.00	0.00	4.536
392.15	2.15	2.58	0.65	0.00	0.00	0.00	3.52	4.43	3.52	1.49	1.24	1.24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	4.754
392.20	2.20	2.64	0.70	0.00	0.00	0.00	3.57	4.44	3.57	1.72	1.41	1.41	0.00	0.00	0.00	0.00	0.00	0.00	0.00	4.980
392.25	2.25	2.70	0.75	0.00	0.00	0.00	3.62	4.45	3.62	1.92	1.60	1.60	0.00	0.00	0.00	0.00	0.00	0.00	0.00	5.213
392.30	2.30	2.76	0.80	0.00	0.00	0.00	3.66	4.46	3.66	2.11	1.79	1.79	0.00	0.00	0.00	0.00	0.00	0.00	0.00	5.452
392.35	2.35	2.82	0.85	0.00	0.00	0.00	3.71	4.46	3.71	2.27	1.98	1.98	0.00	0.00	0.00	0.00	0.00	0.00	0.00	5.696
392.40	2.40	2.88	0.90	0.00	0.00	0.00	3.76	4.46	3.76	2.43	2.18	2.18	0.00	0.00	0.00	0.00	0.00	0.00	0.00	5.944
392.45	2.45	2.94	0.95	0.00	0.00	0.00	3.81	4.47	3.81	2.58	2.39	2.39	0.00	0.00	0.00	0.00	0.00	0.00	0.00	6.196
392.50	2.50	3.00	1.00	0.00	0.00	0.00	3.85	4.47	3.85	2.72	2.60	2.60	0.00	0.00	0.00	0.00	0.00	0.00	0.00	6.451
392.55	2.55	3.06	1.05	0.00	0.00	0.00	3.90	4.49	3.90	2.85	2.81	2.81	0.00	0.00	0.00	0.00	0.00	0.00	0.00	6.708
392.60	2.60	3.12	1.10	0.00	0.00	0.00	3.95	4.51	3.95	2.98	3.02	2.98	0.00	0.00	0.00	0.00	0.00	0.00	0.00	6.923
392.65	2.65	3.18	1.15	0.00	0.00	0.00	3.99	4.54	3.99	3.10	3.23	3.10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	7.090
392.70	2.70	3.24	1.20	0.00	0.00	0.00	4.03	4.58	4.03	3.22	3.45	3.22	0.00	0.00	0.00	0.00	0.00	0.00	0.00	7.251
392.75	2.75	3.30	1.25	0.00	0.00	0.00	4.08	4.64	4.08	3.33	3.66	3.33	0.00	0.00	0.00	0.00	0.00	0.00	0.00	7.408
392.80	2.80	3.36	1.30	0.00	0.00	0.00	4.12	4.71	4.12	3.44	3.87	3.44	0.00	0.00	0.00	0.00	0.00	0.00	0.00	7.561
392.85	2.85	3.42	1.35	0.00	0.00	0.00	4.16	4.80	4.16	3.54	4.08	3.54	0.00	0.00	0.00	0.00	0.00	0.00	0.00	7.710
392.90	2.90	3.48	1.40	0.00	0.00	0.00	4.21	4.92	4.21	3.65	4.28	3.65	0.00	0.00	0.00	0.00	0.00	0.00	0.00	7.855

390.20	0.20	0.24	0.00	0.00	0.00	0.00	0.00	0.12	0.12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.120	
390.25	0.25	0.30	0.00	0.00	0.00	0.00	0.00	0.18	0.18	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.184	
402.15	12.15	14.58	10.65	6.10	4.43	0.12	9.15	22706.15	9.15	12.25	6156.96	12.25	50.14	1058.36	50.14	63.04	173.25	63.04	111.68	246.259
402.20	12.20	14.64	10.70	6.13	4.47	0.13	9.17	23222.63	9.17	12.28	6325.25	12.28	50.29	1098.44	50.29	63.30	181.77	63.30	124.82	259.856
402.25	12.25	14.70	10.75	6.17	4.50	0.14	9.18	23748.41	9.18	12.31	6497.16	12.31	50.44	1139.74	50.44	63.57	190.75	63.57	138.43	273.929
402.30	12.30	14.76	10.80	6.20	4.53	0.15	9.20	24283.61	9.20	12.34	6672.74	12.34	50.59	1182.26	50.59	63.83	200.22	63.83	152.50	288.461
402.35	12.35	14.82	10.85	6.23	4.57	0.16	9.22	24828.35	9.22	12.37	6852.05	12.37	50.74	1226.04	50.74	64.10	210.18	64.10	167.01	303.439
402.40	12.40	14.88	10.90	6.27	4.60	0.17	9.24	25382.76	9.24	12.40	7035.16	12.40	50.88	1271.11	50.88	64.36	220.66	64.36	181.97	318.849
402.45	12.45	14.94	10.95	6.30	4.63	0.18	9.26	25946.97	9.26	12.43	7222.12	12.43	51.03	1317.48	51.03	64.62	231.67	64.62	197.34	334.678
402.50	12.50	15.00	11.00	6.33	4.67	0.19	9.28	26521.11	9.28	12.46	7412.99	12.46	51.18	1365.20	51.18	64.88	243.24	64.88	213.12	350.916
402.55	12.55	15.06	11.05	6.37	4.70	0.20	9.30	27105.31	9.30	12.49	7607.83	12.49	51.32	1414.27	51.32	65.14	255.37	65.14	229.30	367.552
402.60	12.60	15.12	11.10	6.40	4.73	0.21	9.32	27699.69	9.32	12.52	7806.71	12.52	51.47	1464.74	51.47	65.40	268.10	65.40	245.87	384.577
402.65	12.65	15.18	11.15	6.43	4.77	0.22	9.34	28304.39	9.34	12.55	8009.68	12.55	51.61	1516.64	51.61	65.65	281.43	65.65	262.83	401.981
402.70	12.70	15.24	11.20	6.47	4.80	0.23	9.36	28919.54	9.36	12.58	8216.80	12.58	51.76	1569.98	51.76	65.91	295.39	65.91	280.15	419.756
402.75	12.75	15.30	11.25	6.50	4.83	0.23	9.38	29545.28	9.38	12.61	8428.14	12.61	51.90	1624.80	51.90	66.16	310.00	66.16	297.84	437.894
402.80	12.80	15.36	11.30	6.53	4.87	0.24	9.40	30181.74	9.40	12.63	8643.77	12.63	52.05	1681.13	52.05	66.42	325.29	66.42	315.89	456.389
402.85	12.85	15.42	11.35	6.57	4.90	0.25	9.41	30829.05	9.41	12.66	8863.74	12.66	52.19	1738.99	52.19	66.67	341.26	66.67	334.29	475.232
402.90	12.90	15.48	11.40	6.60	4.93	0.26	9.43	31487.86	9.43	12.69	9088.12	12.69	52.33	1798.43	52.33	66.92	357.95	66.92	353.03	494.418
402.95	12.95	15.54	11.45	6.63	4.97	0.27	9.45	32156.81	9.45	12.72	9316.97	12.72	52.48	1859.46	52.48	67.17	375.37	67.17	372.11	513.940
403.00	13.00	15.60	11.50	6.67	5.00	0.28	9.47	32837.53	9.47	12.75	9550.37	12.75	52.62	1922.12	52.62	67.42	393.54	67.42	383.71	532.972
403.05	13.05	15.66	11.55	6.70	5.03	0.29	9.49	33529.66	9.49	12.78	9788.37	12.78	52.76	1986.44	52.76	67.67	412.50	67.67	390.05	552.754
403.10	13.10	15.72	11.60	6.73	5.07	0.30	9.51	34233.34	9.51	12.81	10031.05	12.81	52.90	2052.45	52.90	67.92	432.25	67.92	396.29	573.433
403.15	13.15	15.78	11.65	6.77	5.10	0.31	9.53	34948.73	9.53	12.84	10278.48	12.84	53.04	2120.18	53.04	68.17	452.83	68.17	402.43	596.013
403.20	13.20	15.84	11.70	6.80	5.13	0.32	9.55	35675.96	9.55	12.87	10530.71	12.87	53.19	2189.68	53.19	68.42	474.26	68.42	408.49	618.500
403.25	13.25	15.90	11.75	6.83	5.17	0.33	9.56	36415.18	9.56	12.90	10787.82	12.90	53.33	2260.96	53.33	68.66	496.57	68.66	414.45	642.897
403.30	13.30	15.96	11.80	6.87	5.20	0.34	9.58	37166.53	9.58	12.92	11049.89	12.92	53.47	2334.06	53.47	68.91	519.77	68.91	420.33	668.209
403.35	13.35	16.02	11.85	6.90	5.23	0.35	9.60	37930.17	9.60	12.95	11316.97	12.95	53.61	2409.01	53.61	69.15	543.89	69.15	426.13	694.457
403.40	13.40	16.08	11.90	6.93	5.27	0.36	9.62	38706.24	9.62	12.98	11599.15	12.98	53.75	2485.86	53.75	69.39	568.97	69.39	431.85	721.587
403.45	13.45	16.14	11.95	6.97	5.30	0.37	9.64	39494.89	9.64	13.01	11866.48	13.01	53.88	2564.63	53.88	69.64	595.01	69.64	437.49	749.661
403.50	13.50	16.20	12.00	7.00	5.33	0.38	9.66	40296.28	9.66	13.04	12149.06	13.04	54.02	2645.35	54.02	69.88	622.06	69.88	443.07	778.661
403.55	13.55	16.26	12.05	7.03	5.37	0.38	9.68	41110.56	9.68	13.07	12436.94	13.07	54.16	2728.07	54.16	70.12	650.14	70.12	448.57	808.590
403.60	13.60	16.32	12.10	7.07	5.40	0.39	9.69	41937.87	9.69	13.09	12730.20	13.09	54.30	2812.82	54.30	70.36	679.26	70.36	454.01	839.458
403.65	13.65	16.38	12.15	7.10	5.43	0.40	9.71	42778.39	9.71	13.12	13028.92	13.12	54.44	2899.63	54.44	70.60	709.48	70.60	459.38	871.248
403.70	13.70	16.44	12.20	7.13	5.47	0.41	9.73	43632.26	9.73	13.15	13333.17	13.15	54.57	2988.54	54.57	70.83	740.80	70.83	464.69	904.981
403.75	13.75	16.50	12.25	7.17	5.50	0.42	9.75	44499.63	9.75	13.18	13643.03	13.18	54.71	3079.59	54.71	71.07	773.26	71.07	469.94	940.653
403.80	13.80	16.56	12.30	7.20	5.53	0.43	9.77	45380.68	9.77	13.21	13958.57	13.21	54.85	3172.82	54.85	71.31	806.89	71.31	475.13	978.265
403.85	13.85	16.62	12.35	7.23	5.57	0.44	9.79	46275.56	9.79	13.23	14279.87	13.23	54.98	3268.25	54.98	71.54	841.71	71.54	480.27	1017.820
403.90	13.90	16.68	12.40	7.27	5.60	0.45	9.80	47184.42	9.80	13.26	14607.00	13.26	55.12	3365.94	55.12	71.78	877.76	71.78	485.35	1059.319

Otay Ranch Village 7 R-8
/ Stage Storage

Ex Detention Bain	
Depth (ft)	Area (sqft)
0.00	17535
0.05	17647
0.10	17758
0.15	17870
0.20	17982
0.25	18093
0.30	18205
0.35	18317
0.40	18428
0.45	18540
0.50	18651
0.55	18763
0.60	18875
0.65	18986
0.70	19098
0.75	19210
0.80	19321
0.85	19433
0.90	19545
0.95	19656
1.00	19768
1.05	19873
1.10	19979
1.15	20085
1.20	20190
1.25	20296
1.30	20401
1.35	20507
1.40	20613
1.45	20718
1.50	20824
1.55	20929
1.60	21035
1.65	21140
1.70	21246
1.75	21352
1.80	21457
1.85	21563
1.90	21668
1.95	21774
2.00	21880
2.05	21991

Depth (ft)	Area (sqft)
2.10	22102
2.15	22213
2.20	22325
2.25	22436
2.30	22547
2.35	22659
2.40	22770
2.45	22881
2.50	22993
2.55	23104
2.60	23215
2.65	23326
2.70	23438
2.75	23549
2.80	23660
2.85	23772
2.90	23883
2.95	23994
3.00	24106
3.05	24215
3.10	24325
3.15	24435
3.20	24545
3.25	24655
3.30	24765
3.35	24875
3.40	24985
3.45	25095
3.50	25205
3.55	25315
3.60	25425
3.65	25535
3.70	25645
3.75	25755
3.80	25865
3.85	25975
3.90	26085
3.95	26195
4.00	26305
4.05	26420
4.10	26536
4.15	26652
4.20	26768
4.25	26883
4.30	26999

Depth (ft)	Area (sqft)
4.35	27115
4.40	27230
4.45	27346
4.50	27462
4.55	27578
4.60	27693
4.65	27809
4.70	27925
4.75	28040
4.80	28156
4.85	28272
4.90	28388
4.95	28503
5.00	28619
5.05	28739
5.10	28860
5.15	28980
5.20	29101
5.25	29221
5.30	29342
5.35	29462
5.40	29582
5.45	29703
5.50	29823
5.55	29944
5.60	30064
5.65	30184
5.70	30305
5.75	30425
5.80	30546
5.85	30666
5.90	30787
5.95	30907
6.00	31027
6.05	31160
6.10	31292
6.15	31424
6.20	31557
6.25	31689
6.30	31821
6.35	31954
6.40	32086
6.45	32219
6.50	32351
6.55	32483

Depth (ft)	Area (sqft)
6.60	32616
6.65	32748
6.70	32880
6.75	33013
6.80	33145
6.85	33277
6.90	33410
6.95	33542
7.00	33674
7.05	33810
7.10	33945
7.15	34081
7.20	34216
7.25	34352
7.30	34487
7.35	34623
7.40	34758
7.45	34894
7.50	35029
7.55	35165
7.60	35300
7.65	35436
7.70	35571
7.75	35707
7.80	35842
7.85	35977
7.90	36113
7.95	36248
8.00	36384
8.05	36523
8.10	36662
8.15	36801
8.20	36941
8.25	37080
8.30	37219
8.35	37358
8.40	37497
8.45	37636
8.50	37776
8.55	37915
8.60	38054
8.65	38193
8.70	38332
8.75	38471
8.80	38611

Depth (ft)	Area (sqft)
8.85	38750
8.90	38889
8.95	39028
9.00	39167
9.05	39311
9.10	39455
9.15	39599
9.20	39743
9.25	39887
9.30	40031
9.35	40175
9.40	40319
9.45	40463
9.50	40607
9.55	40751
9.60	40895
9.65	41039
9.70	41183
9.75	41327
9.80	41471
9.85	41615
9.90	41759
9.95	41903
10.00	42047
10.05	42200
10.10	42353
10.15	42506
10.20	42659
10.25	42812
10.30	42965
10.35	43118
10.40	43271
10.45	43424
10.50	43577
10.55	43730
10.60	43883
10.65	44036
10.70	44189
10.75	44342
10.80	44495
10.85	44648
10.90	44801
10.95	44954
11.00	45107
11.05	45270

Depth (ft)	Area (sqft)
11.10	45432
11.15	45595
11.20	45758
11.25	45920
11.30	46083
11.35	46246
11.40	46408
11.45	46571
11.50	46734
11.55	46896
11.60	47059
11.65	47222
11.70	47384
11.75	47547
11.80	47710
11.85	47872
11.90	48035
11.95	48198
12.00	48360
12.05	48537
12.10	48713
12.15	48889
12.20	49066
12.25	49242
12.30	49419
12.35	49595
12.40	49772
12.45	49948
12.50	50124
12.55	50301
12.60	50477
12.65	50654
12.70	50830
12.75	51007
12.80	51183
12.85	51359
12.90	51536
12.95	51712
13.00	51889
13.05	52065
13.10	52242
13.15	52418
13.20	52594
13.25	52771
13.30	52947

Depth (ft)	Area (sqft)
13.35	53124
13.40	53300
13.45	53477
13.50	53653
13.55	53829
13.60	54006
13.65	54182
13.70	54359
13.75	54535
13.80	54712
13.85	54888
13.90	55064

Ex Basin		Q _{Sub Drain} =	0.000	cfs
Elevation	Q _{AVG} (CFS)	DV (CF)	DT (HR)	Total T
0.00	0.000	879.5	60.807	93.32
0.05	0.008	885.1	12.577	32.51
0.10	0.031	890.7	4.968	19.94
0.15	0.069	896.3	2.642	14.97
0.20	0.120	901.9	1.646	12.33
0.25	0.184	907.5	1.130	10.68
0.30	0.262	913.0	0.829	9.55
0.35	0.350	918.6	0.637	8.72
0.40	0.450	924.2	0.508	8.08
0.45	0.560	929.8	0.417	7.58
0.50	0.680	935.4	0.349	7.16
0.55	0.808	940.9	0.299	6.81
0.60	0.943	946.5	0.259	6.51
0.65	1.085	952.1	0.228	6.25
0.70	1.232	957.7	0.203	6.02
0.75	1.385	963.3	0.183	5.82
0.80	1.541	968.9	0.166	5.64
0.85	1.700	974.4	0.152	5.47
0.90	1.861	980.0	0.143	5.32
0.95	1.950	985.6	0.137	5.18
1.00	2.039	991.0	0.132	5.04
1.05	2.125	996.3	0.128	4.91
1.10	2.207	1001.6	0.124	4.78
1.15	2.286	1006.9	0.120	4.65
1.20	2.363	1012.1	0.117	4.53
1.25	2.437	1017.4	0.114	4.42
1.30	2.509	1022.7	0.112	4.30
1.35	2.579	1028.0	0.109	4.19
1.40	2.648	1033.3	0.107	4.08
1.45	2.714	1038.5	0.105	3.98
1.50	2.779	1043.8	0.103	3.87
1.55	2.851	1049.1	0.101	3.77
1.60	2.939	1054.4	0.098	3.67
1.65	3.041	1059.7	0.095	3.57
1.70	3.157	1064.9	0.092	3.47
1.75	3.288	1070.2	0.088	3.38
1.80	3.431	1075.5	0.085	3.29
1.85	3.588	1080.8	0.082	3.21
1.90	3.756	1086.1	0.078	3.13
1.95	3.936	1091.3	0.075	3.05
2.00	4.126	1096.8	0.072	2.97
2.05	4.327	1102.3	0.069	2.90
2.10	4.536	1107.9	0.066	2.83
2.15	4.754	1113.5	0.064	2.77
2.20	4.980	1119.0	0.061	2.70
2.25	5.213	1124.6	0.059	2.64
2.30	5.452	1130.1	0.056	2.58
2.35	5.696	1135.7	0.054	2.53
2.40	5.944	1141.3	0.052	2.47
2.45	6.196	1146.8	0.050	2.42
2.50	6.451	1152.4	0.049	2.37

Ex Basin		Q _{Sub Drain} =	0.000	cfs
Elevation	Q _{AVG} (CFS)	DV (CF)	DT (HR)	Total T
2.55	6.708	1158.0	0.047	2.32
2.60	6.923	1163.5	0.046	2.27
2.65	7.090	1169.1	0.045	2.23
2.70	7.251	1174.7	0.045	2.18
2.75	7.408	1180.2	0.044	2.14
2.80	7.561	1185.8	0.043	2.09
2.85	7.710	1191.4	0.043	2.05
2.90	7.855	1196.9	0.042	2.01
2.95	7.997	1202.5	0.041	1.97
3.00	8.136	1208.0	0.041	1.92
3.05	8.295	1213.5	0.040	1.88
3.10	8.492	1219.0	0.039	1.84
3.15	8.726	1224.5	0.038	1.80
3.20	8.998	1230.0	0.037	1.77
3.25	9.306	1235.5	0.036	1.73
3.30	9.649	1241.0	0.035	1.69
3.35	10.027	1246.5	0.034	1.66
3.40	10.439	1252.0	0.033	1.62
3.45	10.884	1257.5	0.031	1.59
3.50	11.360	1263.0	0.030	1.56
3.55	11.868	1268.5	0.029	1.53
3.60	12.405	1274.0	0.028	1.50
3.65	12.971	1279.5	0.027	1.47
3.70	13.565	1285.0	0.026	1.45
3.75	14.185	1290.5	0.025	1.42
3.80	14.831	1296.0	0.024	1.39
3.85	15.500	1301.5	0.023	1.37
3.90	16.192	1307.0	0.022	1.35
3.95	16.906	1312.5	0.021	1.33
4.00	17.639	1318.1	0.020	1.31
4.05	18.392	1323.9	0.020	1.28
4.10	19.162	1329.7	0.019	1.27
4.15	19.948	1335.5	0.018	1.25
4.20	20.750	1341.3	0.018	1.23
4.25	21.564	1347.1	0.017	1.21
4.30	22.391	1352.8	0.016	1.19
4.35	23.228	1358.6	0.016	1.18
4.40	24.074	1364.4	0.015	1.16
4.45	24.929	1370.2	0.015	1.15
4.50	25.789	1376.0	0.015	1.13
4.55	26.655	1381.8	0.014	1.12
4.60	27.525	1387.6	0.014	1.10
4.65	28.397	1393.3	0.014	1.09
4.70	28.706	1399.1	0.013	1.07
4.75	29.235	1404.9	0.013	1.06
4.80	29.752	1410.7	0.013	1.05
4.85	30.258	1416.5	0.013	1.03
4.90	30.754	1422.3	0.013	1.02
4.95	31.241	1428.1	0.013	1.01
5.00	31.718	1434.0	0.012	1.00
5.05	32.188	1440.0	0.012	0.98

Ex Basin		$Q_{\text{Sub Drain}} =$	0.000	cfs
Elevation	Q_{AVG} (CFS)	DV (CF)	DT (HR)	Total T
5.10	32.649	1446.0	0.012	0.97
5.15	33.103	1452.0	0.012	0.96
5.20	33.550	1458.0	0.012	0.95
5.25	33.990	1464.1	0.012	0.94
5.30	34.424	1470.1	0.012	0.92
5.35	34.852	1476.1	0.012	0.91
5.40	35.273	1482.1	0.012	0.90
5.45	35.689	1488.2	0.012	0.89
5.50	36.100	1494.2	0.011	0.88
5.55	36.539	1500.2	0.011	0.87
5.60	37.034	1506.2	0.011	0.85
5.65	37.585	1512.2	0.011	0.84
5.70	38.190	1518.3	0.011	0.83
5.75	38.848	1524.3	0.011	0.82
5.80	39.558	1530.3	0.011	0.81
5.85	40.320	1536.3	0.010	0.80
5.90	41.130	1542.3	0.010	0.79
5.95	41.989	1548.4	0.010	0.78
6.00	42.895	1554.7	0.010	0.77
6.05	43.845	1561.3	0.010	0.76
6.10	44.840	1567.9	0.010	0.75
6.15	45.876	1574.5	0.009	0.74
6.20	46.952	1581.1	0.009	0.73
6.25	48.067	1587.8	0.009	0.72
6.30	49.218	1594.4	0.009	0.71
6.35	50.405	1601.0	0.009	0.70
6.40	51.624	1607.6	0.009	0.69
6.45	52.874	1614.2	0.008	0.69
6.50	54.154	1620.9	0.008	0.68
6.55	55.461	1627.5	0.008	0.67
6.60	56.793	1634.1	0.008	0.66
6.65	58.148	1640.7	0.008	0.65
6.70	59.524	1647.3	0.008	0.65
6.75	60.919	1653.9	0.007	0.64
6.80	62.332	1660.6	0.007	0.63
6.85	63.759	1667.2	0.007	0.62
6.90	65.199	1673.8	0.007	0.62
6.95	66.650	1680.4	0.007	0.61
7.00	68.110	1687.1	0.007	0.60
7.05	69.577	1693.9	0.007	0.59
7.10	71.048	1700.7	0.007	0.59
7.15	72.522	1707.4	0.007	0.58
7.20	73.151	1714.2	0.006	0.58
7.25	74.108	1721.0	0.006	0.57
7.30	75.047	1727.7	0.006	0.56
7.35	75.968	1734.5	0.006	0.56
7.40	76.874	1741.3	0.006	0.55
7.45	77.764	1748.1	0.006	0.54
7.50	78.641	1754.8	0.006	0.54
7.55	79.504	1761.6	0.006	0.53
7.60	80.355	1768.4	0.006	0.52

Ex Basin		$Q_{\text{Sub Drain}} =$	0.000	cfs
Elevation	Q_{AVG} (CFS)	DV (CF)	DT (HR)	Total T
7.65	81.193	1775.2	0.006	0.52
7.70	82.020	1781.9	0.006	0.51
7.75	82.836	1788.7	0.006	0.51
7.80	83.642	1795.5	0.006	0.50
7.85	84.437	1802.3	0.006	0.49
7.90	85.223	1809.0	0.006	0.49
7.95	86.000	1815.8	0.006	0.48
8.00	86.768	1822.7	0.006	0.48
8.05	87.528	1829.6	0.006	0.47
8.10	88.279	1836.6	0.006	0.47
8.15	89.023	1843.6	0.006	0.46
8.20	89.758	1850.5	0.006	0.45
8.25	90.487	1857.5	0.006	0.45
8.30	91.208	1864.4	0.006	0.44
8.35	91.923	1871.4	0.006	0.44
8.40	92.631	1878.3	0.006	0.43
8.45	93.332	1885.3	0.006	0.43
8.50	94.028	1892.3	0.006	0.42
8.55	94.717	1899.2	0.006	0.41
8.60	95.400	1906.2	0.006	0.41
8.65	96.078	1913.1	0.006	0.40
8.70	96.750	1920.1	0.005	0.40
8.75	97.416	1927.1	0.005	0.39
8.80	98.077	1934.0	0.005	0.39
8.85	98.734	1941.0	0.005	0.38
8.90	99.385	1947.9	0.005	0.38
8.95	100.031	1954.9	0.005	0.37
9.00	100.672	1962.0	0.005	0.37
9.05	101.309	1969.2	0.005	0.36
9.10	101.941	1976.4	0.005	0.35
9.15	102.569	1983.6	0.005	0.35
9.20	103.193	1990.8	0.005	0.34
9.25	103.812	1998.0	0.005	0.34
9.30	104.427	2005.2	0.005	0.33
9.35	105.038	2012.4	0.005	0.33
9.40	105.645	2019.6	0.005	0.32
9.45	106.248	2026.8	0.005	0.32
9.50	106.848	2034.0	0.005	0.31
9.55	107.443	2041.2	0.005	0.31
9.60	108.035	2048.4	0.005	0.30
9.65	108.624	2055.6	0.005	0.30
9.70	109.209	2062.8	0.005	0.29
9.75	109.790	2070.0	0.005	0.29
9.80	110.368	2077.2	0.005	0.28
9.85	110.943	2084.4	0.005	0.28
9.90	111.515	2091.6	0.005	0.27
9.95	112.083	2098.8	0.005	0.26
10.00	112.648	2106.2	0.005	0.26
10.05	113.210	2113.8	0.005	0.25
10.10	113.769	2121.5	0.005	0.25
10.15	114.325	2129.1	0.005	0.24

Ex Basin		$Q_{\text{Sub Drain}} =$	0.000	cfs
Elevation	Q_{AVG} (CFS)	DV (CF)	DT (HR)	Total T
10.20	114.878	2136.8	0.005	0.24
10.25	115.428	2144.4	0.005	0.23
10.30	115.976	2152.1	0.005	0.23
10.35	116.520	2159.7	0.005	0.22
10.40	117.062	2167.4	0.005	0.22
10.45	117.601	2175.0	0.005	0.21
10.50	118.138	2182.7	0.005	0.21
10.55	118.672	2190.3	0.005	0.20
10.60	119.203	2198.0	0.005	0.20
10.65	119.732	2205.6	0.005	0.19
10.70	120.258	2213.3	0.005	0.19
10.75	120.782	2220.9	0.005	0.18
10.80	121.303	2228.6	0.005	0.18
10.85	121.822	2236.2	0.005	0.17
10.90	122.339	2243.9	0.005	0.17
10.95	122.853	2251.5	0.005	0.16
11.00	123.365	2259.4	0.005	0.16
11.05	123.875	2267.6	0.005	0.15
11.10	124.383	2275.7	0.005	0.15
11.15	124.888	2283.8	0.005	0.14
11.20	125.391	2292.0	0.005	0.14
11.25	125.892	2300.1	0.005	0.13
11.30	126.391	2308.2	0.005	0.13
11.35	126.888	2316.4	0.005	0.12
11.40	127.383	2324.5	0.005	0.12
11.45	127.875	2332.6	0.005	0.11
11.50	128.366	2340.7	0.005	0.11
11.55	131.238	2348.9	0.005	0.10
11.60	136.081	2357.0	0.005	0.10
11.65	142.208	2365.1	0.005	0.09
11.70	149.372	2373.3	0.004	0.09
11.75	157.431	2381.4	0.004	0.08
11.80	166.289	2389.5	0.004	0.08
11.85	175.877	2397.7	0.004	0.08
11.90	186.139	2405.8	0.003	0.07
11.95	199.282	2413.9	0.003	0.07
12.00	208.518	2422.4	0.003	0.06
12.05	220.569	2431.2	0.003	0.06
12.10	233.157	2440.1	0.003	0.06
12.15	246.259	2448.9	0.003	0.06
12.20	259.856	2457.7	0.003	0.05
12.25	273.929	2466.5	0.002	0.05
12.30	288.461	2475.3	0.002	0.05
12.35	303.439	2484.2	0.002	0.05
12.40	318.849	2493.0	0.002	0.04
12.45	334.678	2501.8	0.002	0.04
12.50	350.916	2510.6	0.002	0.04
12.55	367.552	2519.5	0.002	0.04
12.60	384.577	2528.3	0.002	0.04
12.65	401.981	2537.1	0.002	0.03
12.70	419.756	2545.9	0.002	0.03

Ex Basin		$Q_{Sub\ Drain} =$	0.000	cfs
Elevation	Q_{AVG} (CFS)	DV (CF)	DT (HR)	Total T
12.75	437.894	2554.7	0.002	0.03
12.80	456.389	2563.6	0.002	0.03
12.85	475.232	2572.4	0.001	0.03
12.90	494.418	2581.2	0.001	0.03
12.95	513.940	2590.0	0.001	0.02
13.00	525.972	2598.8	0.001	0.02
13.05	532.754	2607.7	0.001	0.02
13.10	539.433	2616.5	0.001	0.02
13.15	546.013	2625.3	0.001	0.02
13.20	552.500	2634.1	0.001	0.02
13.25	558.897	2643.0	0.001	0.02
13.30	565.209	2651.8	0.001	0.01
13.35	571.437	2660.6	0.001	0.01
13.40	577.587	2669.4	0.001	0.01
13.45	583.660	2678.2	0.001	0.01
13.50	589.661	2687.1	0.001	0.01
13.55	595.590	2695.9	0.001	0.01
13.60	601.452	2704.7	0.001	0.01
13.65	607.248	2713.5	0.001	0.01
13.70	612.981	2722.3	0.001	0.00
13.75	618.653	2731.2	0.001	0.00
13.80	624.265	2740.0	0.001	0.00
13.85	629.820	2748.8	0.001	0.00
13.90	635.319	0.0	0.000	0.00

POC 2

Vault #1 Discharge

Discharge vs Elevation Table

Low orifice:	1.922 "	Top orifice:	3 "
Number:	1	Number:	4
Cg-low:	0.61	Cg-low:	0.61
Invert elev:	0.00 ft	Invert elev:	4.00 ft
Middle orifice:	2.5 "	Emergency inlet:	
number of orif:	2	Rim height:	4.65 ft
Cg-middle:	0.61	Riser Box LxW	3x1.92 sq ft
Invert elev:	2.70 ft	Weir Length	5.75 ft

h (ft)	H/D-low	H/D-mid	H/D-top	Qlow-orif (cfs)	Qlow-weir (cfs)	Qtot-low (cfs)	Qmid-orif (cfs)	Qmid-weir (cfs)	Qtot-med (cfs)	Qtot-orif (cfs)	Qtot-weir (cfs)	Qtot-top (cfs)	Qemerg (cfs)	Qtot (cfs)
0.0	0.00	0.00	0.00	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
0.1	0.62	0.00	0.00	0.014	0.012	0.012	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.012
0.2	1.25	0.00	0.00	0.034	0.037	0.034	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.034
0.3	1.87	0.00	0.00	0.046	0.061	0.046	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.046
0.4	2.50	0.00	0.00	0.056	0.071	0.056	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.056
0.5	3.12	0.00	0.00	0.064	0.073	0.064	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.0639
0.6	3.75	0.00	0.00	0.071	0.094	0.071	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.0711
0.7	4.37	0.00	0.00	0.078	0.197	0.078	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.0776
0.8	5.00	0.00	0.00	0.084	0.485	0.084	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.0837
0.9	5.62	0.00	0.00	0.089	1.117	0.089	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.0893
1.0	6.24	0.00	0.00	0.095	2.310	0.095	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.0946
1.1	6.87	0.00	0.00	0.100	4.354	0.100	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.0996
1.2	7.49	0.00	0.00	0.104	7.619	0.104	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.1044
1.3	8.12	0.00	0.00	0.109	12.564	0.109	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.1089
1.4	8.74	0.00	0.00	0.113	19.747	0.113	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.1133
1.5	9.37	0.00	0.00	0.118	29.837	0.118	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.1175
1.6	9.99	0.00	0.00	0.122	43.617	0.122	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.1216
1.7	10.61	0.00	0.00	0.126	61.999	0.126	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.1255
1.8	11.24	0.00	0.00	0.129	86.031	0.129	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.1293
1.9	11.86	0.00	0.00	0.133	116.909	0.133	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.1330
2.0	12.49	0.00	0.00	0.137	155.979	0.137	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.1366
2.1	13.11	0.00	0.00	0.140	204.758	0.140	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.1402
2.2	13.74	0.00	0.00	0.144	264.932	0.144	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.1436
2.3	14.36	0.00	0.00	0.147	338.373	0.147	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.1469
2.4	14.99	0.00	0.00	0.150	427.143	0.150	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.1502
2.5	15.61	0.00	0.00	0.153	533.510	0.153	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.1534
2.6	16.23	0.00	0.00	0.157	659.949	0.157	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.1566
2.7	16.86	0.00	0.00	0.160	809.159	0.160	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.1596
2.8	17.48	0.48	0.00	0.163	984.067	0.163	0.000	0.028	0.028	0.000	0.000	0.000	0.000	0.1908
2.9	18.11	0.96	0.00	0.166	1187.841	0.166	0.103	0.096	0.096	0.000	0.000	0.000	0.000	0.2619
3.0	18.73	1.44	0.00	0.169	1423.896	0.169	0.148	0.176	0.148	0.000	0.000	0.000	0.000	0.3162
3.1	19.36	1.92	0.00	0.171	1695.907	0.171	0.182	0.240	0.182	0.000	0.000	0.000	0.000	0.3529
3.2	19.98	2.40	0.00	0.174	2007.816	0.174	0.210	0.273	0.210	0.000	0.000	0.000	0.000	0.3842
3.3	20.60	2.88	0.00	0.177	2363.841	0.177	0.235	0.279	0.235	0.000	0.000	0.000	0.000	0.4120
3.4	21.23	3.36	0.00	0.180	2768.486	0.180	0.258	0.294	0.258	0.000	0.000	0.000	0.000	0.4373
3.5	21.85	3.84	0.00	0.182	3226.553	0.182	0.278	0.395	0.278	0.000	0.000	0.000	0.000	0.4608
3.6	22.48	4.32	0.00	0.185	3743.147	0.185	0.298	0.707	0.298	0.000	0.000	0.000	0.000	0.4828
3.7	23.10	4.80	0.00	0.188	4323.686	0.188	0.316	1.416	0.316	0.000	0.000	0.000	0.000	0.5035
3.8	23.73	5.28	0.00	0.190	4973.915	0.190	0.333	2.776	0.333	0.000	0.000	0.000	0.000	0.5233
3.9	24.35	5.76	0.00	0.193	5699.910	0.193	0.349	5.123	0.349	0.000	0.000	0.000	0.000	0.5421
4.0	24.98	6.24	0.00	0.195	6508.090	0.195	0.365	8.878	0.365	0.000	0.000	0.000	0.000	0.5602
4.1	25.60	6.72	0.40	0.198	7405.224	0.198	0.380	14.564	0.380	0.000	0.063	0.063	0.000	0.6407
4.2	26.22	7.20	0.80	0.200	8398.446	0.200	0.394	22.809	0.394	0.263	0.223	0.223	0.000	0.8180
4.3	26.85	7.68	1.20	0.203	9495.256	0.203	0.408	34.362	0.408	0.402	0.431	0.402	0.000	1.0129
4.4	27.47	8.16	1.60	0.205	10703.536	0.205	0.422	50.098	0.422	0.504	0.631	0.504	0.000	1.1306
4.5	28.10	8.64	2.00	0.207	12031.558	0.207	0.435	71.029	0.435	0.589	0.782	0.589	0.000	1.2305
4.6	28.72	9.12	2.40	0.210	13487.992	0.210	0.447	98.314	0.447	0.662	0.860	0.662	0.000	1.3194
4.7	29.35	9.60	2.80	0.212	15081.914	0.212	0.460	133.269	0.460	0.729	0.879	0.729	0.214	1.6144
4.8	29.97	10.08	3.20	0.214	16822.820	0.214	0.471	177.378	0.471	0.790	0.897	0.790	1.112	2.5878
4.9	30.59	10.56	3.60	0.217	18720.631	0.217	0.483	232.298	0.483	0.846	1.033	0.846	2.393	3.9393
5.0	31.22	11.04	4.00	0.219	20785.704	0.219	0.495	299.874	0.495	0.899	1.476	0.899	3.965	5.5771
5.1	31.84	11.52	4.40	0.221	23028.842	0.221	0.506	382.146	0.506	0.949	2.497	0.949	5.780	7.4558
5.2	32.47	12.00	4.80	0.223	25461.303	0.223	0.517	481.359	0.517	0.997	4.466	0.997	7.810	9.5464
5.3	33.09	12.48	5.20	0.225	28094.808	0.225	0.527	599.973	0.527	1.042	7.859	1.042	10.034	11.8286
5.4	33.72	12.96	5.60	0.227	30941.553	0.227	0.538	740.673	0.538	1.085	13.272	1.085	12.437	14.2872
5.5	34.34	13.44	6.00	0.230	34014.214	0.230	0.548	906.377	0.548	1.127	21.435	1.127	15.005	16.9098
5.6	34.97	13.92	6.40	0.232	37325.962	0.232	0.558	1100.248	0.558	1.167	33.221	1.167	17.730	19.6866
5.7	35.59	14.40	6.80	0.234	40890.467	0.234	0.568	1325.704	0.568	1.206	49.663	1.206	20.601	22.6094
5.8	36.21	14.88	7.20	0.236	44721.914	0.236	0.578	1586.423	0.578	1.244	71.961	1.244	23.613	25.6709
5.9	36.84	15.36	7.60	0.238	48835.003	0.238	0.587	1886.359	0.587	1.281	101.498	1.281	26.759	28.8651
6.0	37.46	15.84	8.00	0.240	53244.966	0.240	0.597	2229.747	0.597	1.316	139.852	1.316	30.034	32.1867

Vault #1 Stage Storage

depth	area	area (ac)	elevation	volume (cf)	volume (acft)
0.0	2500	0.06	446.0	0	0.00
0.1	2500	0.06	446.1	250	0.01
0.2	2500	0.06	446.2	500	0.01
0.3	2500	0.06	446.3	750	0.02
0.4	2500	0.06	446.4	1000	0.02
0.5	2500	0.06	446.5	1250	0.03
0.6	2500	0.06	446.6	1500	0.03
0.7	2500	0.06	446.7	1750	0.04
0.8	2500	0.06	446.8	2000	0.05
0.9	2500	0.06	446.9	2250	0.05
1.0	2500	0.06	447.0	2500	0.06
1.1	2500	0.06	447.1	2750	0.06
1.2	2500	0.06	447.2	3000	0.07
1.3	2500	0.06	447.3	3250	0.07
1.4	2500	0.06	447.4	3500	0.08
1.5	2500	0.06	447.5	3750	0.09
1.6	2500	0.06	447.6	4000	0.09
1.7	2500	0.06	447.7	4250	0.10
1.8	2500	0.06	447.8	4500	0.10
1.9	2500	0.06	447.9	4750	0.11
2.0	2500	0.06	448.0	5000	0.11
2.1	2500	0.06	448.1	5250	0.12
2.2	2500	0.06	448.2	5500	0.13
2.3	2500	0.06	448.3	5750	0.13
2.4	2500	0.06	448.4	6000	0.14
2.5	2500	0.06	448.5	6250	0.14
2.6	2500	0.06	448.6	6500	0.15
2.7	2500	0.06	448.7	6750	0.15
2.8	2500	0.06	448.8	7000	0.16
2.9	2500	0.06	448.9	7250	0.17
3.0	2500	0.06	449.0	7500	0.17
3.1	2500	0.06	449.1	7750	0.18
3.2	2500	0.06	449.2	8000	0.18
3.3	2500	0.06	449.3	8250	0.19
3.4	2500	0.06	449.4	8500	0.20
3.5	2500	0.06	449.5	8750	0.20
3.6	2500	0.06	449.6	9000	0.21
3.7	2500	0.06	449.7	9250	0.21
3.8	2500	0.06	449.8	9500	0.22
3.9	2500	0.06	449.9	9750	0.22
4.0	2500	0.06	450.0	10000	0.23
4.1	2500	0.06	450.1	10250	0.24
4.2	2500	0.06	450.2	10500	0.24
4.3	2500	0.06	450.3	10750	0.25
4.4	2500	0.06	450.4	11000	0.25
4.5	2500	0.06	450.5	11250	0.26
4.6	2500	0.06	450.6	11500	0.26
4.7	2500	0.06	450.7	11750	0.27
4.8	2500	0.06	450.8	12000	0.28
4.9	2500	0.06	450.9	12250	0.28
5.0	2500	0.06	451.0	12500	0.29
5.1	2500	0.06	451.1	12750	0.29
5.2	2500	0.06	451.2	13000	0.30
5.3	2500	0.06	451.3	13250	0.30
5.4	2500	0.06	451.4	13500	0.31
5.5	2500	0.06	451.5	13750	0.32
5.6	2500	0.06	451.6	14000	0.32
5.7	2500	0.06	451.7	14250	0.33
5.8	2500	0.06	451.8	14500	0.33
5.9	2500	0.06	451.9	14750	0.34
6.0	2500	0.06	452.0	15000	0.34

Vault #1		Q _{Sub Drain} =	N/A cfs	
Elevation	Q _{AVG} (CFS)	DV (CF)	DT (HR)	Total T
446.1	0.006	250	11.74	35.8
446.2	0.023	250	3.02	24.1
446.3	0.040	250	1.73	21.1
446.4	0.051	250	1.36	19.3
446.5	0.060	250	1.16	18.0
446.6	0.068	250	1.03	16.8
446.7	0.074	250	0.93	15.8
446.8	0.081	250	0.86	14.8
446.9	0.086	250	0.80	14.0
447.0	0.092	250	0.76	13.2
447.1	0.097	250	0.72	12.4
447.2	0.102	250	0.68	11.7
447.3	0.107	250	0.65	11.0
447.4	0.111	250	0.62	10.4
447.5	0.115	250	0.60	9.8
447.6	0.120	250	0.58	9.2
447.7	0.124	250	0.56	8.6
447.8	0.127	250	0.54	8.0
447.9	0.131	250	0.53	7.5
448.0	0.135	250	0.51	6.9
448.1	0.138	250	0.50	6.4
448.2	0.142	250	0.49	5.9
448.3	0.145	250	0.48	5.4
448.4	0.149	250	0.47	5.0
448.5	0.152	250	0.46	4.5
448.6	0.155	250	0.45	4.0
448.7	0.158	250	0.44	3.6
448.8	0.175	250	0.40	3.1
448.9	0.226	250	0.31	2.7
449.0	0.289	250	0.24	2.4
449.1	0.335	250	0.21	2.2
449.2	0.369	250	0.19	2.0
449.3	0.398	250	0.17	1.8
449.4	0.425	250	0.16	1.6
449.5	0.449	250	0.15	1.5
449.6	0.472	250	0.15	1.3
449.7	0.493	250	0.14	1.2
449.8	0.513	250	0.14	1.0
449.9	0.533	250	0.13	0.9
450.0	0.551	250	0.13	0.8
450.1	0.600	250	0.12	0.6
450.2	0.729	250	0.10	0.5
450.3	0.915	250	0.08	0.4
450.4	1.072	250	0.06	0.3
450.5	1.181	250	0.06	0.3
450.6	1.275	250	0.05	0.2
450.7	1.467	250	0.05	0.2
450.8	2.101	250	0.03	0.1
450.9	3.264	250	0.02	0.1
451.0	4.758	250	0.01	0.1
451.1	6.516	250	0.01	0.0
451.2	8.501	250	0.01	0.0
451.3	10.688	250	0.01	0.0
451.4	13.058	250	0.01	0.0
451.5	15.598	250	0.00	0.0
451.6	18.298	250	0.00	0.0
451.7	21.148	250	0.00	0.0
451.8	24.140	250	0.00	0.0
451.9	27.268	250	0.00	0.0
452.0	30.526	250	0.00	0.0

IV. SWMM INPUT DATA (EXISTING AND PROPOSED MODELS)

Attached are the EPA-SWMM Model outputs for pre-development and post-development conditions for the POC. Similar modeling was used for analyzing the remaining POC locations using the input data found in the previous section. Each area, i.e., sub-catchments, outfalls, storage units, weir as a discharge, and outfalls (point of compliance), also are shown, as applicable.

Variables in the modeling are associated with typical recommended values by the EPA-SWMM model and typical values found in technical literature (such as Maidment's Handbook of Hydrology). Recommended values for the SWMM model have been obtained from the City of Chula Vista BMP manual Appendix G.

Soil characteristics of the existing soils were determined based on data provided by Geotechnical Engineer/Geologist. KEY ASSUMPTIONS: Group C & D soils are representative of the existing conditions for the site.

Some values incorporated within the SWMM model have been determined from the professional experience of H&A using conservative assumptions that tend to increase the size of the needed BMP and also generate a long-term runoff as a percentage of rainfall similar to those measured in gage stations in Southern California by the USGS.

Description of model parameters and assumptions:

N-Imperv – Manning's N for impervious surfaces

0.012 (typical)

N-Perv – Manning's N for pervious surfaces

0.15

Dstore-Imperv – Depth of depression storage on impervious area (in)

0.05 (typical)

Dstore-Perv – Depth of depression storage on pervious area (in)

0.100

%Zero-Imperv – Percentage of impervious area with no depression storage (%)

25 (typical)

Suction Head – Soil capillary suction head (in)

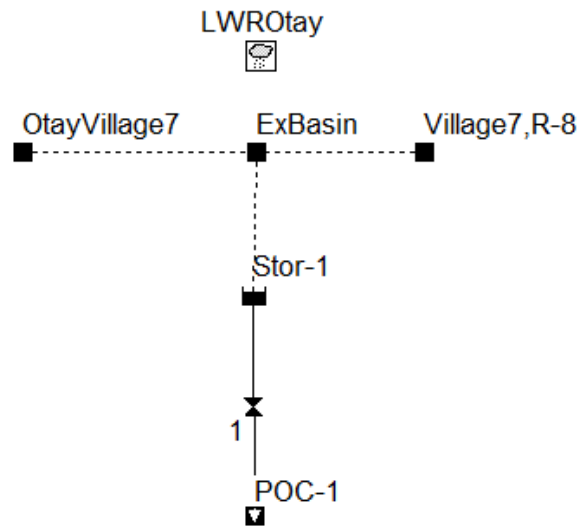
Conductivity – Soil saturated hydraulic conductivity (in/hr)

-75% of these values if subcatchment is graded/compacted

Initial Deficit – Initial moisture deficit (fraction)

Soil Type	Suction Head	Conductivity	Initial Deficit
A	1.5	0.3	0.33
B	3	0.2	0.32
C	6	0.1	0.31
D	9	0.025	0.30

POC 1 – Pre-Developed Condition



SWMM Model - EX Condition Input Data

[TITLE]

::Project Title/Notes

[OPTIONS]

```

::Option      Value
FLOW_UNITS    CFS
INFILTRATION  GREEN_AMPT
FLOW_ROUTING  KINWAVE
LINK_OFFSETS  DEPTH
MIN_SLOPE     0
ALLOW_PONDING NO
SKIP_STEADY_STATE NO
    
```

```

START_DATE    08/28/1951
START_TIME    00:00:00
REPORT_START_DATE 08/28/1951
REPORT_START_TIME 00:00:00
END_DATE      03/16/2008
END_TIME      23:00:00
SWEEP_START   01/01
SWEEP_END     12/31
DRY_DAYS      0
REPORT_STEP   01:00:00
WET_STEP      00:15:00
DRY_STEP      04:00:00
ROUTING_STEP  0:01:00
RULE_STEP     00:00:00
    
```

```

INERTIAL_DAMPING PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQUATION H-W
VARIABLE_STEP    0.75
LENGTHENING_STEP 0
MIN_SURFAREA     12.557
MAX_TRIALS       8
HEAD_TOLERANCE   0.005
SYS_FLOW_TOL     5
LAT_FLOW_TOL     5
MINIMUM_STEP     0.5
THREADS          1
    
```

[EVAPORATION]

::Data Source Parameters

```

::-----
MONTHLY      0.06  0.08  0.11  0.16  0.18  0.21  0.21  0.20  0.16  0.12  0.08  0.06
DRY_ONLY     NO
    
```

[RAINGAGES]

::Name Format Interval SCF Source

```

::-----
LWR0tay INTENSITY 1:00 1.0 TIMESERIES LWR0tay
    
```

[SUBCATCHMENTS]

::Name Rain Gage Outlet Area %Imperv Width %Slope CurbLen SnowPack

```

::-----
ExBasin LWR0tay Stor-1 2.96 1.5 225 0.1 0
;Proposed Developed Area - Village 7 R-8
Village7, R-8 LWR0tay ExBasin 13.58 0 1193 12.78 0
;Existing Developed Area - Otay Village 7
OtayVillage7 LWR0tay ExBasin 136.19 47 1594 2.37 0
    
```

[SUBAREAS]

::Subcatchment N-Imperv N-Perv S-Imperv S-Perv PctZero RouteTo PctRouted

```

::-----
ExBasin 0.012 0.15 0.05 .10 25 OUTLET
Village7, R-8 0.012 0.15 0.05 0.10 25 OUTLET
OtayVillage7 0.012 0.15 0.05 0.10 25 OUTLET
    
```

[INFILTRATION]

::Subcatchment Suction Ksat IMD

```

::-----
    
```

ExBasin	9	0.01875	0.33
Village7, R-8	9	0.025	0.33
OtayVillage7	9	0.01875	.33

[OUTFALLS]

;; Name	Elevation	Type	Stage Data	Gated	Route To
POC-1	0	FREE		NO	

[STORAGE]

;; Name	El ev.	MaxDepth	Ini tDepth	Shape	Curve Name/Params	N/A	Fevap	Psi
ExBasin Stor-1	390	10.9	0	TABULAR	ExBasin	0	1	

[OUTLETS]

;; Name	From Node	To Node	Offset	Type	QTable/Qcoeff	Qexpon	Gated
1	Stor-1	POC-1	0	TABULAR/DEPTH	ExBasinOutlet		NO

[CURVES]

;; Name	Type	X-Val ue	Y-Val ue
Ex Basin Outlet	Rating	0.00	0.000
ExBasinOutlet		0.05	0.015
ExBasinOutlet		0.10	0.057
ExBasinOutlet		0.15	0.125
ExBasinOutlet		0.20	0.219
ExBasinOutlet		0.25	0.339
ExBasinOutlet		0.30	0.484
ExBasinOutlet		0.35	0.654
ExBasinOutlet		0.40	0.849
ExBasinOutlet		0.45	1.069
ExBasinOutlet		0.50	1.312
ExBasinOutlet		0.55	1.579
ExBasinOutlet		0.60	1.869
ExBasinOutlet		0.65	2.182
ExBasinOutlet		0.70	2.518
ExBasinOutlet		0.75	2.876
ExBasinOutlet		0.80	3.255
ExBasinOutlet		0.85	3.656
ExBasinOutlet		0.90	4.078
ExBasinOutlet		0.95	4.520
ExBasinOutlet		1.00	4.982
ExBasinOutlet		1.05	5.463
ExBasinOutlet		1.10	5.964
ExBasinOutlet		1.15	6.483
ExBasinOutlet		1.20	7.020
ExBasinOutlet		1.25	7.575
ExBasinOutlet		1.30	8.147
ExBasinOutlet		1.35	8.735
ExBasinOutlet		1.40	9.340
ExBasinOutlet		1.45	9.960
ExBasinOutlet		1.50	10.596
ExBasinOutlet		1.55	11.246
ExBasinOutlet		1.60	11.910
ExBasinOutlet		1.65	12.588
ExBasinOutlet		1.70	13.279
ExBasinOutlet		1.75	13.982
ExBasinOutlet		1.80	14.698
ExBasinOutlet		1.85	15.424
ExBasinOutlet		1.90	16.162
ExBasinOutlet		1.95	16.910
ExBasinOutlet		2.00	17.668
ExBasinOutlet		2.05	18.435

ExBasi nOutlet	2. 10	19. 211
ExBasi nOutlet	2. 15	19. 995
ExBasi nOutlet	2. 20	20. 787
ExBasi nOutlet	2. 25	21. 586
ExBasi nOutlet	2. 30	22. 391
ExBasi nOutlet	2. 35	23. 203
ExBasi nOutlet	2. 40	24. 020
ExBasi nOutlet	2. 45	24. 842
ExBasi nOutlet	2. 50	25. 668
ExBasi nOutlet	2. 55	26. 498
ExBasi nOutlet	2. 60	27. 331
ExBasi nOutlet	2. 65	28. 167
ExBasi nOutlet	2. 70	29. 005
ExBasi nOutlet	2. 75	29. 430
ExBasi nOutlet	2. 80	29. 916
ExBasi nOutlet	2. 85	30. 395
ExBasi nOutlet	2. 90	30. 866
ExBasi nOutlet	2. 95	31. 330
ExBasi nOutlet	3. 00	31. 788
ExBasi nOutlet	3. 05	32. 239
ExBasi nOutlet	3. 10	32. 683
ExBasi nOutlet	3. 15	33. 122
ExBasi nOutlet	3. 20	33. 555
ExBasi nOutlet	3. 25	33. 983
ExBasi nOutlet	3. 30	34. 405
ExBasi nOutlet	3. 35	34. 822
ExBasi nOutlet	3. 40	35. 234
ExBasi nOutlet	3. 45	35. 641
ExBasi nOutlet	3. 50	36. 044
ExBasi nOutlet	3. 55	36. 442
ExBasi nOutlet	3. 60	36. 836
ExBasi nOutlet	3. 65	37. 226
ExBasi nOutlet	3. 70	37. 612
ExBasi nOutlet	3. 75	37. 994
ExBasi nOutlet	3. 80	38. 372
ExBasi nOutlet	3. 85	38. 746
ExBasi nOutlet	3. 90	39. 117
ExBasi nOutlet	3. 95	39. 484
ExBasi nOutlet	4. 00	39. 848
ExBasi nOutlet	4. 05	40. 209
ExBasi nOutlet	4. 10	40. 566
ExBasi nOutlet	4. 15	40. 921
ExBasi nOutlet	4. 20	41. 272
ExBasi nOutlet	4. 25	41. 620
ExBasi nOutlet	4. 30	41. 966
ExBasi nOutlet	4. 35	42. 308
ExBasi nOutlet	4. 40	42. 648
ExBasi nOutlet	4. 45	42. 985
ExBasi nOutlet	4. 50	43. 320
ExBasi nOutlet	4. 55	43. 652
ExBasi nOutlet	4. 60	43. 981
ExBasi nOutlet	4. 65	44. 308
ExBasi nOutlet	4. 70	44. 633
ExBasi nOutlet	4. 75	44. 955
ExBasi nOutlet	4. 80	45. 275
ExBasi nOutlet	4. 85	45. 593
ExBasi nOutlet	4. 90	45. 908
ExBasi nOutlet	4. 95	46. 221
ExBasi nOutlet	5. 00	46. 533
ExBasi nOutlet	5. 05	46. 842
ExBasi nOutlet	5. 10	47. 149
ExBasi nOutlet	5. 15	47. 454
ExBasi nOutlet	5. 20	47. 757
ExBasi nOutlet	5. 25	48. 059
ExBasi nOutlet	5. 30	48. 358
ExBasi nOutlet	5. 35	48. 656
ExBasi nOutlet	5. 40	48. 952
ExBasi nOutlet	5. 45	49. 246
ExBasi nOutlet	5. 50	49. 538
ExBasi nOutlet	5. 55	49. 828
ExBasi nOutlet	5. 60	50. 117

ExBasi nOutlet	5. 65	50. 404
ExBasi nOutlet	5. 70	50. 690
ExBasi nOutlet	5. 75	50. 974
ExBasi nOutlet	5. 80	51. 256
ExBasi nOutlet	5. 85	51. 537
ExBasi nOutlet	5. 90	51. 817
ExBasi nOutlet	5. 95	52. 094
ExBasi nOutlet	6. 00	52. 371
ExBasi nOutlet	6. 05	52. 646
ExBasi nOutlet	6. 10	52. 919
ExBasi nOutlet	6. 15	53. 191
ExBasi nOutlet	6. 20	53. 462
ExBasi nOutlet	6. 25	53. 731
ExBasi nOutlet	6. 30	53. 999
ExBasi nOutlet	6. 35	54. 266
ExBasi nOutlet	6. 40	54. 531
ExBasi nOutlet	6. 45	54. 795
ExBasi nOutlet	6. 50	55. 058
ExBasi nOutlet	6. 55	55. 320
ExBasi nOutlet	6. 60	55. 580
ExBasi nOutlet	6. 65	55. 839
ExBasi nOutlet	6. 70	56. 097
ExBasi nOutlet	6. 75	56. 354
ExBasi nOutlet	6. 80	56. 609
ExBasi nOutlet	6. 85	56. 864
ExBasi nOutlet	6. 90	57. 117
ExBasi nOutlet	6. 95	57. 369
ExBasi nOutlet	7. 00	57. 620
ExBasi nOutlet	7. 05	57. 870
ExBasi nOutlet	7. 10	58. 119
ExBasi nOutlet	7. 15	58. 367
ExBasi nOutlet	7. 20	58. 614
ExBasi nOutlet	7. 25	58. 860
ExBasi nOutlet	7. 30	59. 104
ExBasi nOutlet	7. 35	59. 348
ExBasi nOutlet	7. 40	59. 591
ExBasi nOutlet	7. 45	59. 833
ExBasi nOutlet	7. 50	60. 073
ExBasi nOutlet	7. 55	60. 313
ExBasi nOutlet	7. 60	60. 552
ExBasi nOutlet	7. 65	60. 790
ExBasi nOutlet	7. 70	61. 027
ExBasi nOutlet	7. 75	61. 263
ExBasi nOutlet	7. 80	61. 498
ExBasi nOutlet	7. 85	61. 733
ExBasi nOutlet	7. 90	61. 966
ExBasi nOutlet	7. 95	62. 199
ExBasi nOutlet	8. 00	62. 430
ExBasi nOutlet	8. 05	62. 661
ExBasi nOutlet	8. 10	62. 891
ExBasi nOutlet	8. 15	63. 120
ExBasi nOutlet	8. 20	63. 348
ExBasi nOutlet	8. 25	63. 576
ExBasi nOutlet	8. 30	63. 802
ExBasi nOutlet	8. 35	64. 028
ExBasi nOutlet	8. 40	64. 253
ExBasi nOutlet	8. 45	64. 478
ExBasi nOutlet	8. 50	64. 701
ExBasi nOutlet	8. 55	64. 924
ExBasi nOutlet	8. 60	65. 146
ExBasi nOutlet	8. 65	65. 367
ExBasi nOutlet	8. 70	65. 587
ExBasi nOutlet	8. 75	65. 807
ExBasi nOutlet	8. 80	66. 026
ExBasi nOutlet	8. 85	66. 244
ExBasi nOutlet	8. 90	66. 462
ExBasi nOutlet	8. 95	66. 679
ExBasi nOutlet	9. 00	66. 895
ExBasi nOutlet	9. 05	67. 110
ExBasi nOutlet	9. 10	67. 325
ExBasi nOutlet	9. 15	67. 539

ExBasi nOutlet	9. 20	67. 753
ExBasi nOutlet	9. 25	67. 965
ExBasi nOutlet	9. 30	68. 177
ExBasi nOutlet	9. 35	68. 389
ExBasi nOutlet	9. 40	68. 600
ExBasi nOutlet	9. 45	68. 810
ExBasi nOutlet	9. 50	69. 019
ExBasi nOutlet	9. 55	69. 228
ExBasi nOutlet	9. 60	69. 436
ExBasi nOutlet	9. 65	69. 644
ExBasi nOutlet	9. 70	69. 851
ExBasi nOutlet	9. 75	70. 057
ExBasi nOutlet	9. 80	70. 263
ExBasi nOutlet	9. 85	70. 468
ExBasi nOutlet	9. 90	70. 673
ExBasi nOutlet	9. 95	70. 877
ExBasi nOutlet	10. 00	71. 080
ExBasi nOutlet	10. 05	71. 283
ExBasi nOutlet	10. 10	71. 485
ExBasi nOutlet	10. 15	71. 687
ExBasi nOutlet	10. 20	71. 888
ExBasi nOutlet	10. 25	72. 088
ExBasi nOutlet	10. 30	72. 288
ExBasi nOutlet	10. 35	72. 488
ExBasi nOutlet	10. 40	72. 686
ExBasi nOutlet	10. 45	72. 885
ExBasi nOutlet	10. 50	73. 083
ExBasi nOutlet	10. 55	73. 280
ExBasi nOutlet	10. 60	73. 477
ExBasi nOutlet	10. 65	73. 673
ExBasi nOutlet	10. 70	73. 868
ExBasi nOutlet	10. 75	74. 064
ExBasi nOutlet	10. 80	74. 258
ExBasi nOutlet	10. 85	74. 452
ExBasi nOutlet	10. 90	74. 646
ExBasi nOutlet	10. 95	74. 839
ExBasi nOutlet	11. 00	75. 032
ExBasi nOutlet	11. 05	75. 224
ExBasi nOutlet	11. 10	75. 416
ExBasi nOutlet	11. 15	75. 607
ExBasi nOutlet	11. 20	75. 797
ExBasi nOutlet	11. 25	75. 988
ExBasi nOutlet	11. 30	76. 177
ExBasi nOutlet	11. 35	76. 367
ExBasi nOutlet	11. 40	76. 555
ExBasi nOutlet	11. 45	76. 744
ExBasi nOutlet	11. 50	76. 932
ExBasi nOutlet	11. 55	77. 119
ExBasi nOutlet	11. 60	77. 306
ExBasi nOutlet	11. 65	77. 492
ExBasi nOutlet	11. 70	77. 678
ExBasi nOutlet	11. 75	77. 864
ExBasi nOutlet	11. 80	78. 049
ExBasi nOutlet	11. 85	78. 234
ExBasi nOutlet	11. 90	78. 418
ExBasi nOutlet	11. 95	80. 985
ExBasi nOutlet	12. 00	85. 525
ExBasi nOutlet	12. 05	91. 350
ExBasi nOutlet	12. 10	98. 213
ExBasi nOutlet	12. 15	105. 973
ExBasi nOutlet	12. 20	114. 534
ExBasi nOutlet	12. 25	123. 826
ExBasi nOutlet	12. 30	133. 793
ExBasi nOutlet	12. 35	144. 392
ExBasi nOutlet	12. 40	155. 587
ExBasi nOutlet	12. 45	167. 347
ExBasi nOutlet	12. 50	179. 646
ExBasi nOutlet	12. 55	192. 461
ExBasi nOutlet	12. 60	205. 771
ExBasi nOutlet	12. 65	219. 558
ExBasi nOutlet	12. 70	233. 807

ExBasi nOutlet	12. 75	248. 502
ExBasi nOutlet	12. 80	263. 630
ExBasi nOutlet	12. 85	279. 179
ExBasi nOutlet	12. 90	295. 137
ExBasi nOutlet	12. 95	311. 495
ExBasi nOutlet	13. 00	328. 243
ExBasi nOutlet	13. 05	345. 371
ExBasi nOutlet	13. 10	362. 872
ExBasi nOutlet	13. 15	380. 737
ExBasi nOutlet	13. 20	398. 959
ExBasi nOutlet	13. 25	417. 531
ExBasi nOutlet	13. 30	436. 447
ExBasi nOutlet	13. 35	455. 701
ExBasi nOutlet	13. 40	467. 465
ExBasi nOutlet	13. 45	473. 980
ExBasi nOutlet	13. 50	480. 393
ExBasi nOutlet	13. 55	486. 708
ExBasi nOutlet	13. 60	492. 931
ExBasi nOutlet	13. 65	499. 066
ExBasi nOutlet	13. 70	505. 115
ExBasi nOutlet	13. 75	511. 083
ExBasi nOutlet	13. 80	516. 973
ExBasi nOutlet	13. 85	522. 788
ExBasi nOutlet	13. 90	528. 530

;			
; Ex Basi n			
ExBasi n	Storage	0. 00	17535
ExBasi n		0. 05	17647
ExBasi n		0. 10	17758
ExBasi n		0. 15	17870
ExBasi n		0. 20	17982
ExBasi n		0. 25	18093
ExBasi n		0. 30	18205
ExBasi n		0. 35	18317
ExBasi n		0. 40	18428
ExBasi n		0. 45	18540
ExBasi n		0. 50	18651
ExBasi n		0. 55	18763
ExBasi n		0. 60	18875
ExBasi n		0. 65	18986
ExBasi n		0. 70	19098
ExBasi n		0. 75	19210
ExBasi n		0. 80	19321
ExBasi n		0. 85	19433
ExBasi n		0. 90	19545
ExBasi n		0. 95	19656
ExBasi n		1. 00	19768
ExBasi n		1. 05	19873
ExBasi n		1. 10	19979
ExBasi n		1. 15	20085
ExBasi n		1. 20	20190
ExBasi n		1. 25	20296
ExBasi n		1. 30	20401
ExBasi n		1. 35	20507
ExBasi n		1. 40	20613
ExBasi n		1. 45	20718
ExBasi n		1. 50	20824
ExBasi n		1. 55	20929
ExBasi n		1. 60	21035
ExBasi n		1. 65	21140
ExBasi n		1. 70	21246
ExBasi n		1. 75	21352
ExBasi n		1. 80	21457
ExBasi n		1. 85	21563
ExBasi n		1. 90	21668
ExBasi n		1. 95	21774
ExBasi n		2. 00	21880
ExBasi n		2. 05	21991
ExBasi n		2. 10	22102
ExBasi n		2. 15	22213
ExBasi n		2. 20	22325

ExBasi n	2. 25	22436
ExBasi n	2. 30	22547
ExBasi n	2. 35	22659
ExBasi n	2. 40	22770
ExBasi n	2. 45	22881
ExBasi n	2. 50	22993
ExBasi n	2. 55	23104
ExBasi n	2. 60	23215
ExBasi n	2. 65	23326
ExBasi n	2. 70	23438
ExBasi n	2. 75	23549
ExBasi n	2. 80	23660
ExBasi n	2. 85	23772
ExBasi n	2. 90	23883
ExBasi n	2. 95	23994
ExBasi n	3. 00	24106
ExBasi n	3. 05	24215
ExBasi n	3. 10	24325
ExBasi n	3. 15	24435
ExBasi n	3. 20	24545
ExBasi n	3. 25	24655
ExBasi n	3. 30	24765
ExBasi n	3. 35	24875
ExBasi n	3. 40	24985
ExBasi n	3. 45	25095
ExBasi n	3. 50	25205
ExBasi n	3. 55	25315
ExBasi n	3. 60	25425
ExBasi n	3. 65	25535
ExBasi n	3. 70	25645
ExBasi n	3. 75	25755
ExBasi n	3. 80	25865
ExBasi n	3. 85	25975
ExBasi n	3. 90	26085
ExBasi n	3. 95	26195
ExBasi n	4. 00	26305
ExBasi n	4. 05	26420
ExBasi n	4. 10	26536
ExBasi n	4. 15	26652
ExBasi n	4. 20	26768
ExBasi n	4. 25	26883
ExBasi n	4. 30	26999
ExBasi n	4. 35	27115
ExBasi n	4. 40	27230
ExBasi n	4. 45	27346
ExBasi n	4. 50	27462
ExBasi n	4. 55	27578
ExBasi n	4. 60	27693
ExBasi n	4. 65	27809
ExBasi n	4. 70	27925
ExBasi n	4. 75	28040
ExBasi n	4. 80	28156
ExBasi n	4. 85	28272
ExBasi n	4. 90	28388
ExBasi n	4. 95	28503
ExBasi n	5. 00	28619
ExBasi n	5. 05	28739
ExBasi n	5. 10	28860
ExBasi n	5. 15	28980
ExBasi n	5. 20	29101
ExBasi n	5. 25	29221
ExBasi n	5. 30	29342
ExBasi n	5. 35	29462
ExBasi n	5. 40	29582
ExBasi n	5. 45	29703
ExBasi n	5. 50	29823
ExBasi n	5. 55	29944
ExBasi n	5. 60	30064
ExBasi n	5. 65	30184
ExBasi n	5. 70	30305
ExBasi n	5. 75	30425

ExBasi n	5. 80	30546
ExBasi n	5. 85	30666
ExBasi n	5. 90	30787
ExBasi n	5. 95	30907
ExBasi n	6. 00	31027
ExBasi n	6. 05	31160
ExBasi n	6. 10	31292
ExBasi n	6. 15	31424
ExBasi n	6. 20	31557
ExBasi n	6. 25	31689
ExBasi n	6. 30	31821
ExBasi n	6. 35	31954
ExBasi n	6. 40	32086
ExBasi n	6. 45	32219
ExBasi n	6. 50	32351
ExBasi n	6. 55	32483
ExBasi n	6. 60	32616
ExBasi n	6. 65	32748
ExBasi n	6. 70	32880
ExBasi n	6. 75	33013
ExBasi n	6. 80	33145
ExBasi n	6. 85	33277
ExBasi n	6. 90	33410
ExBasi n	6. 95	33542
ExBasi n	7. 00	33674
ExBasi n	7. 05	33810
ExBasi n	7. 10	33945
ExBasi n	7. 15	34081
ExBasi n	7. 20	34216
ExBasi n	7. 25	34352
ExBasi n	7. 30	34487
ExBasi n	7. 35	34623
ExBasi n	7. 40	34758
ExBasi n	7. 45	34894
ExBasi n	7. 50	35029
ExBasi n	7. 55	35165
ExBasi n	7. 60	35300
ExBasi n	7. 65	35436
ExBasi n	7. 70	35571
ExBasi n	7. 75	35707
ExBasi n	7. 80	35842
ExBasi n	7. 85	35977
ExBasi n	7. 90	36113
ExBasi n	7. 95	36248
ExBasi n	8. 00	36384
ExBasi n	8. 05	36523
ExBasi n	8. 10	36662
ExBasi n	8. 15	36801
ExBasi n	8. 20	36941
ExBasi n	8. 25	37080
ExBasi n	8. 30	37219
ExBasi n	8. 35	37358
ExBasi n	8. 40	37497
ExBasi n	8. 45	37636
ExBasi n	8. 50	37776
ExBasi n	8. 55	37915
ExBasi n	8. 60	38054
ExBasi n	8. 65	38193
ExBasi n	8. 70	38332
ExBasi n	8. 75	38471
ExBasi n	8. 80	38611
ExBasi n	8. 85	38750
ExBasi n	8. 90	38889
ExBasi n	8. 95	39028
ExBasi n	9. 00	39167
ExBasi n	9. 05	39311
ExBasi n	9. 10	39455
ExBasi n	9. 15	39599
ExBasi n	9. 20	39743
ExBasi n	9. 25	39887
ExBasi n	9. 30	40031

ExBasi n	9. 35	40175
ExBasi n	9. 40	40319
ExBasi n	9. 45	40463
ExBasi n	9. 50	40607
ExBasi n	9. 55	40751
ExBasi n	9. 60	40895
ExBasi n	9. 65	41039
ExBasi n	9. 70	41183
ExBasi n	9. 75	41327
ExBasi n	9. 80	41471
ExBasi n	9. 85	41615
ExBasi n	9. 90	41759
ExBasi n	9. 95	41903
ExBasi n	10. 00	42047
ExBasi n	10. 05	42200
ExBasi n	10. 10	42353
ExBasi n	10. 15	42506
ExBasi n	10. 20	42659
ExBasi n	10. 25	42812
ExBasi n	10. 30	42965
ExBasi n	10. 35	43118
ExBasi n	10. 40	43271
ExBasi n	10. 45	43424
ExBasi n	10. 50	43577
ExBasi n	10. 55	43730
ExBasi n	10. 60	43883
ExBasi n	10. 65	44036
ExBasi n	10. 70	44189
ExBasi n	10. 75	44342
ExBasi n	10. 80	44495
ExBasi n	10. 85	44648
ExBasi n	10. 90	44801
ExBasi n	10. 95	44954
ExBasi n	11. 00	45107
ExBasi n	11. 05	45270
ExBasi n	11. 10	45432
ExBasi n	11. 15	45595
ExBasi n	11. 20	45758
ExBasi n	11. 25	45920
ExBasi n	11. 30	46083
ExBasi n	11. 35	46246
ExBasi n	11. 40	46408
ExBasi n	11. 45	46571
ExBasi n	11. 50	46734
ExBasi n	11. 55	46896
ExBasi n	11. 60	47059
ExBasi n	11. 65	47222
ExBasi n	11. 70	47384
ExBasi n	11. 75	47547
ExBasi n	11. 80	47710
ExBasi n	11. 85	47872
ExBasi n	11. 90	48035
ExBasi n	11. 95	48198
ExBasi n	12. 00	48360
ExBasi n	12. 05	48537
ExBasi n	12. 10	48713
ExBasi n	12. 15	48889
ExBasi n	12. 20	49066
ExBasi n	12. 25	49242
ExBasi n	12. 30	49419
ExBasi n	12. 35	49595
ExBasi n	12. 40	49772
ExBasi n	12. 45	49948
ExBasi n	12. 50	50124
ExBasi n	12. 55	50301
ExBasi n	12. 60	50477
ExBasi n	12. 65	50654
ExBasi n	12. 70	50830
ExBasi n	12. 75	51007
ExBasi n	12. 80	51183
ExBasi n	12. 85	51359

ExBasi n	12. 90	51536
ExBasi n	12. 95	51712
ExBasi n	13. 00	51889
ExBasi n	13. 05	52065
ExBasi n	13. 10	52242
ExBasi n	13. 15	52418
ExBasi n	13. 20	52594
ExBasi n	13. 25	52771
ExBasi n	13. 30	52947
ExBasi n	13. 35	53124
ExBasi n	13. 40	53300
ExBasi n	13. 45	53477
ExBasi n	13. 50	53653
ExBasi n	13. 55	53829
ExBasi n	13. 60	54006
ExBasi n	13. 65	54182
ExBasi n	13. 70	54359
ExBasi n	13. 75	54535
ExBasi n	13. 80	54712
ExBasi n	13. 85	54888
ExBasi n	13. 90	55064

[TIMESERIES]

```
;; Name          Date      Time      Value
;; -----
; Lower Otay Rain Gage from San Diego County
LWR0tay          FILE "lower_otay.prn"
```

[REPORT]

```
;; Reporting Options
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL
```

[TAGS]

[MAP]

```
DIMENSIONS 0.000 0.000 10000.000 10000.000
Units      None
```

[COORDINATES]

```
;; Node          X-Coord          Y-Coord
;; -----
POC-1            1488. 812         3184. 165
Stor-1           1480. 207         4268. 503
```

[VERTICES]

```
;; Link          X-Coord          Y-Coord
;; -----
```

[Polygons]

```
;; Subcatchment X-Coord          Y-Coord
;; -----
ExBasi n        1497. 418         4991. 394
Village7, R-8   2332. 186         4991. 394
OtayVillage7    335. 628          4991. 394
OtayVillage7    335. 628          4991. 394
```

[SYMBOLS]

```
;; Gage          X-Coord          Y-Coord
;; -----
LWR0tay         1531. 842         5456. 110
```

SWMM Model - EX Condition Output Report

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.014)

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

 Analysis Options

Flow Units CFS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed NO
 Water Quality NO
 Infiltration Method GREEN_AMPT
 Flow Routing Method KINWAVE
 Starting Date 08/28/1951 00:00:00
 Ending Date 03/16/2008 23:00:00
 Antecedent Dry Days 0.0
 Report Time Step 01:00:00
 Wet Time Step 00:15:00
 Dry Time Step 04:00:00
 Routing Time Step 60.00 sec

*****	Volume	Depth
Runoff Quantity Continuity	acre-feet	inches
*****	-----	-----
Total Precipitation	7547.280	592.990
Evaporation Loss	853.542	67.063
Infiltration Loss	3808.808	299.258
Surface Runoff	2917.807	229.252
Final Storage	0.307	0.024
Continuity Error (%)	-0.440	

*****	Volume	Volume
Flow Routing Continuity	acre-feet	10^6 gal
*****	-----	-----
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	2917.805	950.810
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	2906.865	947.245
Flooding Loss	0.000	0.000
Evaporation Loss	9.134	2.977
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.102	0.033
Continuity Error (%)	0.058	

 Highest Flow Instability Indexes

 All links are stable.

 Routing Time Step Summary

Minimum Time Step : 60.00 sec
 Average Time Step : 60.00 sec
 Maximum Time Step : 60.00 sec
 Percent in Steady State : 0.00
 Average Iterations per Step : 1.00
 Percent Not Converging : 0.00

 Subcatchment Runoff Summary

Peak Runoff Runoff Coeff Subcatchment CFS	Total Precip in	Total Runon in	Total Evap in	Total Infil in	Imperv Runoff in	Perv Runoff in	Total Runoff in	Total Runoff 10^6 gal
ExBasin 78.19 0.904	592.99	12486.50	118.96	1162.72	194.50	11634.44	11828.95	950.74
Village7, R-8 9.46 0.140	592.99	0.00	17.91	493.97	0.00	83.14	83.14	30.66
OtayVillage7 75.92 0.444	592.99	0.00	70.84	261.08	223.04	40.06	263.10	972.93

 Node Depth Summary

Node	Type	Average Depth Feet	Maximum Depth Feet	Maximum HGL Feet	Time of Max Occurrence days hr: min	Reported Max Depth Feet
POC-1	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
Stor-1	STORAGE	0.02	5.73	395.73	16971 17:45	5.58

 Node Inflow Summary

Node	Type	Maximum Lateral Inflow CFS	Maximum Total Inflow CFS	Time of Max Occurrence days hr: min	Lateral Inflow Volume 10^6 gal	Total Inflow Volume 10^6 gal	Flow Balance Error Percent
POC-1	OUTFALL	0.00	50.84	16971 17:45	0	947	0.000
Stor-1	STORAGE	78.19	78.19	16971 17:16	951	951	0.058

 Node Flooding Summary

No nodes were flooded.

 Storage Volume Summary

Storage Unit	Average Volume 1000 ft3	Avg Pcnt Full	Evap Pcnt Loss	Exfil Pcnt Loss	Maximum Volume 1000 ft3	Max Pcnt Full	Time of Max Occurrence days hr: min	Maximum Outflow CFS
Stor-1	0.380	0	0	0	136.579	41	16971 17:44	50.84

 Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow CFS	Max Flow CFS	Total Volume 10^6 gal
POC-1	11.73	0.60	50.84	947.175
System	11.73	0.60	50.84	947.175

 Link Flow Summary

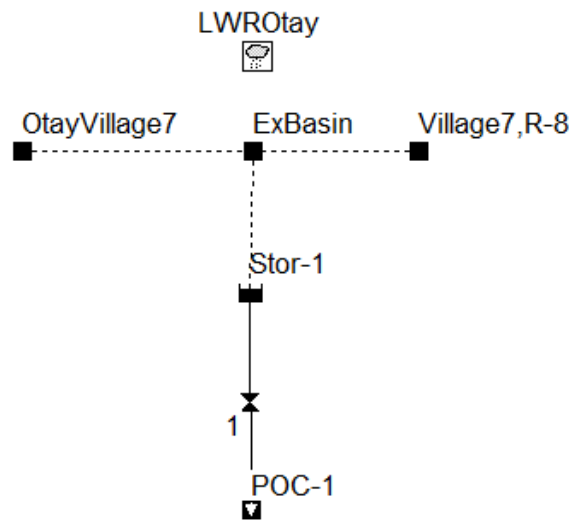
Link	Type	Maximum Flow CFS	Time of Max Occurrence days hr: min	Maximum Veloc ft/sec	Max/ Full Flow	Max/ Full Depth
1	DUMMY	50.84	16971 17:45			

 Conduit Surcharge Summary

No conduits were surcharged.

Analysis begun on: Thu Apr 6 10:23:43 2023
 Analysis ended on: Thu Apr 6 10:24:14 2023
 Total elapsed time: 00:00:31

POC 1 – Developed Condition



SWMM Model - PR Condition Input Data

[TITLE]

::Project Title/Notes

[OPTIONS]

```

::Option      Value
FLOW_UNITS   CFS
INFILTRATION GREEN_AMPT
FLOW_ROUTING KINWAVE
LINK_OFFSETS DEPTH
MIN_SLOPE    0
ALLOW_PONDING NO
SKIP_STEADY_STATE NO
    
```

```

START_DATE    08/28/1951
START_TIME    00:00:00
REPORT_START_DATE 08/28/1951
REPORT_START_TIME 00:00:00
END_DATE      03/16/2008
END_TIME      23:00:00
SWEEP_START   01/01
SWEEP_END     12/31
DRY_DAYS      0
REPORT_STEP   01:00:00
WET_STEP      00:15:00
DRY_STEP      04:00:00
ROUTING_STEP  0:01:00
RULE_STEP     00:00:00
    
```

```

INERTIAL_DAMPING PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQUATION H-W
VARIABLE_STEP    0.75
LENGTHENING_STEP 0
MIN_SURFAREA     12.557
MAX_TRIALS       8
HEAD_TOLERANCE   0.005
SYS_FLOW_TOL     5
LAT_FLOW_TOL     5
MINIMUM_STEP     0.5
THREADS          1
    
```

[EVAPORATION]

::Data Source Parameters

```

::-----
MONTHLY      0.06  0.08  0.11  0.16  0.18  0.21  0.21  0.20  0.16  0.12  0.08  0.06
DRY_ONLY     NO
    
```

[RAINGAGES]

::Name Format Interval SCF Source

```

::-----
LWR0tay INTENSITY 1:00 1.0 TIMESERIES LWR0tay
    
```

[SUBCATCHMENTS]

::Name Rain Gage Outlet Area %Imperv Width %Slope CurbLen SnowPack

```

::-----
ExBasin LWR0tay Stor-1 2.96 1.5 225 0.1 0
;Proposed Developed Area - Village 7 R-8
Village7, R-8 LWR0tay ExBasin 13.82 48 1648 3.28 0
;Existing Developed Area - Otay Village 7
OtayVillage7 LWR0tay ExBasin 135.87 47 1594 2.37 0
    
```

[SUBAREAS]

::Subcatchment N-Imperv N-Perv S-Imperv S-Perv PctZero RouteTo PctRouted

```

::-----
ExBasin 0.012 0.15 0.05 .1 25 OUTLET
Village7, R-8 0.012 0.15 0.05 0.10 25 OUTLET
OtayVillage7 0.012 0.15 0.05 0.10 25 OUTLET
    
```

[INFILTRATION]

::Subcatchment Suction Ksat IMD

```

::-----
    
```

ExBasin	9	0.01875	0.33
Village7, R-8	9	0.01875	0.33
OtayVillage7	9	0.01875	0.33

[OUTFALLS]

Name	Elevation	Type	Stage Data	Gated	Route To
POC-1	0	FREE		NO	

[STORAGE]

Name	El ev.	MaxDepth	Ini tDepth	Shape	Curve Name/Params	N/A	Fevap	Psi
ExBasin Stor-1	390	10.9	0	TABULAR	ExBasin	0	1	

[OUTLETS]

Name	From Node	To Node	Offset	Type	QTable/Qcoeff	Qexpon	Gated
1	Stor-1	POC-1	0	TABULAR/DEPTH	ExBasinOutlet		NO

[CURVES]

Name	Type	X-Val ue	Y-Val ue
Ex Basin Outlet	Rating	0.00	0.000
ExBasinOutlet		0.05	0.008
ExBasinOutlet		0.10	0.031
ExBasinOutlet		0.15	0.069
ExBasinOutlet		0.20	0.120
ExBasinOutlet		0.25	0.184
ExBasinOutlet		0.30	0.262
ExBasinOutlet		0.35	0.350
ExBasinOutlet		0.40	0.450
ExBasinOutlet		0.45	0.560
ExBasinOutlet		0.50	0.680
ExBasinOutlet		0.55	0.808
ExBasinOutlet		0.60	0.943
ExBasinOutlet		0.65	1.085
ExBasinOutlet		0.70	1.232
ExBasinOutlet		0.75	1.385
ExBasinOutlet		0.80	1.541
ExBasinOutlet		0.85	1.700
ExBasinOutlet		0.90	1.861
ExBasinOutlet		0.95	1.950
ExBasinOutlet		1.00	2.039
ExBasinOutlet		1.05	2.125
ExBasinOutlet		1.10	2.207
ExBasinOutlet		1.15	2.286
ExBasinOutlet		1.20	2.363
ExBasinOutlet		1.25	2.437
ExBasinOutlet		1.30	2.509
ExBasinOutlet		1.35	2.579
ExBasinOutlet		1.40	2.648
ExBasinOutlet		1.45	2.714
ExBasinOutlet		1.50	2.779
ExBasinOutlet		1.55	2.851
ExBasinOutlet		1.60	2.939
ExBasinOutlet		1.65	3.041
ExBasinOutlet		1.70	3.157
ExBasinOutlet		1.75	3.288
ExBasinOutlet		1.80	3.431
ExBasinOutlet		1.85	3.588
ExBasinOutlet		1.90	3.756
ExBasinOutlet		1.95	3.936
ExBasinOutlet		2.00	4.126
ExBasinOutlet		2.05	4.327

ExBasi nOutlet	2. 10	4. 536
ExBasi nOutlet	2. 15	4. 754
ExBasi nOutlet	2. 20	4. 980
ExBasi nOutlet	2. 25	5. 213
ExBasi nOutlet	2. 30	5. 452
ExBasi nOutlet	2. 35	5. 696
ExBasi nOutlet	2. 40	5. 944
ExBasi nOutlet	2. 45	6. 196
ExBasi nOutlet	2. 50	6. 451
ExBasi nOutlet	2. 55	6. 708
ExBasi nOutlet	2. 60	6. 923
ExBasi nOutlet	2. 65	7. 090
ExBasi nOutlet	2. 70	7. 251
ExBasi nOutlet	2. 75	7. 408
ExBasi nOutlet	2. 80	7. 561
ExBasi nOutlet	2. 85	7. 710
ExBasi nOutlet	2. 90	7. 855
ExBasi nOutlet	2. 95	7. 997
ExBasi nOutlet	3. 00	8. 136
ExBasi nOutlet	3. 05	8. 295
ExBasi nOutlet	3. 10	8. 492
ExBasi nOutlet	3. 15	8. 726
ExBasi nOutlet	3. 20	8. 998
ExBasi nOutlet	3. 25	9. 306
ExBasi nOutlet	3. 30	9. 649
ExBasi nOutlet	3. 35	10. 027
ExBasi nOutlet	3. 40	10. 439
ExBasi nOutlet	3. 45	10. 884
ExBasi nOutlet	3. 50	11. 360
ExBasi nOutlet	3. 55	11. 868
ExBasi nOutlet	3. 60	12. 405
ExBasi nOutlet	3. 65	12. 971
ExBasi nOutlet	3. 70	13. 565
ExBasi nOutlet	3. 75	14. 185
ExBasi nOutlet	3. 80	14. 831
ExBasi nOutlet	3. 85	15. 500
ExBasi nOutlet	3. 90	16. 192
ExBasi nOutlet	3. 95	16. 906
ExBasi nOutlet	4. 00	17. 639
ExBasi nOutlet	4. 05	18. 392
ExBasi nOutlet	4. 10	19. 162
ExBasi nOutlet	4. 15	19. 948
ExBasi nOutlet	4. 20	20. 750
ExBasi nOutlet	4. 25	21. 564
ExBasi nOutlet	4. 30	22. 391
ExBasi nOutlet	4. 35	23. 228
ExBasi nOutlet	4. 40	24. 074
ExBasi nOutlet	4. 45	24. 929
ExBasi nOutlet	4. 50	25. 789
ExBasi nOutlet	4. 55	26. 655
ExBasi nOutlet	4. 60	27. 525
ExBasi nOutlet	4. 65	28. 397
ExBasi nOutlet	4. 70	28. 706
ExBasi nOutlet	4. 75	29. 235
ExBasi nOutlet	4. 80	29. 752
ExBasi nOutlet	4. 85	30. 258
ExBasi nOutlet	4. 90	30. 754
ExBasi nOutlet	4. 95	31. 241
ExBasi nOutlet	5. 00	31. 718
ExBasi nOutlet	5. 05	32. 188
ExBasi nOutlet	5. 10	32. 649
ExBasi nOutlet	5. 15	33. 103
ExBasi nOutlet	5. 20	33. 550
ExBasi nOutlet	5. 25	33. 990
ExBasi nOutlet	5. 30	34. 424
ExBasi nOutlet	5. 35	34. 852
ExBasi nOutlet	5. 40	35. 273
ExBasi nOutlet	5. 45	35. 689
ExBasi nOutlet	5. 50	36. 100
ExBasi nOutlet	5. 55	36. 539
ExBasi nOutlet	5. 60	37. 034

ExBasi nOutlet	5. 65	37. 585
ExBasi nOutlet	5. 70	38. 190
ExBasi nOutlet	5. 75	38. 848
ExBasi nOutlet	5. 80	39. 558
ExBasi nOutlet	5. 85	40. 320
ExBasi nOutlet	5. 90	41. 130
ExBasi nOutlet	5. 95	41. 989
ExBasi nOutlet	6. 00	42. 895
ExBasi nOutlet	6. 05	43. 845
ExBasi nOutlet	6. 10	44. 840
ExBasi nOutlet	6. 15	45. 876
ExBasi nOutlet	6. 20	46. 952
ExBasi nOutlet	6. 25	48. 067
ExBasi nOutlet	6. 30	49. 218
ExBasi nOutlet	6. 35	50. 405
ExBasi nOutlet	6. 40	51. 624
ExBasi nOutlet	6. 45	52. 874
ExBasi nOutlet	6. 50	54. 154
ExBasi nOutlet	6. 55	55. 461
ExBasi nOutlet	6. 60	56. 793
ExBasi nOutlet	6. 65	58. 148
ExBasi nOutlet	6. 70	59. 524
ExBasi nOutlet	6. 75	60. 919
ExBasi nOutlet	6. 80	62. 332
ExBasi nOutlet	6. 85	63. 759
ExBasi nOutlet	6. 90	65. 199
ExBasi nOutlet	6. 95	66. 650
ExBasi nOutlet	7. 00	68. 110
ExBasi nOutlet	7. 05	69. 577
ExBasi nOutlet	7. 10	71. 048
ExBasi nOutlet	7. 15	72. 522
ExBasi nOutlet	7. 20	73. 151
ExBasi nOutlet	7. 25	74. 108
ExBasi nOutlet	7. 30	75. 047
ExBasi nOutlet	7. 35	75. 968
ExBasi nOutlet	7. 40	76. 874
ExBasi nOutlet	7. 45	77. 764
ExBasi nOutlet	7. 50	78. 641
ExBasi nOutlet	7. 55	79. 504
ExBasi nOutlet	7. 60	80. 355
ExBasi nOutlet	7. 65	81. 193
ExBasi nOutlet	7. 70	82. 020
ExBasi nOutlet	7. 75	82. 836
ExBasi nOutlet	7. 80	83. 642
ExBasi nOutlet	7. 85	84. 437
ExBasi nOutlet	7. 90	85. 223
ExBasi nOutlet	7. 95	86. 000
ExBasi nOutlet	8. 00	86. 768
ExBasi nOutlet	8. 05	87. 528
ExBasi nOutlet	8. 10	88. 279
ExBasi nOutlet	8. 15	89. 023
ExBasi nOutlet	8. 20	89. 758
ExBasi nOutlet	8. 25	90. 487
ExBasi nOutlet	8. 30	91. 208
ExBasi nOutlet	8. 35	91. 923
ExBasi nOutlet	8. 40	92. 631
ExBasi nOutlet	8. 45	93. 332
ExBasi nOutlet	8. 50	94. 028
ExBasi nOutlet	8. 55	94. 717
ExBasi nOutlet	8. 60	95. 400
ExBasi nOutlet	8. 65	96. 078
ExBasi nOutlet	8. 70	96. 750
ExBasi nOutlet	8. 75	97. 416
ExBasi nOutlet	8. 80	98. 077
ExBasi nOutlet	8. 85	98. 734
ExBasi nOutlet	8. 90	99. 385
ExBasi nOutlet	8. 95	100. 031
ExBasi nOutlet	9. 00	100. 672
ExBasi nOutlet	9. 05	101. 309
ExBasi nOutlet	9. 10	101. 941
ExBasi nOutlet	9. 15	102. 569

ExBasi nOutlet	9. 20	103. 193
ExBasi nOutlet	9. 25	103. 812
ExBasi nOutlet	9. 30	104. 427
ExBasi nOutlet	9. 35	105. 038
ExBasi nOutlet	9. 40	105. 645
ExBasi nOutlet	9. 45	106. 248
ExBasi nOutlet	9. 50	106. 848
ExBasi nOutlet	9. 55	107. 443
ExBasi nOutlet	9. 60	108. 035
ExBasi nOutlet	9. 65	108. 624
ExBasi nOutlet	9. 70	109. 209
ExBasi nOutlet	9. 75	109. 790
ExBasi nOutlet	9. 80	110. 368
ExBasi nOutlet	9. 85	110. 943
ExBasi nOutlet	9. 90	111. 515
ExBasi nOutlet	9. 95	112. 083
ExBasi nOutlet	10. 00	112. 648
ExBasi nOutlet	10. 05	113. 210
ExBasi nOutlet	10. 10	113. 769
ExBasi nOutlet	10. 15	114. 325
ExBasi nOutlet	10. 20	114. 878
ExBasi nOutlet	10. 25	115. 428
ExBasi nOutlet	10. 30	115. 976
ExBasi nOutlet	10. 35	116. 520
ExBasi nOutlet	10. 40	117. 062
ExBasi nOutlet	10. 45	117. 601
ExBasi nOutlet	10. 50	118. 138
ExBasi nOutlet	10. 55	118. 672
ExBasi nOutlet	10. 60	119. 203
ExBasi nOutlet	10. 65	119. 732
ExBasi nOutlet	10. 70	120. 258
ExBasi nOutlet	10. 75	120. 782
ExBasi nOutlet	10. 80	121. 303
ExBasi nOutlet	10. 85	121. 822
ExBasi nOutlet	10. 90	122. 339
ExBasi nOutlet	10. 95	122. 853
ExBasi nOutlet	11. 00	123. 365
ExBasi nOutlet	11. 05	123. 875
ExBasi nOutlet	11. 10	124. 383
ExBasi nOutlet	11. 15	124. 888
ExBasi nOutlet	11. 20	125. 391
ExBasi nOutlet	11. 25	125. 892
ExBasi nOutlet	11. 30	126. 391
ExBasi nOutlet	11. 35	126. 888
ExBasi nOutlet	11. 40	127. 383
ExBasi nOutlet	11. 45	127. 875
ExBasi nOutlet	11. 50	128. 366
ExBasi nOutlet	11. 55	131. 238
ExBasi nOutlet	11. 60	136. 081
ExBasi nOutlet	11. 65	142. 208
ExBasi nOutlet	11. 70	149. 372
ExBasi nOutlet	11. 75	157. 431
ExBasi nOutlet	11. 80	166. 289
ExBasi nOutlet	11. 85	175. 877
ExBasi nOutlet	11. 90	186. 139
ExBasi nOutlet	11. 95	197. 031
ExBasi nOutlet	12. 00	208. 518
ExBasi nOutlet	12. 05	220. 569
ExBasi nOutlet	12. 10	233. 157
ExBasi nOutlet	12. 15	246. 259
ExBasi nOutlet	12. 20	259. 856
ExBasi nOutlet	12. 25	273. 929
ExBasi nOutlet	12. 30	288. 461
ExBasi nOutlet	12. 35	303. 439
ExBasi nOutlet	12. 40	318. 849
ExBasi nOutlet	12. 45	334. 678
ExBasi nOutlet	12. 50	350. 916
ExBasi nOutlet	12. 55	367. 552
ExBasi nOutlet	12. 60	384. 577
ExBasi nOutlet	12. 65	401. 981
ExBasi nOutlet	12. 70	419. 756

ExBasi nOutlet	12.75	437.894
ExBasi nOutlet	12.80	456.389
ExBasi nOutlet	12.85	475.232
ExBasi nOutlet	12.90	494.418
ExBasi nOutlet	12.95	513.940
ExBasi nOutlet	13.00	525.972
ExBasi nOutlet	13.05	532.754
ExBasi nOutlet	13.10	539.433
ExBasi nOutlet	13.15	546.013
ExBasi nOutlet	13.20	552.500
ExBasi nOutlet	13.25	558.897
ExBasi nOutlet	13.30	565.209
ExBasi nOutlet	13.35	571.437
ExBasi nOutlet	13.40	577.587
ExBasi nOutlet	13.45	583.660
ExBasi nOutlet	13.50	589.661
ExBasi nOutlet	13.55	595.590
ExBasi nOutlet	13.60	601.452
ExBasi nOutlet	13.65	607.248
ExBasi nOutlet	13.70	612.981
ExBasi nOutlet	13.75	618.653
ExBasi nOutlet	13.80	624.265
ExBasi nOutlet	13.85	629.820
ExBasi nOutlet	13.90	635.319

;			
; Ex Basi n			
ExBasi n	Storage	0.00	17535
ExBasi n		0.05	17647
ExBasi n		0.10	17758
ExBasi n		0.15	17870
ExBasi n		0.20	17982
ExBasi n		0.25	18093
ExBasi n		0.30	18205
ExBasi n		0.35	18317
ExBasi n		0.40	18428
ExBasi n		0.45	18540
ExBasi n		0.50	18651
ExBasi n		0.55	18763
ExBasi n		0.60	18875
ExBasi n		0.65	18986
ExBasi n		0.70	19098
ExBasi n		0.75	19210
ExBasi n		0.80	19321
ExBasi n		0.85	19433
ExBasi n		0.90	19545
ExBasi n		0.95	19656
ExBasi n		1.00	19768
ExBasi n		1.05	19873
ExBasi n		1.10	19979
ExBasi n		1.15	20085
ExBasi n		1.20	20190
ExBasi n		1.25	20296
ExBasi n		1.30	20401
ExBasi n		1.35	20507
ExBasi n		1.40	20613
ExBasi n		1.45	20718
ExBasi n		1.50	20824
ExBasi n		1.55	20929
ExBasi n		1.60	21035
ExBasi n		1.65	21140
ExBasi n		1.70	21246
ExBasi n		1.75	21352
ExBasi n		1.80	21457
ExBasi n		1.85	21563
ExBasi n		1.90	21668
ExBasi n		1.95	21774
ExBasi n		2.00	21880
ExBasi n		2.05	21991
ExBasi n		2.10	22102
ExBasi n		2.15	22213
ExBasi n		2.20	22325

ExBasi n	2. 25	22436
ExBasi n	2. 30	22547
ExBasi n	2. 35	22659
ExBasi n	2. 40	22770
ExBasi n	2. 45	22881
ExBasi n	2. 50	22993
ExBasi n	2. 55	23104
ExBasi n	2. 60	23215
ExBasi n	2. 65	23326
ExBasi n	2. 70	23438
ExBasi n	2. 75	23549
ExBasi n	2. 80	23660
ExBasi n	2. 85	23772
ExBasi n	2. 90	23883
ExBasi n	2. 95	23994
ExBasi n	3. 00	24106
ExBasi n	3. 05	24215
ExBasi n	3. 10	24325
ExBasi n	3. 15	24435
ExBasi n	3. 20	24545
ExBasi n	3. 25	24655
ExBasi n	3. 30	24765
ExBasi n	3. 35	24875
ExBasi n	3. 40	24985
ExBasi n	3. 45	25095
ExBasi n	3. 50	25205
ExBasi n	3. 55	25315
ExBasi n	3. 60	25425
ExBasi n	3. 65	25535
ExBasi n	3. 70	25645
ExBasi n	3. 75	25755
ExBasi n	3. 80	25865
ExBasi n	3. 85	25975
ExBasi n	3. 90	26085
ExBasi n	3. 95	26195
ExBasi n	4. 00	26305
ExBasi n	4. 05	26420
ExBasi n	4. 10	26536
ExBasi n	4. 15	26652
ExBasi n	4. 20	26768
ExBasi n	4. 25	26883
ExBasi n	4. 30	26999
ExBasi n	4. 35	27115
ExBasi n	4. 40	27230
ExBasi n	4. 45	27346
ExBasi n	4. 50	27462
ExBasi n	4. 55	27578
ExBasi n	4. 60	27693
ExBasi n	4. 65	27809
ExBasi n	4. 70	27925
ExBasi n	4. 75	28040
ExBasi n	4. 80	28156
ExBasi n	4. 85	28272
ExBasi n	4. 90	28388
ExBasi n	4. 95	28503
ExBasi n	5. 00	28619
ExBasi n	5. 05	28739
ExBasi n	5. 10	28860
ExBasi n	5. 15	28980
ExBasi n	5. 20	29101
ExBasi n	5. 25	29221
ExBasi n	5. 30	29342
ExBasi n	5. 35	29462
ExBasi n	5. 40	29582
ExBasi n	5. 45	29703
ExBasi n	5. 50	29823
ExBasi n	5. 55	29944
ExBasi n	5. 60	30064
ExBasi n	5. 65	30184
ExBasi n	5. 70	30305
ExBasi n	5. 75	30425

ExBasi n	5. 80	30546
ExBasi n	5. 85	30666
ExBasi n	5. 90	30787
ExBasi n	5. 95	30907
ExBasi n	6. 00	31027
ExBasi n	6. 05	31160
ExBasi n	6. 10	31292
ExBasi n	6. 15	31424
ExBasi n	6. 20	31557
ExBasi n	6. 25	31689
ExBasi n	6. 30	31821
ExBasi n	6. 35	31954
ExBasi n	6. 40	32086
ExBasi n	6. 45	32219
ExBasi n	6. 50	32351
ExBasi n	6. 55	32483
ExBasi n	6. 60	32616
ExBasi n	6. 65	32748
ExBasi n	6. 70	32880
ExBasi n	6. 75	33013
ExBasi n	6. 80	33145
ExBasi n	6. 85	33277
ExBasi n	6. 90	33410
ExBasi n	6. 95	33542
ExBasi n	7. 00	33674
ExBasi n	7. 05	33810
ExBasi n	7. 10	33945
ExBasi n	7. 15	34081
ExBasi n	7. 20	34216
ExBasi n	7. 25	34352
ExBasi n	7. 30	34487
ExBasi n	7. 35	34623
ExBasi n	7. 40	34758
ExBasi n	7. 45	34894
ExBasi n	7. 50	35029
ExBasi n	7. 55	35165
ExBasi n	7. 60	35300
ExBasi n	7. 65	35436
ExBasi n	7. 70	35571
ExBasi n	7. 75	35707
ExBasi n	7. 80	35842
ExBasi n	7. 85	35977
ExBasi n	7. 90	36113
ExBasi n	7. 95	36248
ExBasi n	8. 00	36384
ExBasi n	8. 05	36523
ExBasi n	8. 10	36662
ExBasi n	8. 15	36801
ExBasi n	8. 20	36941
ExBasi n	8. 25	37080
ExBasi n	8. 30	37219
ExBasi n	8. 35	37358
ExBasi n	8. 40	37497
ExBasi n	8. 45	37636
ExBasi n	8. 50	37776
ExBasi n	8. 55	37915
ExBasi n	8. 60	38054
ExBasi n	8. 65	38193
ExBasi n	8. 70	38332
ExBasi n	8. 75	38471
ExBasi n	8. 80	38611
ExBasi n	8. 85	38750
ExBasi n	8. 90	38889
ExBasi n	8. 95	39028
ExBasi n	9. 00	39167
ExBasi n	9. 05	39311
ExBasi n	9. 10	39455
ExBasi n	9. 15	39599
ExBasi n	9. 20	39743
ExBasi n	9. 25	39887
ExBasi n	9. 30	40031

ExBasi n	9. 35	40175
ExBasi n	9. 40	40319
ExBasi n	9. 45	40463
ExBasi n	9. 50	40607
ExBasi n	9. 55	40751
ExBasi n	9. 60	40895
ExBasi n	9. 65	41039
ExBasi n	9. 70	41183
ExBasi n	9. 75	41327
ExBasi n	9. 80	41471
ExBasi n	9. 85	41615
ExBasi n	9. 90	41759
ExBasi n	9. 95	41903
ExBasi n	10. 00	42047
ExBasi n	10. 05	42200
ExBasi n	10. 10	42353
ExBasi n	10. 15	42506
ExBasi n	10. 20	42659
ExBasi n	10. 25	42812
ExBasi n	10. 30	42965
ExBasi n	10. 35	43118
ExBasi n	10. 40	43271
ExBasi n	10. 45	43424
ExBasi n	10. 50	43577
ExBasi n	10. 55	43730
ExBasi n	10. 60	43883
ExBasi n	10. 65	44036
ExBasi n	10. 70	44189
ExBasi n	10. 75	44342
ExBasi n	10. 80	44495
ExBasi n	10. 85	44648
ExBasi n	10. 90	44801
ExBasi n	10. 95	44954
ExBasi n	11. 00	45107
ExBasi n	11. 05	45270
ExBasi n	11. 10	45432
ExBasi n	11. 15	45595
ExBasi n	11. 20	45758
ExBasi n	11. 25	45920
ExBasi n	11. 30	46083
ExBasi n	11. 35	46246
ExBasi n	11. 40	46408
ExBasi n	11. 45	46571
ExBasi n	11. 50	46734
ExBasi n	11. 55	46896
ExBasi n	11. 60	47059
ExBasi n	11. 65	47222
ExBasi n	11. 70	47384
ExBasi n	11. 75	47547
ExBasi n	11. 80	47710
ExBasi n	11. 85	47872
ExBasi n	11. 90	48035
ExBasi n	11. 95	48198
ExBasi n	12. 00	48360
ExBasi n	12. 05	48537
ExBasi n	12. 10	48713
ExBasi n	12. 15	48889
ExBasi n	12. 20	49066
ExBasi n	12. 25	49242
ExBasi n	12. 30	49419
ExBasi n	12. 35	49595
ExBasi n	12. 40	49772
ExBasi n	12. 45	49948
ExBasi n	12. 50	50124
ExBasi n	12. 55	50301
ExBasi n	12. 60	50477
ExBasi n	12. 65	50654
ExBasi n	12. 70	50830
ExBasi n	12. 75	51007
ExBasi n	12. 80	51183
ExBasi n	12. 85	51359

ExBasi n	12. 90	51536
ExBasi n	12. 95	51712
ExBasi n	13. 00	51889
ExBasi n	13. 05	52065
ExBasi n	13. 10	52242
ExBasi n	13. 15	52418
ExBasi n	13. 20	52594
ExBasi n	13. 25	52771
ExBasi n	13. 30	52947
ExBasi n	13. 35	53124
ExBasi n	13. 40	53300
ExBasi n	13. 45	53477
ExBasi n	13. 50	53653
ExBasi n	13. 55	53829
ExBasi n	13. 60	54006
ExBasi n	13. 65	54182
ExBasi n	13. 70	54359
ExBasi n	13. 75	54535
ExBasi n	13. 80	54712
ExBasi n	13. 85	54888
ExBasi n	13. 90	55064

[TIMESERIES]

```
;; Name          Date      Time      Value
;; -----
; Lower Otay Rain Gage from San Diego County
LWR0tay          FILE "lower_otay.prn"
```

[REPORT]

```
;; Reporting Options
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL
```

[TAGS]

[MAP]

```
DIMENSIONS 0.000 0.000 10000.000 10000.000
Units      None
```

[COORDINATES]

```
;; Node          X-Coord          Y-Coord
;; -----
POC-1            1488. 812        3184. 165
Stor-1           1480. 207        4268. 503
```

[VERTICES]

```
;; Link          X-Coord          Y-Coord
;; -----
```

[Polygons]

```
;; Subcatchment X-Coord          Y-Coord
;; -----
ExBasi n        1497. 418        4991. 394
Village7, R-8   2332. 186        4991. 394
OtayVillage7    335. 628         4991. 394
OtayVillage7    335. 628         4991. 394
```

[SYMBOLS]

```
;; Gage          X-Coord          Y-Coord
;; -----
LWR0tay         1531. 842        5456. 110
```

SWMM Model - PR Condition Output Report

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.014)

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

 Analysis Options

Flow Units CFS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed NO
 Water Quality NO
 Infiltration Method GREEN_AMPT
 Flow Routing Method KINWAVE
 Starting Date 08/28/1951 00:00:00
 Ending Date 03/16/2008 23:00:00
 Antecedent Dry Days 0.0
 Report Time Step 01:00:00
 Wet Time Step 00:15:00
 Dry Time Step 04:00:00
 Routing Time Step 60.00 sec

	Volume acre-feet	Depth inches
Runoff Quantity Continuity		
*****	-----	-----
Total Precipitation	7543.327	592.990
Evaporation Loss	904.241	71.083
Infiltration Loss	3525.937	277.178
Surface Runoff	3151.641	247.754
Final Storage	0.330	0.026
Continuity Error (%)	-0.515	

	Volume acre-feet	Volume 10 ⁶ gal
Flow Routing Continuity		
*****	-----	-----
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	3151.639	1027.009
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	3137.994	1022.562
Flooding Loss	0.000	0.000
Evaporation Loss	12.151	3.960
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.152	0.049
Continuity Error (%)	0.043	

 Highest Flow Instability Indexes

 All links are stable.

 Routing Time Step Summary

Minimum Time Step : 60.00 sec
 Average Time Step : 60.00 sec
 Maximum Time Step : 60.00 sec
 Percent in Steady State : 0.00
 Average Iterations per Step : 1.00
 Percent Not Converging : 0.00

 Subcatchment Runoff Summary

Peak Runoff Runoff Coeff Subcatchment CFS	Total Precip in	Total Runon in	Total Evap in	Total Infil in	Imperv Runoff in	Perv Runoff in	Total Runoff in	Total Runoff 10^6 gal
ExBasin 79.33 0.909	592.99	13467.52	120.55	1197.01	209.24	12567.69	12776.92	1026.93
Village7, R-8 10.43 0.499	592.99	0.00	62.51	241.17	239.04	56.57	295.61	110.93
OtayVillage7 75.78 0.444	592.99	0.00	70.88	260.80	223.05	40.28	263.33	971.51

 Node Depth Summary

Node	Type	Average Depth Feet	Maximum Depth Feet	Maximum HGL Feet	Time of Max Occurrence days hr: min	Reported Max Depth Feet
POC-1	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
Stor-1	STORAGE	0.04	6.82	396.82	16971 17:31	6.40

 Node Inflow Summary

Node	Type	Maximum Lateral Inflow CFS	Maximum Total Inflow CFS	Time of Max Occurrence days hr: min	Lateral Inflow Volume 10^6 gal	Total Inflow Volume 10^6 gal	Flow Balance Error Percent
POC-1	OUTFALL	0.00	62.83	16971 17:31	0	1.02e+03	0.000
Stor-1	STORAGE	79.33	79.33	16971 17:16	1.03e+03	1.03e+03	0.043

 Node Flooding Summary

No nodes were flooded.

 Storage Volume Summary

Storage Unit	Average Volume 1000 ft3	Avg Pcnt Full	Evap Pcnt Loss	Exfil Pcnt Loss	Maximum Volume 1000 ft3	Max Pcnt Full	Time of Max Occurrence days hr: min	Maximum Outflow CFS
Stor-1	0.775	0	0	0	171.244	52	16971 17:30	62.84

 Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow CFS	Max Flow CFS	Total Volume 10^6 gal
POC-1	15.09	0.51	62.83	1022.486
System	15.09	0.51	62.83	1022.486

 Link Flow Summary

Link	Type	Maximum Flow CFS	Time of Max Occurrence days hr: min	Maximum Veloc ft/sec	Max/ Full Flow	Max/ Full Depth
1	DUMMY	62.83	16971 17:31			

 Conduit Surcharge Summary

No conduits were surcharged.

Analysis begun on: Thu Apr 6 10:59:56 2023
 Analysis ended on: Thu Apr 6 11:00:24 2023
 Total elapsed time: 00:00:28

POC 2 – Pre-Developed Condition

LWROtay



Village7+DMA2&3



POC-2



SWMM Model - EX Condition Input Data

[TITLE]

::Project Title/Notes

[OPTIONS]

```

::Option      Value
FLOW_UNITS    CFS
INFILTRATION  GREEN_AMPT
FLOW_ROUTING  KINWAVE
LINK_OFFSETS  DEPTH
MIN_SLOPE     0
ALLOW_PONDING NO
SKIP_STEADY_STATE NO
    
```

```

START_DATE    08/28/1951
START_TIME    00:00:00
REPORT_START_DATE 08/28/1951
REPORT_START_TIME 00:00:00
END_DATE      03/16/2008
END_TIME      23:00:00
SWEEP_START   01/01
SWEEP_END     12/31
DRY_DAYS      0
REPORT_STEP   01:00:00
WET_STEP      00:15:00
DRY_STEP      04:00:00
ROUTING_STEP  0:01:00
RULE_STEP     00:00:00
    
```

```

INERTIAL_DAMPING PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQUATION H-W
VARIABLE_STEP    0.75
LENGTHENING_STEP 0
MIN_SURFAREA     12.557
MAX_TRIALS       8
HEAD_TOLERANCE   0.005
SYS_FLOW_TOL     5
LAT_FLOW_TOL     5
MINIMUM_STEP     0.5
THREADS          1
    
```

[EVAPORATION]

```

::Data Source Parameters
::-----
MONTHLY      0.06  0.08  0.11  0.16  0.18  0.21  0.21  0.20  0.16  0.12  0.08  0.06
DRY_ONLY     NO
    
```

[RAINGAGES]

```

::Name      Format  Interval SCF      Source
::-----
LWR0tay     INTENSITY 1:00    1.0    TIMESERIES LWR0tay
    
```

[SUBCATCHMENTS]

```

::Name      Rain Gage      Outlet      Area      %Imperv  Width  %Slope  CurbLen  SnowPack
::-----
;Proposed Developed Area - Village 7 R-8
Village7+DMA2&3 LWR0tay      POC-2      89.63  64.9  1190  3.40  0
    
```

[SUBAREAS]

```

::Subcatchment N-Imperv N-Perv  S-Imperv S-Perv  PctZero  RouteTo  PctRouted
::-----
Village7+DMA2&3 0.012  0.15  0.05  0.10  25  OUTLET
    
```

[INFILTRATION]

```

::Subcatchment Suction  Ksat  IMD
::-----
Village7+DMA2&3 9  0.01875  0.33
    
```

[OUTFALLS]

```

::Name      Elevation Type      Stage Data      Gated  Route To
::-----
    
```

POC-2 0 FREE

[TIMESERIES]

```
;; Name      Date      Time      Value
;; -----
; Lower Otay Rain Gage from San Diego County
LWR0tay      FILE "lower_otay.prn"
```

[REPORT]

```
;; Reporting Options
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL
```

[TAGS]

[MAP]

```
DIMENSIONS 0.000 0.000 10000.000 10000.000
Units      None
```

[COORDINATES]

```
;; Node      X-Coord      Y-Coord
;; -----
POC-2        1488.812      3184.165
```

[VERTICES]

```
;; Link      X-Coord      Y-Coord
;; -----
```

[Polygons]

```
;; Subcatchment X-Coord      Y-Coord
;; -----
Village7+DMA2&3 1503.198      4989.339
```

[SYMBOLS]

```
;; Gage      X-Coord      Y-Coord
;; -----
LWR0tay      1663.113      7089.552
```

SWMM Model - EX Condition Output Report

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.014)

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

 Analysis Options

Flow Units CFS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing NO
 Water Quality NO
 Infiltration Method GREEN_AMPT
 Starting Date 08/28/1951 00:00:00
 Ending Date 03/16/2008 23:00:00
 Antecedent Dry Days 0.0
 Report Time Step 01:00:00
 Wet Time Step 00:15:00
 Dry Time Step 04:00:00

*****	Volume	Depth
Runoff Quantity Continuity	acre-feet	inches
*****	-----	-----
Total Precipitation	4429.141	592.990
Evaporation Loss	657.141	87.980
Infiltration Loss	1268.424	169.821
Surface Runoff	2526.138	338.209
Final Storage	0.187	0.025
Continuity Error (%)	-0.514	

*****	Volume	Volume
Flow Routing Continuity	acre-feet	10^6 gal
*****	-----	-----
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	2526.138	823.180
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	2526.138	823.180
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.000	

 Subcatchment Runoff Summary

-----	Total	Total	Total	Total	Imperv	Perv	Total	Total
Peak Runoff	Precip	Runon	Evap	Infil	Runoff	Runoff	Runoff	Runoff
Runoff Coeff	in	in	in	in	in	in	in	10^6 gal
Subcatchment								


CFS

Village7&R-4	592.99	0.00	87.98	169.82	307.85	30.36	338.21	823.12
60.64 0.570								

Analysis begun on: Thu Apr 6 08:42:54 2023
Analysis ended on: Thu Apr 6 08:43:13 2023
Total elapsed time: 00:00:19

POC 2 – Mass Condition

LWROtay


Village7+DMA2&3


POC-2


SWMM Model - PR Condition Input Data

[TITLE]

::Project Title/Notes

[OPTIONS]

```

::Option      Value
FLOW_UNITS    CFS
INFILTRATION  GREEN_AMPT
FLOW_ROUTING  KINWAVE
LINK_OFFSETS  DEPTH
MIN_SLOPE     0
ALLOW_PONDING NO
SKIP_STEADY_STATE NO
    
```

```

START_DATE    08/28/1951
START_TIME    00:00:00
REPORT_START_DATE 08/28/1951
REPORT_START_TIME 00:00:00
END_DATE      03/16/2008
END_TIME      23:00:00
SWEEP_START   01/01
SWEEP_END     12/31
DRY_DAYS      0
REPORT_STEP   01:00:00
WET_STEP      00:15:00
DRY_STEP      04:00:00
ROUTING_STEP  0:01:00
RULE_STEP     00:00:00
    
```

```

INERTIAL_DAMPING PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQUATION H-W
VARIABLE_STEP    0.75
LENGTHENING_STEP 0
MIN_SURFAREA     12.557
MAX_TRIALS       8
HEAD_TOLERANCE   0.005
SYS_FLOW_TOL     5
LAT_FLOW_TOL     5
MINIMUM_STEP     0.5
THREADS          1
    
```

[EVAPORATION]

```

::Data Source Parameters
::-----
MONTHLY      0.06  0.08  0.11  0.16  0.18  0.21  0.21  0.20  0.16  0.12  0.08  0.06
DRY_ONLY     NO
    
```

[RAINGAGES]

```

::Name      Format   Interval SCF      Source
::-----
LWR0tay     INTENSITY 1:00    1.0     TIMESERIES LWR0tay
    
```

[SUBCATCHMENTS]

```

::Name      Rain Gage      Outlet      Area      %Imperv  Width  %Slope  CurbLen  SnowPack
::-----
;Proposed Developed Area - Village 7 R-8
Village7+DMA2&3 LWR0tay      POC-2      89.63  65.1  1190  3.40  0
    
```

[SUBAREAS]

```

::Subcatchment N-Imperv N-Perv  S-Imperv S-Perv  PctZero  RouteTo  PctRouted
::-----
Village7+DMA2&3 0.012  0.15  0.05  0.10  25  OUTLET
    
```

[INFILTRATION]

```

::Subcatchment Suction  Ksat  IMD
::-----
Village7+DMA2&3 9  0.01875  0.33
    
```

[OUTFALLS]

```

::Name      Elevation Type      Stage Data      Gated  Route To
::-----
    
```

POC-2 0 FREE

[TIMESERIES]

```
;; Name      Date      Time      Value
;; -----
; Lower Otay Rain Gage from San Diego County
LWR0tay      FILE "lower_otay.prn"
```

[REPORT]

```
;; Reporting Options
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL
```

[TAGS]

[MAP]

```
DIMENSIONS 0.000 0.000 10000.000 10000.000
Units      None
```

[COORDINATES]

```
;; Node      X-Coord      Y-Coord
;; -----
POC-2        1488.812     3184.165
```

[VERTICES]

```
;; Link      X-Coord      Y-Coord
;; -----
```

[Polygons]

```
;; Subcatchment X-Coord      Y-Coord
;; -----
Village7+DMA2&3 1503.198     4989.339
```

[SYMBOLS]

```
;; Gage      X-Coord      Y-Coord
;; -----
LWR0tay      1663.113     7089.552
```

SWMM Model - PR Condition Output Report

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.014)

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

 Analysis Options

Flow Units CFS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing NO
 Water Quality NO
 Infiltration Method GREEN_AMPT
 Starting Date 08/28/1951 00:00:00
 Ending Date 03/16/2008 23:00:00
 Antecedent Dry Days 0.0
 Report Time Step 01:00:00
 Wet Time Step 00:15:00
 Dry Time Step 04:00:00

*****	Volume	Depth
Runoff Quantity Continuity	acre-feet	inches
*****	-----	-----
Total Precipitation	4429.141	592.990
Evaporation Loss	658.698	88.189
Infiltration Loss	1260.938	168.819
Surface Runoff	2532.003	338.994
Final Storage	0.188	0.025
Continuity Error (%)	-0.512	

*****	Volume	Volume
Flow Routing Continuity	acre-feet	10^6 gal
*****	-----	-----
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	2532.003	825.091
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	2532.003	825.091
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000
Continuity Error (%)	0.000	

 Subcatchment Runoff Summary

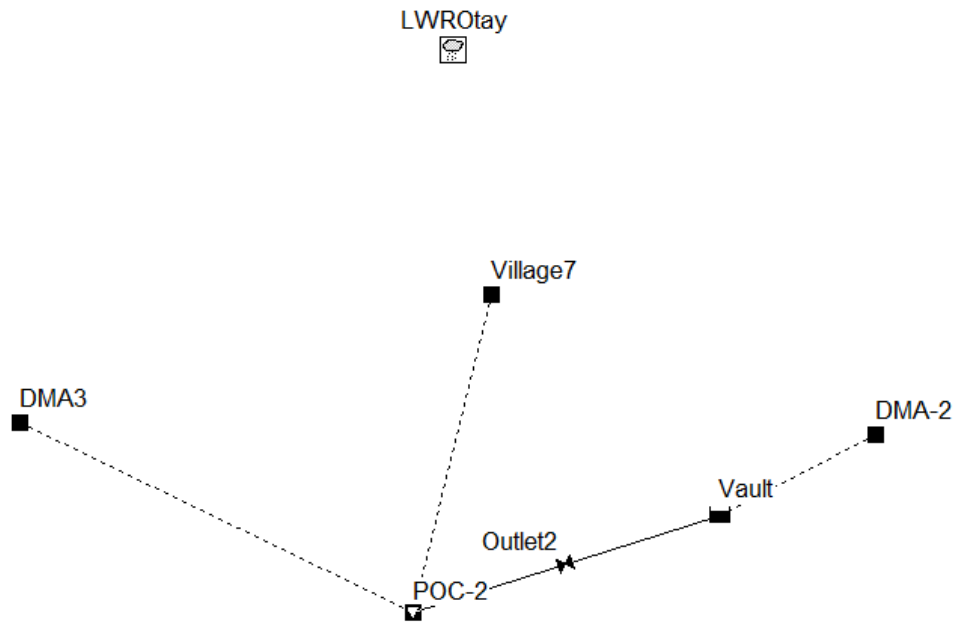
-----	Total	Total	Total	Total	Imperv	Perv	Total	Total
Peak Runoff	Precip	Runon	Evap	Infil	Runoff	Runoff	Runoff	Runoff
Runoff Coeff	in	in	in	in	in	in	in	10^6 gal
Subcatchment								

CFS

Village7+DMA2&3	592.99	0.00	88.19	168.82	308.77	30.22	338.99	825.03
60.72 0.572								

Analysis begun on: Thu Apr 6 10:44:29 2023
Analysis ended on: Thu Apr 6 10:44:49 2023
Total elapsed time: 00:00:20

POC 2 – Developed Condition



SWMM Model - PR Condition Input Data

[TITLE]

:: Project Title/Notes

[OPTIONS]

```

:: Option      Value
FLOW_UNITS    CFS
INFILTRATION  GREEN_AMPT
FLOW_ROUTING  KINWAVE
LINK_OFFSETS  DEPTH
MIN_SLOPE     0
ALLOW_PONDING NO
SKIP_STEADY_STATE NO
    
```

```

START_DATE    08/28/1951
START_TIME    00:00:00
REPORT_START_DATE 08/28/1951
REPORT_START_TIME 00:00:00
END_DATE      03/16/2008
END_TIME      23:00:00
SWEEP_START   01/01
SWEEP_END     12/31
DRY_DAYS      0
REPORT_STEP   01:00:00
WET_STEP      00:15:00
DRY_STEP      04:00:00
ROUTING_STEP  0:01:00
RULE_STEP     00:00:00
    
```

```

INERTIAL_DAMPING PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQUATION H-W
VARIABLE_STEP    0.75
LENGTHENING_STEP 0
MIN_SURFAREA     12.557
MAX_TRIALS       8
HEAD_TOLERANCE   0.005
SYS_FLOW_TOL     5
LAT_FLOW_TOL     5
MINIMUM_STEP     0.5
THREADS          1
    
```

[EVAPORATION]

```

:: Data Source Parameters
::-----
MONTHLY      0.06  0.08  0.11  0.16  0.18  0.21  0.21  0.20  0.16  0.12  0.08  0.06
DRY_ONLY     NO
    
```

[RAINGAGES]

```

:: Name      Format  Interval SCF      Source
::-----
LWR0tay     INTENSITY 1:00    1.0    TIMESERIES LWR0tay
    
```

[SUBCATCHMENTS]

```

:: Name      Rain Gage      Outlet      Area      %Imperv  Width  %Slope  CurbLen  SnowPack
::-----
Proposed Developed Area - Village 7 R-8
Village7    LWR0tay      POC-2      86.52     67.3     1193    3.40    0
DMA3        LWR0tay      POC-2      0.34      49.5     675     2.90    0
DMA-2       LWR0tay      Vault      2.85      78       827     1.25    0
    
```

[SUBAREAS]

```

:: Subcatchment N-Imperv N-Perv  S-Imperv S-Perv  PctZero  RouteTo  PctRouted
::-----
Village7       0.012   0.15   0.05    0.10    25       OUTLET
DMA3            0.012   0.15   0.05    0.10    25       OUTLET
DMA-2          0.012   0.15   0.05    0.10    25       OUTLET
    
```

[INFILTRATION]

```

:: Subcatchment Suction  Ksat  IMD
::-----
Village7       9       0.01875  0.33
    
```

DMA3 9 0.01875 0.33
 DMA-2 9 0.01875 0.33

[LID_CONTROLS]

```

;; Name Type/Layer Parameters
-----
BF-1-1 BC
BF-1-1 SURFACE 7.23 0.0 0.1 0.01 5
BF-1-1 SOIL 24 0.4 0.2 0.1 5 5 1.5
BF-1-1 STORAGE 12 0.67 0 0
BF-1-1 DRAIN 5.6496 0.5 0 6 0 0
    
```

[LID_USAGE]

```

;; Subcatchment LID Process Number Area Width InitSat FromImp ToPerv RptFile
   DrainTo      FromPerv
-----
    
```

[OUTFALLS]

```

;; Name Elevation Type Stage Data Gated Route To
-----
POC-2 0 FREE NO
    
```

[STORAGE]

```

;; Name Elev. MaxDepth InitDepth Shape Curve Name/Params N/A Fevap Psi
   Ksat   IMD
-----
Vault 0 5 0 TABULAR Vault 0 0
    
```

[OUTLETS]

```

;; Name From Node To Node Offset Type QTable/Qcoeff Qexpon Gated
-----
Outlet2 Vault POC-2 0 TABULAR/DEPTH Outlet1 NO
    
```

[CURVES]

```

;; Name Type X-Value Y-Value
-----
Outlet1 Rating 0.0 0.000
Outlet1 0.1 0.012
Outlet1 0.2 0.034
Outlet1 0.3 0.046
Outlet1 0.4 0.056
Outlet1 0.5 0.0639
Outlet1 0.6 0.0711
Outlet1 0.7 0.0776
Outlet1 0.8 0.0837
Outlet1 0.9 0.0893
Outlet1 1.0 0.0946
Outlet1 1.1 0.0996
Outlet1 1.2 0.1044
Outlet1 1.3 0.1089
Outlet1 1.4 0.1133
Outlet1 1.5 0.1175
Outlet1 1.6 0.1216
Outlet1 1.7 0.1255
Outlet1 1.8 0.1293
Outlet1 1.9 0.1330
Outlet1 2.0 0.1366
Outlet1 2.1 0.1402
Outlet1 2.2 0.1436
Outlet1 2.3 0.1469
Outlet1 2.4 0.1502
Outlet1 2.5 0.1534
Outlet1 2.6 0.1566
Outlet1 2.7 0.1596
Outlet1 2.8 0.1908
Outlet1 2.9 0.2619
    
```

Outlet1	3.0	0.3162
Outlet1	3.1	0.3529
Outlet1	3.2	0.3842
Outlet1	3.3	0.4120
Outlet1	3.4	0.4373
Outlet1	3.5	0.4608
Outlet1	3.6	0.4828
Outlet1	3.7	0.5035
Outlet1	3.8	0.5233
Outlet1	3.9	0.5421
Outlet1	4.0	0.5602
Outlet1	4.1	0.6407
Outlet1	4.2	0.8180
Outlet1	4.3	1.0129
Outlet1	4.4	1.1306
Outlet1	4.5	1.2305
Outlet1	4.6	1.3194
Outlet1	4.7	1.6144
Outlet1	4.8	2.5878
Outlet1	4.9	3.9393
Outlet1	5.0	5.5771
Outlet1	5.1	7.4558
Outlet1	5.2	9.5464
Outlet1	5.3	11.8286
Outlet1	5.4	14.2872
Outlet1	5.5	16.9098
Outlet1	5.6	19.6866
Outlet1	5.7	22.6094
Outlet1	5.8	25.6709
Outlet1	5.9	28.8651
Outlet1	6.0	32.1867
;		
Vault	Storage	0 2500
Vault		6 2500

[TIMESERIES]

```
;; Name      Date      Time      Value
;; -----
; Lower Otay Rain Gage from San Diego County
LWR0tay      FILE "lower_otay.prn"
```

[REPORT]

```
;; Reporting Options
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL
```

[TAGS]

[MAP]

```
DI MENSIONS 0.000 0.000 10000.000 10000.000
Units      None
```

[COORDINATES]

```
;; Node      X-Coord      Y-Coord
;; -----
POC-2      1417.910      3891.258
Vault      3166.466      4439.083
```

[VERTICES]

```
;; Link      X-Coord      Y-Coord
;; -----
```

[Polygons]

```
;; Subcatchment X-Coord      Y-Coord
;; -----
Village7      1865.672      5703.625
DMA3           -826.162      4974.182
DMA-2         4059.107      4909.530
```

[SYMBOLS]

:: Gage	X-Coord	Y-Coord
LWR0tay	1663.113	7089.552

SWMM Model - PR Condition Output Report

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.1 (Build 5.1.014)

 NOTE: The summary statistics displayed in this report are based on results found at every computational time step, not just on results from each reporting time step.

 Analysis Options

Flow Units CFS
 Process Models:
 Rainfall/Runoff YES
 RDII NO
 Snowmelt NO
 Groundwater NO
 Flow Routing YES
 Ponding Allowed NO
 Water Quality NO
 Infiltration Method GREEN_AMPT
 Flow Routing Method KINWAVE
 Starting Date 08/28/1951 00:00:00
 Ending Date 03/16/2008 23:00:00
 Antecedent Dry Days 0.0
 Report Time Step 01:00:00
 Wet Time Step 00:15:00
 Dry Time Step 04:00:00
 Routing Time Step 60.00 sec

*****	Volume	Depth
Runoff Quantity Continuity	acre-feet	inches
*****	-----	-----
Total Precipitation	4433.094	592.990
Evaporation Loss	673.021	90.026
Infiltration Loss	1163.827	155.679
Surface Runoff	2620.259	350.497
Final Storage	0.194	0.026
Continuity Error (%)	-0.546	

*****	Volume	Volume
Flow Routing Continuity	acre-feet	10^6 gal
*****	-----	-----
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	2620.259	853.850
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	2620.230	853.841
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.006	0.002
Continuity Error (%)	0.001	

 Highest Flow Instability Indexes

 All links are stable.

 Routing Time Step Summary

Minimum Time Step : 60.00 sec
 Average Time Step : 60.00 sec
 Maximum Time Step : 60.00 sec
 Percent in Steady State : 0.00
 Average Iterations per Step : 1.00
 Percent Not Converging : 0.00

 Subcatchment Runoff Summary

Peak Runoff Runoff Coeff Subcatchment CFS	Total Precip in	Total Runon in	Total Evap in	Total Infil in	Imperv Runoff in	Perv Runoff in	Total Runoff in	Total Runoff 10^6 gal
Village7 60.02 0.588	592.99	0.00	90.23	157.20	319.24	29.30	348.55	818.85
DMA3 0.26 0.530	592.99	0.00	60.45	228.98	250.97	63.52	314.49	2.90
DMA-2 2.31 0.698	592.99	0.00	87.23	100.72	388.08	25.88	413.96	32.04

 Node Depth Summary

Node	Type	Average Depth Feet	Maximum Depth Feet	Maximum HGL Feet	Time of Max Occurrence days hr:mi n	Reported Max Depth Feet
POC-2	OUTFALL	0.00	0.00	0.00	0 00:00	0.00
Vault	STORAGE	0.02	4.64	4.64	16971 17:08	4.55

 Node Inflow Summary

Node	Type	Maximum Lateral Inflow CFS	Maximum Total Inflow CFS	Time of Max Occurrence days hr:mi n	Lateral Inflow Volume 10^6 gal	Total Inflow Volume 10^6 gal	Flow Balance Error Percent
POC-2	OUTFALL	60.29	61.14	16971 17:01	822	854	0.000
Vault	STORAGE	2.31	2.31	17230 06:01	32	32	0.023

 Node Flooding Summary

No nodes were flooded.

 Storage Volume Summary

Storage Unit	Average Volume 1000 ft3	Avg Pcnt Full	Evap Pcnt Loss	Exfil Pcnt Loss	Maximum Volume 1000 ft3	Max Pcnt Full	Time of Max Occurrence days hr: min	Maximum Outflow CFS
Vault	0.057	0	0	0	11.603	93	16971 17:07	1.44

 Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow CFS	Max Flow CFS	Total Volume 10^6 gal
POC-2	7.96	0.80	61.14	853.778
System	7.96	0.80	61.14	853.778

 Link Flow Summary

Link	Type	Maximum Flow CFS	Time of Max Occurrence days hr: min	Maximum Veloc ft/sec	Max/ Full Flow	Max/ Full Depth
Outlet2	DUMMY	1.44	16971 17:08			

 Conduit Surcharge Summary

No conduits were surcharged.

Analysis begun on: Thu Apr 6 10:15:52 2023
 Analysis ended on: Thu Apr 6 10:16:21 2023
 Total elapsed time: 00:00:29

EXPLANATION OF SELECTED VARIABLES

Parameters for the pre- and post-development models include soil type group D in accordance with the San Diego County Hydrology Manual and the USGS Soil Survey. Suction head, conductivity and initial deficit corresponds to average values expected for the soil types, according to sources consulted, professional experience, and approximate values obtained by the City of Chula Vista BMP manual Appendix G.

Selection of a Kinematic Approach: As the continuous model is based on hourly rainfall, and the time of concentration for the pre-development and post-development conditions is significantly smaller than 60 minutes, precise routing of the flows through the impervious surfaces, the underdrain pipe system, and the discharge pipe was considered unnecessary. The truncation error of the precipitation into hourly steps is much more significant than the precise routing in a system where the time of concentration is much smaller than 1 hour.

V. HYDROMODIFICATION WATERSHED MAPS

SEE ATTCHMENT 2A FOR HMP EXHIBITS

Project Name/_____

ATTACHMENT 3

Structural BMP Maintenance Information Hydromodification Control Measures

Project Name/_____

Use this checklist to ensure the required information has been included in the Structural BMP Maintenance Information Attachment:

Attachment 3: For private entity operation and maintenance, Attachment 3 must include a Storm Water Management Facilities Maintenance Agreement with Grant of Access and Covenant's ("Maintenance Agreement") Template can be found at the following link (also refer to Chapter 8.2.1 for more information's):

The following information must be included in the exhibits attached to the Maintenance Agreement:

- Vicinity map (Depiction of Project Site)
- Legal Description for Project Site
- Site design BMPs for which DCV reduction is claimed for meeting the pollutant
- control obligations.
- BMP and HMP type, location, type, manufacture model, and dimensions, specifications, cross section
- LID features such as (permeable paver and LS location, dim, SF).
- Maintenance recommendations and frequency

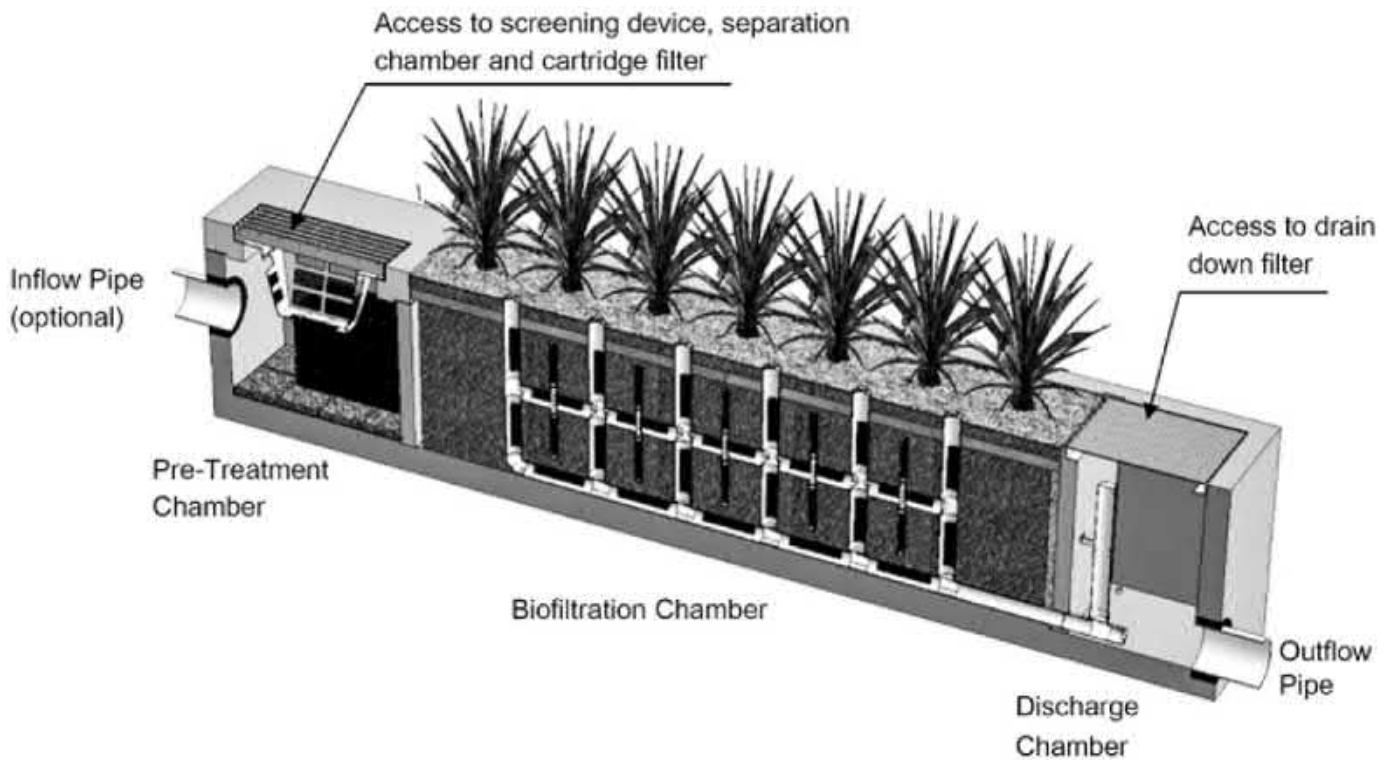
Will be provided during final engineering

Modular Wetlands[®] Linear Operation & Maintenance Manual



Maintenance Summary

- Remove Trash from Screening Device – average maintenance interval is 6 to 12 months.
 - (5 minute average service time).
- Remove Sediment from Separation Chamber – average maintenance interval is 12 to 24 months.
 - (10 minute average service time).
- Replace Cartridge Filter Media – average maintenance interval 12 to 24 months.
 - (10-15 minute per cartridge average service time).
- Replace Drain Down Filter Media – average maintenance interval is 12 to 24 months.
 - (5 minute average service time).
- Trim Vegetation – average maintenance interval is 6 to 12 months.
 - (Service time varies).



System Diagram

Maintenance Procedures

Screening Device

1. Remove grate or manhole cover to gain access to the screening device in the Pre- Treatment Chamber. Vault type units do not have screening device. Maintenance can be performed without entry.
2. Remove all pollutants collected by the screening device. Removal can be done manually or with the use of a vacuum truck.
3. Screening device can easily be removed from the Pre-Treatment Chamber to gain access to separation chamber and media filters below. Replace grate or manhole cover when completed.

Separation Chamber

1. Perform maintenance procedures of screening device listed above before maintaining the separation chamber.
2. With a pressure washer, spray down pollutants accumulated on walls and cartridge filters.
3. Vacuum out Separation Chamber and remove all accumulated pollutants. Replace screening device, grate or manhole cover when completed.

Cartridge Filters

1. Perform maintenance procedures on screening device and separation chamber before maintaining cartridge filters.
2. Enter separation chamber.
3. Unscrew the two bolts holding the lid on each cartridge filter and remove lid.
4. Remove each of 4 to 8 media cages holding the media in place.
5. Spray down the cartridge filter to remove any accumulated pollutants.
6. Vacuum out old media and accumulated pollutants.
7. Reinstall media cages and fill with new media from manufacturer or outside supplier. Manufacturer will provide specification of media and sources to purchase.
8. Replace the lid and tighten down bolts. Replace screening device, grate or manhole cover when completed.

Drain Down Filter

1. Remove hatch or manhole cover over discharge chamber and enter chamber. Entry into chambers may require confined space training based on state and local regulations.
2. Unlock and lift drain down filter housing and remove old media block. Replace with new media block. Lower drain down filter housing and lock into place.
3. Exit chamber and replace hatch or manhole cover.

Maintenance Notes

1. Following maintenance and/or inspection, it is recommended the maintenance operator prepare a maintenance/inspection record. The record should include any maintenance activities performed, amount and description of debris collected, and condition of the system and its various filter mechanisms.
2. The owner should keep maintenance/inspection record(s) for a minimum of five years from the date of maintenance. These records should be made available to the governing municipality for inspection upon request at any time.
3. Transport all debris, trash, organics and sediments to approved facility for disposal in accordance with local and state requirements.
4. Entry into chambers may require confined space training based on state and local regulations.
5. No fertilizer shall be used in the Biofiltration Chamber.
6. Irrigation should be provided as recommended by manufacturer and/or landscape architect. Amount of irrigation required is dependent on plant species. Some plants may require irrigation.

Maintenance Procedure Illustration

Screening Device

The screening device is located directly under the manhole or grate over the Pre-Treatment Chamber. It's mounted directly underneath for easy access and cleaning. Device can be cleaned by hand or with a vacuum truck.



Separation Chamber

The separation chamber is located directly beneath the screening device. It can be quickly cleaned using a vacuum truck or by hand. A pressure washer is useful to assist in the cleaning process.



Cartridge Filters

The cartridge filters are located in the Pre-Treatment chamber connected to the wall adjacent to the biofiltration chamber. The cartridges have removable tops to access the individual media filters. Once the cartridge is open media can be easily removed and replaced by hand or a vacuum truck.



Drain Down Filter

The drain down filter is located in the Discharge Chamber. The drain filter unlocks from the wall mount and hinges up. Remove filter block and replace with new block.



Trim Vegetation

Vegetation should be maintained in the same manner as surrounding vegetation and trimmed as needed. No fertilizer shall be used on the plants. Irrigation per the recommendation of the manufacturer and or landscape architect. Different types of vegetation requires different amounts of irrigation.





Inspection Report Modular Wetlands Linear

Project Name _____

For Office Use Only
(Reviewed By) _____
(Date) _____ Office personnel to complete section to the left.

Project Address _____ (city) (Zip Code)

Owner / Management Company _____

Contact _____ Phone () - _____

Inspector Name _____ Date ____ / ____ / _____ Time _____ AM / PM

Type of Inspection Routine Follow Up Complaint Storm Storm Event in Last 72-hours? No Yes

Weather Condition _____ Additional Notes _____

Inspection Checklist

Modular Wetland System Type (Curb, Grate or UG Vault): _____ Size (22', 14' or etc.): _____

Structural Integrity:	Yes	No	Comments
Damage to pre-treatment access cover (manhole cover/grate) or cannot be opened using normal lifting pressure?			
Damage to discharge chamber access cover (manhole cover/grate) or cannot be opened using normal lifting pressure?			
Does the MWS unit show signs of structural deterioration (cracks in the wall, damage to frame)?			
Is the inlet/outlet pipe or drain down pipe damaged or otherwise not functioning properly?			
Working Condition:			
Is there evidence of illicit discharge or excessive oil, grease, or other automobile fluids entering and clogging the unit?			
Is there standing water in inappropriate areas after a dry period?			
Is the filter insert (if applicable) at capacity and/or is there an accumulation of debris/trash on the shelf system?			
Does the depth of sediment/trash/debris suggest a blockage of the inflow pipe, bypass or cartridge filter? If yes specify which one in the comments section. Note depth of accumulation in in pre-treatment chamber.			Depth:
Does the cartridge filter media need replacement in pre-treatment chamber and/or discharge chamber?			Chamber:
Any signs of improper functioning in the discharge chamber? Note issues in comments section.			
Other Inspection Items:			
Is there an accumulation of sediment/trash/debris in the wetland media (if applicable)?			
Is it evident that the plants are alive and healthy (if applicable)? Please note Plant Information below.			
Is there a septic or foul odor coming from inside the system?			

Waste:	Yes	No
Sediment / Silt / Clay		
Trash / Bags / Bottles		
Green Waste / Leaves / Foliage		

Recommended Maintenance	
No Cleaning Needed	
Schedule Maintenance as Planned	
Needs Immediate Maintenance	

Plant Information	
Damage to Plants	
Plant Replacement	
Plant Trimming	

Additional Notes: _____



Cleaning and Maintenance Report Modular Wetlands Linear

Project Name _____

For Office Use Only

(Reviewed By) _____

(Date) _____

Office personnel to complete section to the left.

Project Address _____ (city) (Zip Code)

Owner / Management Company _____

Contact _____

Phone () - _____

Inspector Name _____

Date ____ / ____ / ____ Time _____ AM / PM

Type of Inspection Routine Follow Up Complaint

Storm Storm Event in Last 72-hours? No Yes

Weather Condition _____

Additional Notes _____

Site Map #	GPS Coordinates of Insert	Manufacturer / Description / Sizing	Trash Accumulation	Foliage Accumulation	Sediment Accumulation	Total Debris Accumulation	Condition of Media 25/50/75/100 (will be changed @ 75%)	Operational Per Manufactures' Specifications (If not, why?)
	Lat: _____ Long: _____	MWS Catch Basins						
		MWS Sedimentation Basin						
		Media Filter Condition						
		Plant Condition						
		Drain Down Media Condition						
		Discharge Chamber Condition						
		Drain Down Pipe Condition						
		Inlet and Outlet Pipe Condition						

Comments: _____



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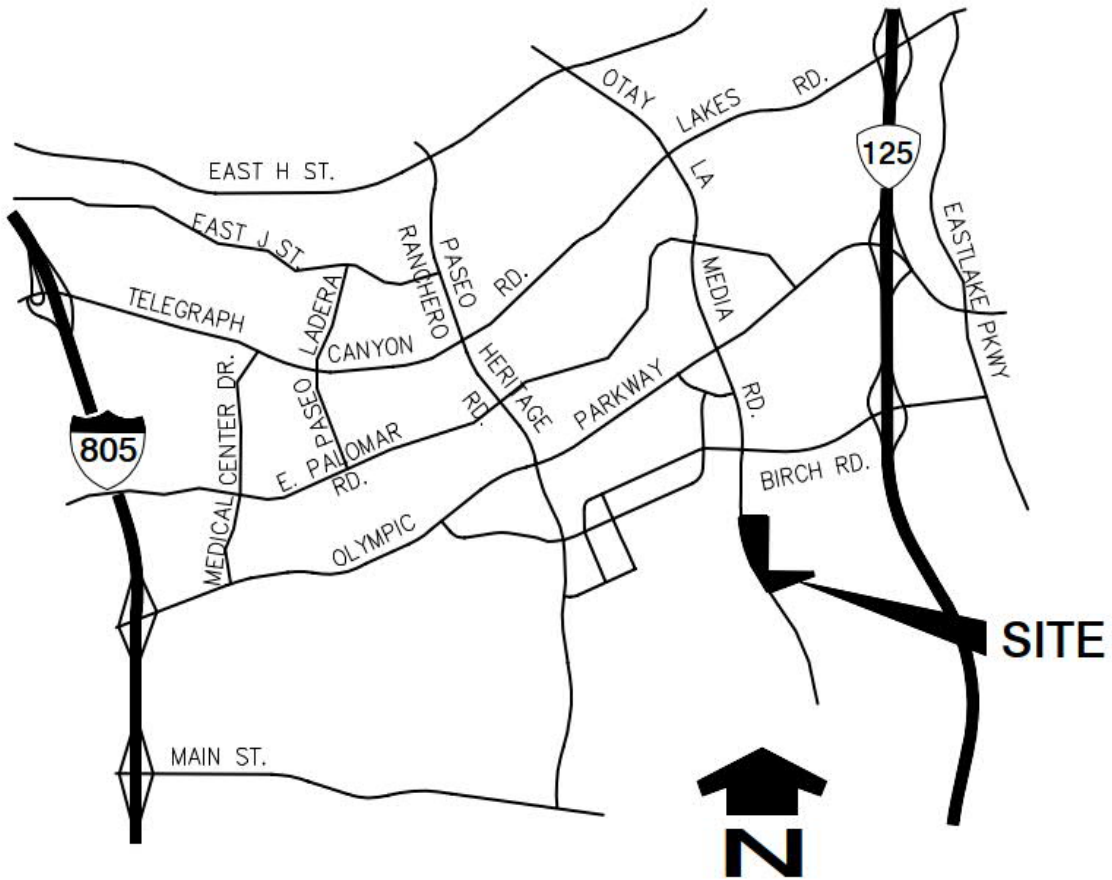
SUPPORT

DRAWINGS AND SPECIFICATIONS ARE AVAILABLE AT WWW.CONTECHES.COM

Modular Wetlands Maintenance Guide 08/22

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EXHIBIT "A"



VICINITY MAP

NOT TO SCALE

EXHIBIT "B"

Maintenance Recommendations and
Frequency Inspection Operation
and Maintenance Plan (IOMP)

SITE DESIGN, SOURCE CONTROL AND POLLUTANT CONTROL BMP OPERATION + MAINTENANCE PROCEDURE

STORM WATER MANAGEMENT AND DISCHARGE CONTROL MAINTENANCE AGREEMENT APPROVAL NO.:

O&M RESPONSIBLE PARTY DESIGNEE: PROPERTY OWNDER:

BMP DESCRIPTION	INSPECTION FREQUENCY	MAINTENANCE FREQUENCY	MAINTENANCE METHOD	QUANTITY	INCLUDED IN O&M MANUAL	SHEET NUMBERS
SITE DESIGN ELEMENTS						NO
DESCRIPTION: LANDSCAPE	WEEKLY	AS-NEEDED	MOW AS NECESSARY	223278 sf		
DESCRIPTION: RUNOFF COLLECTION	ANNUAL	AS-NEEDED	MAINTAIN DRIVEWAYS, CLEAN UP AREA DRAINS	1		
SOURCE CONTROL ELEMENTS						NO
DESCRIPTION: STORM DRAIN STENCILING	ANNUAL	BI-ANNUAL	REPAINT AS NECESSARY	1		
POLLUTANT CONTROL BMP(S)					YES	
DESCRIPTION: PROPRIETARY BIOFILTRATION UNIT	BI-ANNUAL	6-12 MONTHS AS NEEDED	REPLCMNT OF SOIL MATERIAL REMOVE DEBRIS AS NEEDED	3		

Project Name/_____

ATTACHMENT 4

Copy of Plan Sheets Showing Permanent Storm Water BMPs

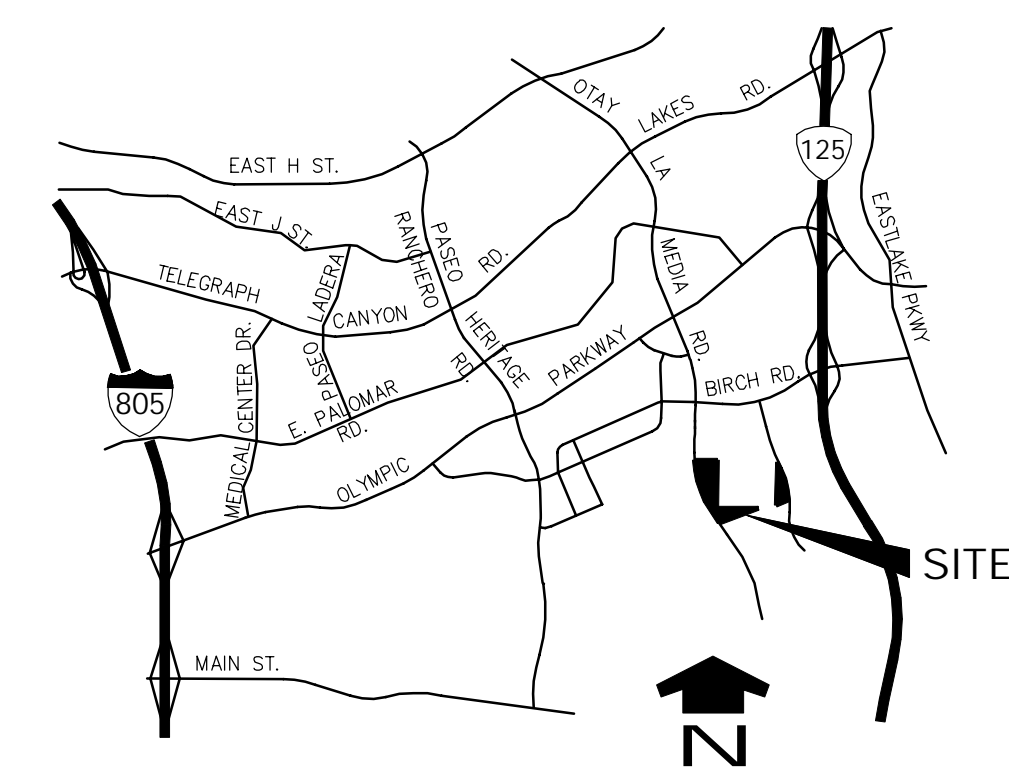
Project Name/_____

Use this checklist to ensure the required information has been included on the plans:

The plans must identify:

- Structural BMP(s) with ID numbers matching Form I-6 Summary of PDP Structural BMPs
- The grading and drainage design shown on the plans must be consistent with the delineation of DMAs shown on the DMA exhibit
- Details and specifications for construction of structural BMP(s)
- Signage indicating the location and boundary of structural BMP(s) as required by the City Engineer
- How to access the structural BMP(s) to inspect and perform maintenance
- Features that are provided to facilitate inspection (e.g., observation ports, cleanouts, silt posts, or other features that allow the inspector to view necessary components of the structural BMP and compare to maintenance thresholds)
- Manufacturer and part number for proprietary parts of structural BMP(s) when applicable
- Maintenance thresholds specific to the structural BMP(s), with a location-specific frame of reference (e.g., level of accumulated materials that triggers removal of the materials, to be identified based on viewing marks on silt posts or measured with a survey rod with respect to a fixed benchmark within the BMP)
- Recommended equipment to perform maintenance
- When applicable, necessary special training or certification requirements for inspection and maintenance personnel such as confined space entry or hazardous waste management
- Include landscaping plan sheets showing vegetation requirements for vegetated structural BMP(s)
- All BMPs must be fully dimensioned on the plans
- When proprietary BMPs are used, site specific cross section with outflow, inflow and model number shall be provided. Broucher photocopies are not allowed.

TENTATIVE MAP CVT 23-0001 CONCEPTUAL GRADING PLAN OTAY RANCH VILLAGE 7 R-3, R-4 & R-8 CITY OF CHULA VISTA, CALIFORNIA



LEGEND

- SUBDIVISION BOUNDARY
- EXISTING TOPO CONTOUR
- LOT LINE
- RESIDENTIAL LOT
- OPEN SPACE LOT
- NEIGHBORHOOD
- BROW DITCH
- SLOPE (2:1 MAX)
- PERCENT OF GRADE
- STREET ELEVATION
- SEWER MAIN
- SEWER INVERT ELEVATION
- SEWER FORCE MAIN
- WATER MAIN
- RECLAIMED WATER MAIN
- STORM DRAIN
- RETAINING WALL
- STREET LIGHT
- EASEMENT LINE
- EASEMENT NUMBER

BOUNDARY DATA

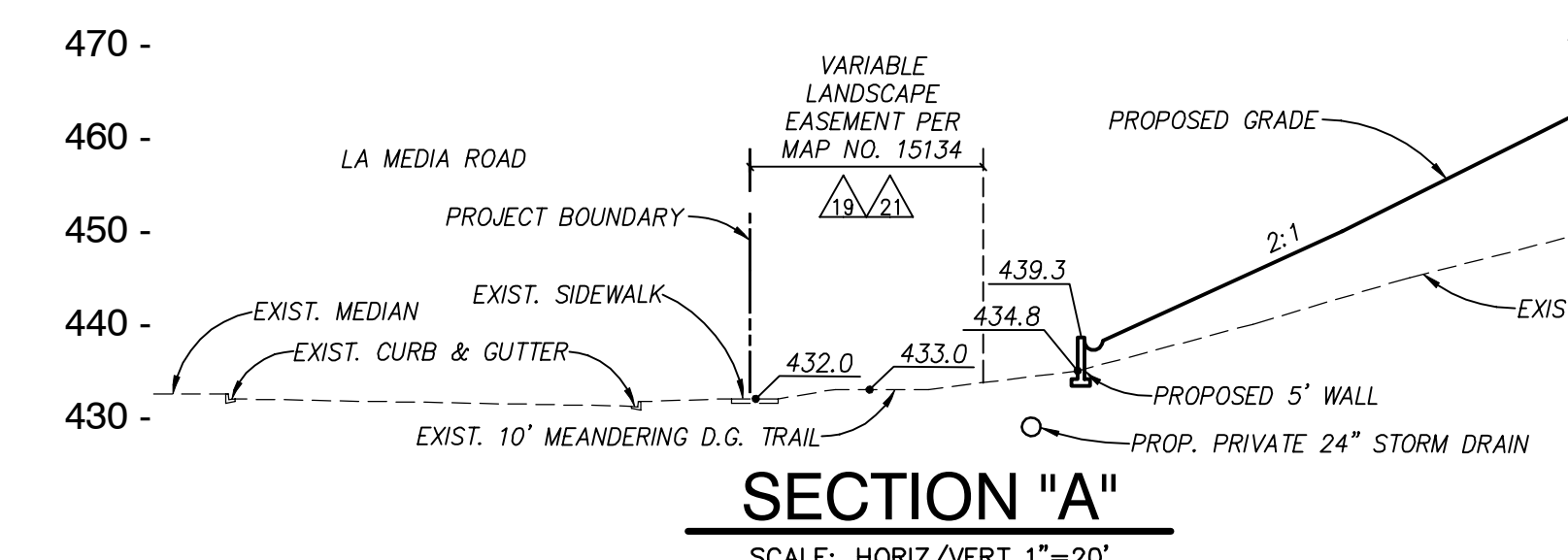
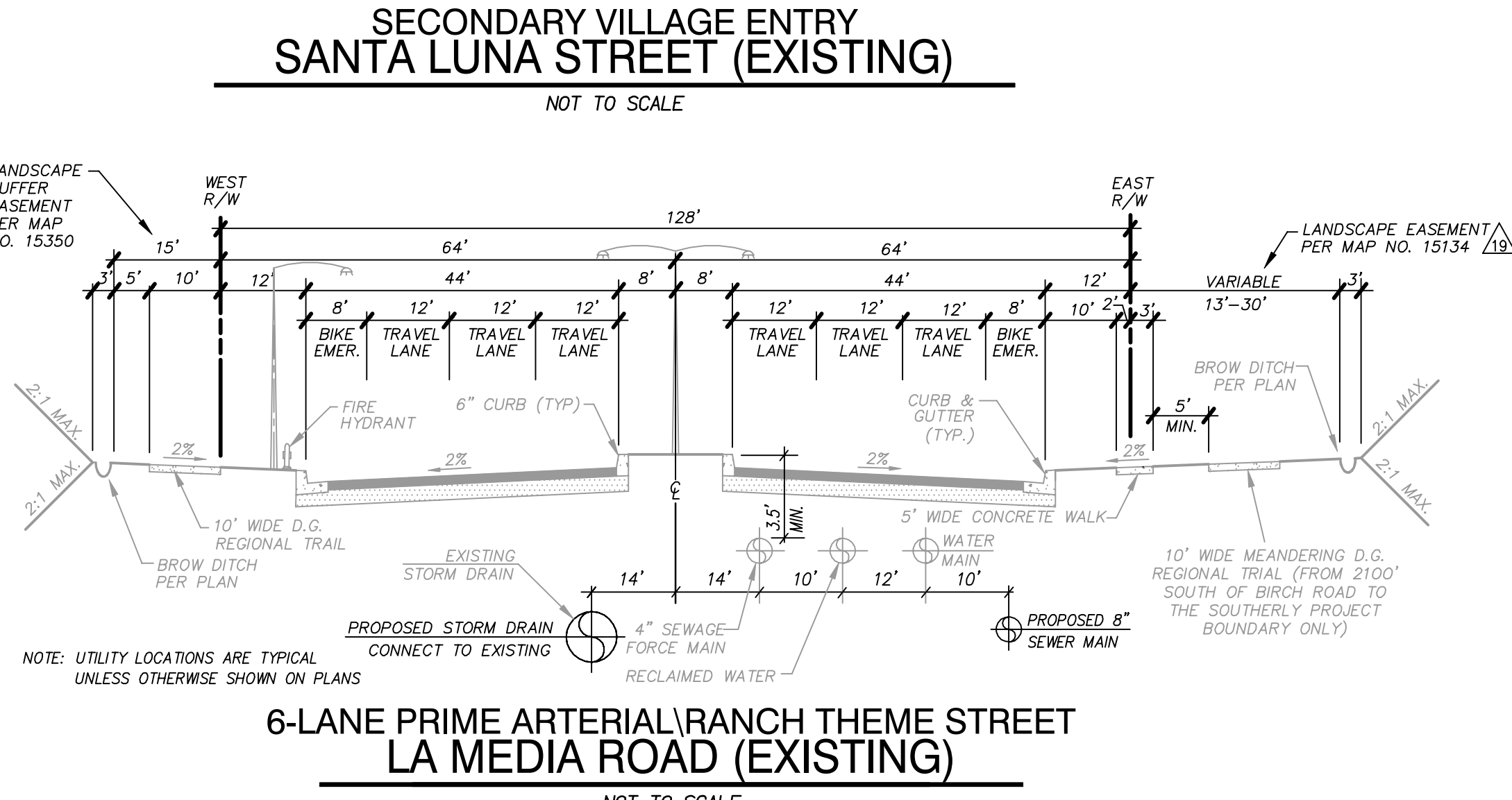
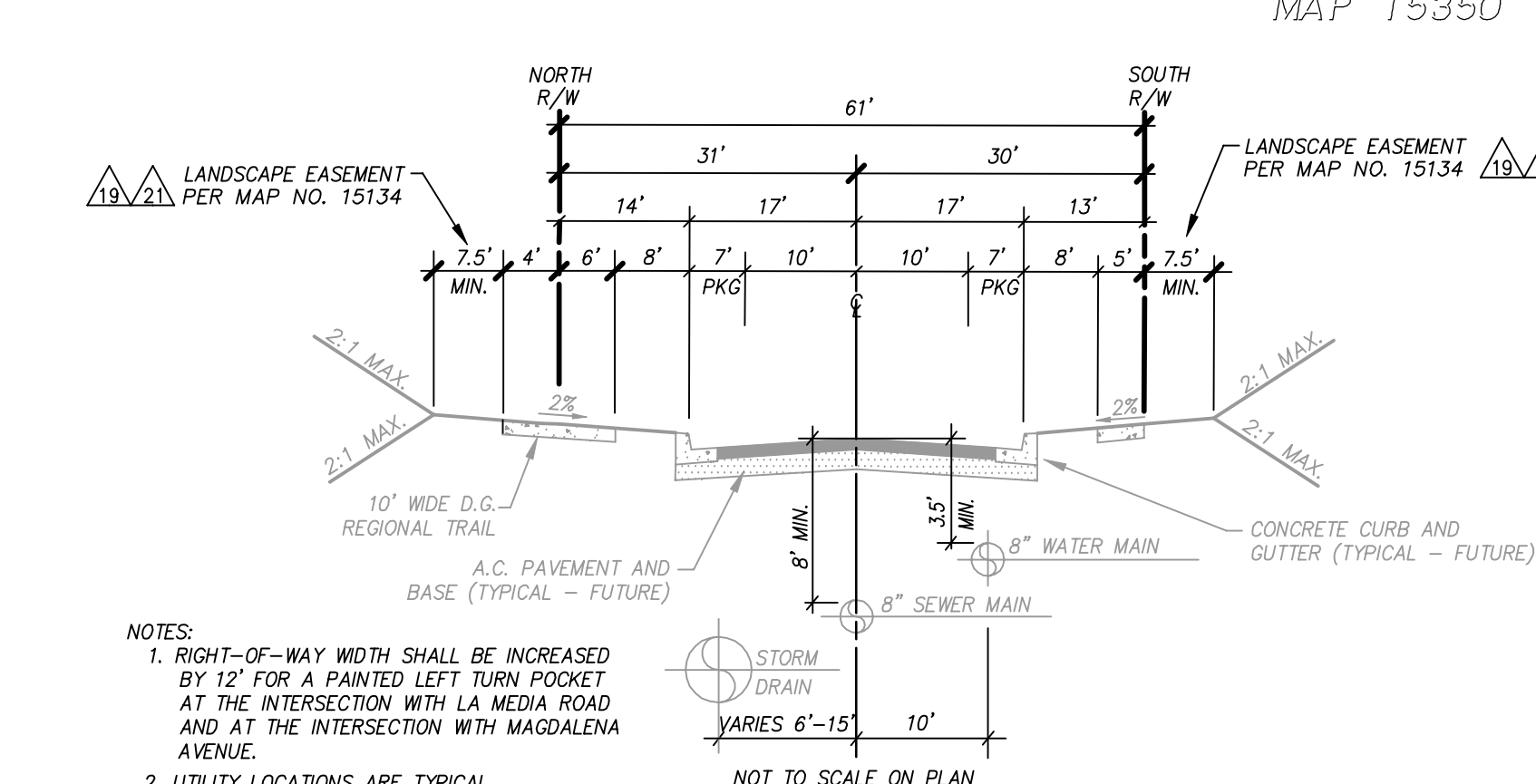
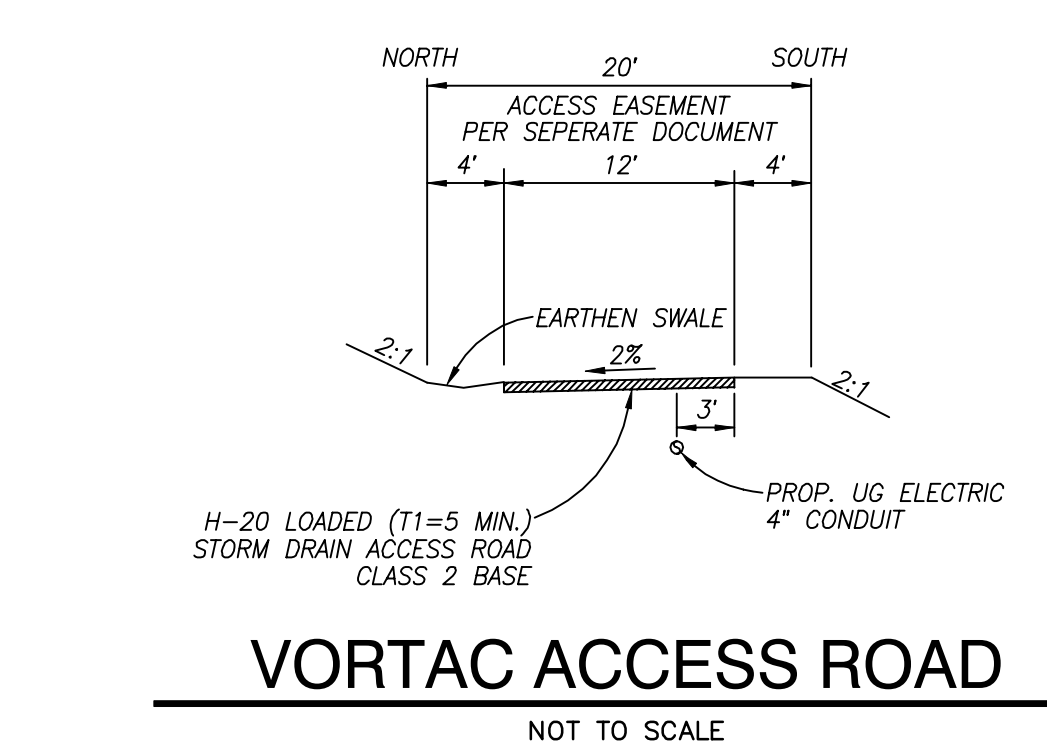
#	BEARING	DISTANCE
L1	N73°41'09"E	69.27'
L2	N16°21'44"W	6.62'
L3	N73°58'16"E	15.00'
L4	N16°21'44"W	25.00'
L5	N62°25'00"E	39.00'
L6	N55°50'00"E	51.00'
L7	N65°45'00"E	39.00'
L8	N63°30'00"E	45.15'
L9	N71°22'00"E	70.68'
L10	N89°36'41"W	118.47'
L11	N02°23'19"E	149.99'
L12	N89°36'41"W	1295.51'
L13	N71°57'57"E	36.39'
L14	N89°36'41"W	972.19'
L15	N55°20'40"E	19.56'
L16	N83°12'07"W	24.08'
L17	N34°39'20"W	471.34'
L18	N11°58'51"E	24.76'
L19	N55°20'40"E	19.56'
L20	N78°19'35"E	49.21'
L21	N89°36'41"W	730.10'
L22	N71°57'57"E	891.19'
L23	N34°39'20"W	212.23'

SUMMARY TABLE

DESC.	LAND USE	EXISTING ZONE	PROPOSED ZONE	LOT AREA	NO. OF UNITS
LOT 1	MULTI-FAMILY	SF3	RM	7.63 A.C.	123
LOT 2	MULTI-FAMILY	SF3	RM	0.62 A.C.	-
LOT 3	MULTI-FAMILY	SF4	RM2	3.11 A.C.	121
LOT 4	MULTI-FAMILY	SF3	RM	2.35 A.C.	43
A	OPEN SPACE	SF3	RM	3.76 A.C.	-
B	OPEN SPACE	SF3	RM	0.59 A.C.	-
ROW	OPEN SPACE	-	-	0.74 A.C.	-
TOTAL	-	-	-	18.80 A.C.	287

CURVE TABLE

#	DELTA	RADIUS	LENGTH
C1	06°38'34"	297.00'	34.43'
C2	47°28'50"	141.00'	117.26'
C3	04°04'04"	1860.00'	132.05'
C4	22°44'41"	115.00'	45.65'
C5	13°19'14"	214.05'	49.76'
C6	21°43'17"	437.00'	165.67'
C7	18°04'51"	2936.00'	926.92'
C8	22°58'55"	364.00'	146.00'
C9	12°03'44"	170.00'	35.79'



CIVIL ENGINEER
HUNSAKER & ASSOCIATES, SAN DIEGO, INC
9707 MAPLES STREET
SAN DIEGO, CA 92121
(858) 558-4500



ALISA S. VIALPANDO R.C.E. 47945 DATE

GENERAL NOTES

- GROSS PROJECT AREA: 18.8 AC
- TOTAL NUMBER OF EXISTING LOTS: 3
- TOTAL NUMBER OF PROPOSED LOTS: 6 (4 MULTI-FAMILY RESIDENTIAL, 2 OPEN SPACE)
- TOTAL NUMBER OF UNITS: 287 (123 UNITS ON LOT 1, 121 UNITS ON LOT 3 & 43 UNITS ON LOT 4)
- ASSESSOR PARCEL NUMBERS: 644-241-07, 644-241-08 & 644-241-10
- EXISTING ZONING: SF3/SF4 (R-3 SF3, R-4 SF4, R-8 SF3)
- PROPOSED ZONING: RM1/RM2 (R-3 RM1, R-4 RM2 & R-8 RM1)
- EXISTING GP DESIGNATION: LM (R-3, R-4 & R-8)
- PROPOSED GP DESIGNATION: M, MH & TC (R-3 MH, R-4 TC & R-8 MH)
- DENSITY: R-3 17.9 DU/AC, R-4 38.9 DU/AC & R-8 15.0 DU/AC
- PRESENT LAND USE: VACANT
- PROPOSED LAND USE: MULTI-FAMILY, OPEN SPACE
- SPECIFIC METHODS OF HANDLING STORM WATER ARE SUBJECT TO DETAILED APPROVAL BY THE CITY ENGINEER AT THE TIME OF SUBMISSION OF IMPROVEMENT AND GRADING PLANS. DESIGN SHALL BE ACCOMPLISHED ON THE BASIS OF THE REQUIREMENTS OF THE SUBDIVISION MANUAL. DRAINAGE EASEMENTS SHALL BE PROVIDED AS REQUIRED BY THE CITY ENGINEER.
- ALL SEWER MAINS ARE PUBLIC AND SHALL BE MAINTAINED BY THE CITY OF CHULA VISTA. PUBLIC SEWER EASEMENTS SHALL BE DEDICATED ON THE FINAL MAP.
- STREET TREE DEPOSITS SHALL BE PAID IN ACCORDANCE WITH ORDINANCE NUMBER 1369 AND NO. 1687 OF THE CITY OF CHULA VISTA.
- UTILITIES SHALL BE UNDERGROUND AND EASEMENTS PROVIDED AS NECESSARY. GRADING SHALL BE CONSTRUCTED IN ACCORDANCE WITH ORDINANCE NO. 1797 (AS AMENDED BY ORDINANCE NO. 1877) OF THE CITY OF CHULA VISTA.
- ALL WORK IN THE PUBLIC RIGHT-OF-WAY SHALL BE DONE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION, THE SAN DIEGO AREA REGIONAL STANDARD DRAWINGS AND THE DESIGN AND CONSTRUCTION STANDARDS OF THE CITY OF CHULA VISTA.
- FIRE HYDRANTS TO BE INSTALLED IN ACCORDANCE WITH THE CITY OF CHULA VISTA DESIGN STANDARD NO. 8. FIRE HYDRANTS AS SHOWN ON THIS MAP SHALL BE USED AS A GUIDE ONLY.
- GRADING SHOWN HEREON IS PRELIMINARY AND SUBJECT TO MODIFICATION IN FINAL DESIGN SUBJECT TO SUBSTANTIAL CONFORMANCE APPROVAL BY THE CITY OF CHULA VISTA ENGINEERING DEPARTMENT.
- STORM DRAIN, WATER AND SEWER SHOWN HEREON IS PRELIMINARY AND SUBJECT TO MODIFICATION IN FINAL DESIGN.
- PRELIMINARY GEOTECHNICAL REPORT PREPARED BY: GECON INCORPORATED
- SOURCE OF TOPOGRAPHY: R.J. LUNG & ROUGH GRADING PLANS BY HUNSAKER & ASSOCIATES
- TEMPORARY AND PERMANENT STRUCTURAL BEST MANAGEMENT PRACTICES WILL BE INCORPORATED IN THE DESIGN AND IMPLEMENTATION OF THE DEVELOPMENT.
- REFER TO THE "WATER QUALITY TECHNICAL REPORT" PREPARED BY HUNSAKER & ASSOCIATES FOR COMPLIANCE WITH THE REGIONAL WATER QUALITY CONTROL BOARD JURISDICTIONAL PERMIT.
- EARTHWORK QUANTITIES: CUT 307,700 C.Y., FILL 102,278 C.Y.

OWNERS/APPLICANTS

LOT "D"
OTAY PROJECT, L.P.
A CALIFORNIA LIMITED PARTNERSHIP
20 CORPORATE PLAZA DR
NEWPORT BEACH, CA 92660
(949) 640-8300
BY: ORYLE MANAGEMENT, LLC
A DELAWARE LIMITED LIABILITY CO.
ITS: GENERAL PARTNER

LOT "C" & "K"
BALDWIN & SONS LLC
A CALIFORNIA LIMITED LIABILITY CO.
20 CORPORATE PLAZA DR
NEWPORT BEACH, CA 92660
(949) 640-8300

CONDOMINIUM NOTE

THIS IS A MAP OF A CONDOMINIUM PROJECT AS DEFINED IN SECTION 4125 OF THE CIVIL CODE OF THE STATE OF CALIFORNIA AND IS FILED PURSUANT TO THE SUBDIVISION MAP ACT. THE TOTAL NUMBER OF RESIDENTIAL CONDOMINIUM DWELLING UNITS IS 287 (123 UNITS ON LOT 1, 121 UNITS ON LOT 3 & 43 UNITS ON LOT 4).

LEGAL DESCRIPTION

SEE SHEET C2

EASEMENTS

SEE SHEET C2

PUBLIC UTILITIES

SEWER: CITY OF CHULA VISTA
WATER: OTAY WATER DISTRICT
STORM DRAIN: CITY OF CHULA VISTA
TELEPHONE: AT&T
GAS AND ELECTRIC: SDC&E
CABLE T.V.: COX COMMUNICATIONS
POLICE & FIRE: CITY OF CHULA VISTA
SCHOOLS: CHULA VISTA ELEMENTARY SCHOOL DIST. SWEETWATER UNION HIGH SCHOOL DIST.

SHEET INDEX

SHEET C1 TENTATIVE MAP/ CONCEPTUAL GRADING PLAN
SHEET C2 BOUNDARY & ENCUMBRANCES
SHEET C3 BOUNDARY & ENCUMBRANCES

NO.	REVISIONS	DATE	BY
1	ORIGINAL	4-07-23	H&A
2	REVISED FOR 2ND SUB	12-07-23	H&A
3	REVISED FOR 3RD SUB	05-24-24	H&A
4	REVISED FOR 4TH SUB	07-30-24	H&A
5			
6			
7			
8			

PREPARED BY:
HUNSAKER & ASSOCIATES
SAN DIEGO, INC
PLANNING: 9707 Maples Street
ENGINEERING: San Diego, CA 92121
SURVEYING: PH858558-4500; PH858558-4144

TENTATIVE MAP
CONCEPTUAL GRADING PLAN
VILLAGE 7
R-3, R-4 & R-8
CITY OF CHULA VISTA, CALIFORNIA

SHEET
C1

ON SITE

OWNERS: BALDWIN & SONS AS TO LOT C OTAY PROJECT AS TO LOT D
APNS: 644-241-07-00 (LOT C) AND 644-241-08-00 (LOT D)

ONSITE

OWNER: BALDWIN & SONS, LOT "K" MAP NO. 15134
APN: 644-241-10-00

VORTAC - OFFSITE

OWNER: UNITED STATES OF AMERICA
APN:644-241-06-00

LOT 12 MAP NO. 15014 - OFFSITE

OWNER: MCMILLIN ROLLING HILLS RANCH, LLC
APN: 644-241-05-00

LEGAL DESCRIPTION

REAL PROPERTY IN THE CITY OF CHULA VISTA, COUNTY OF SAN DIEGO, STATE OF CALIFORNIA, DESCRIBED AS FOLLOWS:
LOTS C AND D OF CHULA VISTA TRACT NO. 05-09, OTAY BRANCH VILLAGE 7 "A" MAP NO. 1, IN THE CITY OF CHULA VISTA, COUNTY OF SAN DIEGO, STATE OF CALIFORNIA ACCORDING TO MAP THEREOF NO. 15134, FILED IN THE OFFICE OF COUNTY RECORDER OF SAN DIEGO COUNTY ON SEPTEMBER 27, 2005, AMENDED PURSUANT TO THAT CERTAIN CERTIFICATE OF CORRECTION RECORDED JUNE 6, 2008 AS INSTRUMENT NO. 2008-005167 OF OFFICIAL RECORDS.

TITLE REFERENCE

THE INFORMATION SHOWN HEREON IS BASED ON THE PRELIMINARY TITLE REPORT ISSUED BY FIRST AMERICAN TITLE COMPANY AS ORDER NO. NHC5-698105 (JD) DATED FEBRUARY 21, 2023 AND AMENDED FEBRUARY 28, 2023.

ASSESSOR'S PARCEL NUMBERS

644-241-07-00 (LOT C) AND 644-241-08-00 (LOT D)

TITLE EXCEPTIONS

- 1. GENERAL AND SPECIAL TAXES AND ASSESSMENTS FOR THE FISCAL YEAR 2023-2024, A LIEN NOT YET DUE OR PAYABLE.
2. GENERAL AND SPECIAL TAXES AND ASSESSMENTS FOR THE FISCAL YEAR 2022-2023.
3. GENERAL AND SPECIAL TAXES AND ASSESSMENTS FOR THE FISCAL YEAR 2022-2023.
4. THE LIEN OF SPECIAL TAX ASSESSED PURSUANT TO CHAPTER 2.5 COMMENCING WITH SECTION 53311 OF THE CALIFORNIA GOVERNMENT CODE FOR COMMUNITY FACILITIES DISTRICT NO. 6 OF THE CHULA VISTA ELEMENTARY SCHOOL, AS DISCLOSED BY NOTICE OF SPECIAL TAX LIEN RECORDED SEPTEMBER 15, 1998 AS INSTRUMENT NO. 1998-0584273 OF OFFICIAL RECORDS.
5. THE LIEN OF SPECIAL TAX ASSESSED PURSUANT TO CHAPTER 2.5 COMMENCING WITH SECTION 53311 OF THE CALIFORNIA GOVERNMENT CODE FOR COMMUNITY FACILITIES DISTRICT NO. 11, AS DISCLOSED BY NOTICE OF SPECIAL TAX LIEN RECORDED OCTOBER 29, 1998 AS INSTRUMENT NO. 1998-0703797 OF OFFICIAL RECORDS.
6. THE LIEN OF SPECIAL TAX ASSESSED PURSUANT TO CHAPTER 2.5 COMMENCING WITH SECTION 53311 OF THE CALIFORNIA GOVERNMENT CODE FOR COMMUNITY FACILITIES DISTRICT NO. 11, AS DISCLOSED BY NOTICE OF SPECIAL TAX LIEN RECORDED DECEMBER 29, 1998 AS INSTRUMENT NO. 1998-0854904 OF OFFICIAL RECORDS.
7. THE LIEN OF SPECIAL TAX ASSESSED PURSUANT TO CHAPTER 2.5 COMMENCING WITH SECTION 53311 OF THE CALIFORNIA GOVERNMENT CODE FOR COMMUNITY FACILITIES DISTRICT NO. 6 SWEETWATER UNION HIGH SCHOOL DISTRICT, AS DISCLOSED BY NOTICE OF SPECIAL TAX LIEN RECORDED DECEMBER 29, 1998 AS INSTRUMENT NO. 1998-0854905 OF OFFICIAL RECORDS.
8. THE LIEN OF SPECIAL TAX ASSESSED PURSUANT TO CHAPTER 2.5 COMMENCING WITH SECTION 53311 OF THE CALIFORNIA GOVERNMENT CODE FOR COMMUNITY FACILITIES DISTRICT NO. 97-2 (PRESERVE MAINTENANCE DISTRICT), AS DISCLOSED BY NOTICE OF SPECIAL TAX LIEN RECORDED AUGUST 21, 2001 AS INSTRUMENT NO. 2001-0594092 OF OFFICIAL RECORDS.
9. THE LIEN OF SUPPLEMENTAL TAXES, IF ANY, ASSESSED PURSUANT TO CHAPTER 3.5 COMMENCING WITH SECTION 75 OF THE CALIFORNIA REVENUE AND TAXATION CODE.
10. AN EASEMENT FOR ROADWAY AND UTILITY LINES AND INCIDENTAL PURPOSES, RECORDED APRIL 23, 1980 AS INSTRUMENT NO. 80-137651 OF OFFICIAL RECORDS.
11. AN EASEMENT FOR PUBLIC UTILITIES, INGRESS AND EGRESS AND INCIDENTAL PURPOSES, RECORDED JUNE 14, 1982 AS INSTRUMENT NO. 82-181896 OF OFFICIAL RECORDS.
12. AN EASEMENT FOR INGRESS AND EGRESS AND INCIDENTAL PURPOSES, RECORDED NOVEMBER 4, 1988 AS INSTRUMENT NO. 88-567316 OF OFFICIAL RECORDS.
13. THE TERMS AND PROVISIONS CONTAINED IN THE DOCUMENT ENTITLED "AGREEMENT INDEMNIFICATION, IMPLEMENTATION OF MITIGATION MEASURES, AND PAYMENT OF CERTAIN FEES IN CONNECTION WITH THE APPROVAL OF THE GENERAL PLAN AMENDMENT, GENERAL AND OTHER DEVELOPMENT PLANS FOR THE OTAY RANCH" RECORDED FEBRUARY 7, 1994 AS INSTRUMENT NO. 1994-0084743 OF OFFICIAL RECORDS.
14. THE TERMS AND PROVISIONS CONTAINED IN THE DOCUMENT ENTITLED "RESTATED AND AMENDED PRE-ANNEXATION DEVELOPMENT AGREEMENT WITH OTAY RANCH, L.P." RECORDED MAY 12, 1997 AS INSTRUMENT NO. 1997-0219970 OF OFFICIAL RECORDS.
15. THE TERMS AND PROVISIONS CONTAINED IN THE DOCUMENT ENTITLED "DETENTION BASIN AND SILTATION AGREEMENT" RECORDED SEPTEMBER 26, 1997 AS INSTRUMENT NO. 1997-0477304 OF OFFICIAL RECORDS.
16. THE TERMS AND PROVISIONS CONTAINED IN THE DOCUMENT ENTITLED "DESILTATION AND MAINTENANCE AGREEMENT WITH OTAY PROJECT, L.P. (POGGI CANYON DRAINAGE IMPROVEMENTS)" RECORDED JUNE 28, 2000 AS INSTRUMENT NO. 2000-0341829 OF OFFICIAL RECORDS.
17. THE TERMS AND PROVISIONS CONTAINED IN THE DOCUMENT ENTITLED "DRAINAGE MAINTENANCE AND INDEMNIFICATION AGREEMENT (OTAY RANCH VILLAGE 7 MASS GRADING)," RECORDED FEBRUARY 2, 2005 AS INSTRUMENT NO. 2005-0089011 OF OFFICIAL RECORDS.

TITLE EXCEPTIONS (CONT.)

- 18. THE TERMS AND PROVISIONS CONTAINED IN THE DOCUMENT ENTITLED "SUPPLEMENTAL SUBDIVISION IMPROVEMENT AGREEMENT FOR THE OTAY RANCH VILLAGE SEVEN "A" MAP" RECORDED SEPTEMBER 27, 2005 AS INSTRUMENT NO. 2005-0835113 OF OFFICIAL RECORDS.
19. DOCUMENT(S) DECLARING MODIFICATIONS THEREOF RECORDED FEBRUARY 13, 2007 AS INSTRUMENT NO. 2007-0099449 OF OFFICIAL RECORDS.
20. COVENANTS, CONDITIONS, RESTRICTIONS, EASEMENTS, ASSESSMENTS, LIENS, CHARGES, TERMS AND PROVISIONS IN THE DOCUMENT RECORDED OCTOBER 4, 2005 AS INSTRUMENT NO. 2005-0856104 OF OFFICIAL RECORDS, WHICH PROVIDE THAT A VIOLATION THEREOF SHALL NOT DEFEAT OR RENDER INVALID THE LIEN OF ANY FIRST MORTGAGE OR DEED OF TRUST MADE IN GOOD FAITH AND FOR VALUE, BUT DELETING ANY COVENANT, CONDITION, OR RESTRICTION, IF ANY, INDICATING A PREFERENCE, LIMITATION, OR DISCRIMINATION BASED ON RACE, COLOR, RELIGION, SEX, GENDER, GENDER IDENTITY, GENDER EXPRESSION, SEXUAL ORIENTATION, FAMILIAL STATUS, MARITAL STATUS, DISABILITY, HANDICAP, VETERAN OR MILITARY STATUS, GENETIC INFORMATION, NATIONAL ORIGIN, SOURCE OF INCOME AS DEFINED IN SUBDIVISION (P) OF SECTION 12955, OR ANCESTRY, TO THE EXTENT THAT SUCH COVENANTS, CONDITIONS OR RESTRICTIONS VIOLATE APPLICABLE STATE OR FEDERAL LAWS, LAWFUL RESTRICTIONS UNDER STATE AND FEDERAL LAW ON THE AGE OF OCCUPANTS IN SENIOR HOUSING OR HOUSING FOR OLDER PERSONS SHALL NOT BE CONSTRUED AS RESTRICTIONS BASED ON FAMILIAL STATUS.
21. AN EASEMENT SHOWN OR DEDICATED ON THE MAP AS REFERRED TO IN THE LEGAL DESCRIPTION FOR: LANDSCAPE BUFFER, SIGHT VISIBILITY AND INCIDENTAL PURPOSES.
22. COVENANTS, CONDITIONS, RESTRICTIONS, EASEMENTS, ASSESSMENTS, LIENS, CHARGES, TERMS AND PROVISIONS IN THE DOCUMENT RECORDED OCTOBER 4, 2005 AS INSTRUMENT NO. 2005-0856104 OF OFFICIAL RECORDS, WHICH PROVIDE THAT A VIOLATION THEREOF SHALL NOT DEFEAT OR RENDER INVALID THE LIEN OF ANY FIRST MORTGAGE OR DEED OF TRUST MADE IN GOOD FAITH AND FOR VALUE, BUT DELETING ANY COVENANT, CONDITION, OR RESTRICTION, IF ANY, INDICATING A PREFERENCE, LIMITATION, OR DISCRIMINATION BASED ON RACE, COLOR, RELIGION, SEX, GENDER, GENDER IDENTITY, GENDER EXPRESSION, SEXUAL ORIENTATION, FAMILIAL STATUS, MARITAL STATUS, DISABILITY, HANDICAP, VETERAN OR MILITARY STATUS, GENETIC INFORMATION, NATIONAL ORIGIN, SOURCE OF INCOME AS DEFINED IN SUBDIVISION (P) OF SECTION 12955, OR ANCESTRY, TO THE EXTENT THAT SUCH COVENANTS, CONDITIONS OR RESTRICTIONS VIOLATE APPLICABLE STATE OR FEDERAL LAWS, LAWFUL RESTRICTIONS UNDER STATE AND FEDERAL LAW ON THE AGE OF OCCUPANTS IN SENIOR HOUSING OR HOUSING FOR OLDER PERSONS SHALL NOT BE CONSTRUED AS RESTRICTIONS BASED ON FAMILIAL STATUS.
23. THE TERMS AND PROVISIONS CONTAINED IN THE DOCUMENT ENTITLED "AGREEMENT FOR THE PROVISION OF COMMUNITY PURPOSE FACILITY ACREE FOR OTAY RANCH VILLAGE TWO JPB SPA AMENDMENT" RECORDED NOVEMBER 19, 2012 AS INSTRUMENT NO. 2012-0722468 OF OFFICIAL RECORDS.
24. AN EASEMENT FOR UNDERGROUND FACILITIES AND APPURTENANCES FOR THE TRANSMISSION AND DISTRIBUTION OF ELECTRICITY, COMMUNICATION FACILITIES, AND APPURTENANCES AND INCIDENTAL PURPOSES, RECORDED APRIL 5, 2006 AS INSTRUMENT NO. 2006-0433349 OF OFFICIAL RECORDS.
25. AN EASEMENT FOR UNDERGROUND FACILITIES AND APPURTENANCES FOR THE TRANSMISSION AND DISTRIBUTION OF ELECTRICITY, COMMUNICATION FACILITIES, AND APPURTENANCES AND INCIDENTAL PURPOSES, RECORDED APRIL 5, 2006 AS INSTRUMENT NO. 2006-0433349 OF OFFICIAL RECORDS.
26. RIGHTS OF THE PUBLIC IN AND TO THAT PORTION OF THE LAND LYING WITHIN ANY ROAD, STREET, ALLEY OR HIGHWAY.
27. WATER RIGHTS, CLAIMS OR TITLE TO WATER, WHETHER OR NOT SHOWN BY THE PUBLIC RECORDS.
28. RIGHTS OF PARTIES IN POSSESSION.
29. WITH RESPECT TO BALDWIN & SONS, LLC, A CALIFORNIA LIMITED LIABILITY COMPANY:
A. A COPY OF ITS OPERATING AGREEMENT OR SIMILAR DOCUMENT AND ANY AMENDMENTS THERETO.
B. A OFFICIAL COPY OF ITS ARTICLES OF ORGANIZATION OR SIMILAR INCORPORATION DOCUMENT AND ANY CORRECTIONS, AMENDMENTS OR RESTATEMENTS THERETO.
C. EVIDENCE THAT THE LIMITED LIABILITY COMPANY IS PROPERLY FORMED AND IS IN GOOD STANDING IN THE STATE OF ITS DOMICILE.
D. OTHER REQUIREMENTS WHICH THE COMPANY MAY IMPOSE FOLLOWING ITS REVIEW OF THE MATERIAL REQUIRED HEREIN AND OTHER INFORMATION WHICH THE COMPANY MAY REQUIRE.
30. WITH RESPECT TO OTAY PROJECT L.P., A CALIFORNIA LIMITED PARTNERSHIP:
A. THAT A CERTIFIED COPY OF THE CERTIFICATE OF LIMITED PARTNERSHIP (FORM LP-1) AND ANY AMENDMENTS THERETO (FORM LP-2) BE RECORDED IN THE PUBLIC RECORDS;
B. A FULL COPY OF THE PARTNERSHIP AGREEMENT AND ANY AMENDMENTS;
C. SATISFACTORY EVIDENCE OF THE CONSENT OF A MAJORITY IN INTEREST OF THE LIMITED PARTNERS TO THE CONTEMPLATED TRANSACTION;
D. OTHER REQUIREMENTS WHICH THE COMPANY MAY IMPOSE FOLLOWING ITS REVIEW OF THE MATERIAL REQUIRED HEREIN AND OTHER INFORMATION WHICH THE COMPANY MAY REQUIRE.

LEGAL DESCRIPTION

REAL PROPERTY IN THE CITY OF CHULA VISTA, COUNTY OF SAN DIEGO, STATE OF CALIFORNIA, DESCRIBED AS FOLLOWS:
LOT K OF CHULA VISTA TRACT NO. 05-09, OTAY BRANCH VILLAGE 7 "A" MAP NO. 1, IN THE CITY OF CHULA VISTA, COUNTY OF SAN DIEGO, STATE OF CALIFORNIA ACCORDING TO MAP THEREOF NO. 15134, FILED IN THE OFFICE OF THE COUNTY RECORDER OF SAN DIEGO COUNTY ON SEPTEMBER 27, 2005.

TITLE REFERENCE

THE INFORMATION SHOWN HEREON IS BASED ON THE PRELIMINARY TITLE REPORT ISSUED BY FIRST AMERICAN TITLE COMPANY AS ORDER NO. NHC5-6985319 DATED JUNE 18, 2024.

ASSESSOR'S PARCEL NUMBER

644-241-10-00

TITLE EXCEPTIONS

- 1. GENERAL AND SPECIAL TAXES AND ASSESSMENTS FOR THE FISCAL YEAR 2024-2025, A LIEN NOT YET DUE OR PAYABLE.
2. THE LIEN OF SPECIAL TAX ASSESSED PURSUANT TO CHAPTER 2.5 COMMENCING WITH SECTION 53311 OF THE CALIFORNIA GOVERNMENT CODE FOR COMMUNITY FACILITIES DISTRICT NO. 6 OF THE CHULA VISTA ELEMENTARY SCHOOL, AS DISCLOSED BY NOTICE OF SPECIAL TAX LIEN RECORDED SEPTEMBER 15, 1998 AS INSTRUMENT NO. 1998-0584273 OF OFFICIAL RECORDS.
3. THE LIEN OF SPECIAL TAX ASSESSED PURSUANT TO CHAPTER 2.5 COMMENCING WITH SECTION 53311 OF THE CALIFORNIA GOVERNMENT CODE FOR COMMUNITY FACILITIES DISTRICT NO. 11, AS DISCLOSED BY NOTICE OF SPECIAL TAX LIEN RECORDED OCTOBER 29, 1998 AS INSTRUMENT NO. 1998-0703797 OF OFFICIAL RECORDS.
4. THE LIEN OF SPECIAL TAX ASSESSED PURSUANT TO CHAPTER 2.5 COMMENCING WITH SECTION 53311 OF THE CALIFORNIA GOVERNMENT CODE FOR COMMUNITY FACILITIES DISTRICT NO. 11, AS DISCLOSED BY NOTICE OF SPECIAL TAX LIEN RECORDED DECEMBER 29, 1998 AS INSTRUMENT NO. 1998-0854904 OF OFFICIAL RECORDS.
5. THE LIEN OF SPECIAL TAX ASSESSED PURSUANT TO CHAPTER 2.5 COMMENCING WITH SECTION 53311 OF THE CALIFORNIA GOVERNMENT CODE FOR COMMUNITY FACILITIES DISTRICT NO. 6 SWEETWATER UNION HIGH SCHOOL DISTRICT, AS DISCLOSED BY NOTICE OF SPECIAL TAX LIEN RECORDED DECEMBER 29, 1998 AS INSTRUMENT NO. 1998-0854905 OF OFFICIAL RECORDS.
6. DOCUMENT(S) DECLARING MODIFICATIONS THEREOF RECORDED MARCH 7, 2001 AS INSTRUMENT NO. 2001-0130147 OF OFFICIAL RECORDS.
7. THE LIEN OF SUPPLEMENTAL TAXES, IF ANY, ASSESSED PURSUANT TO CHAPTER 3.5 COMMENCING WITH SECTION 75 OF THE CALIFORNIA REVENUE AND TAXATION CODE.
8. INTENTIONALLY DELETED
9. INTENTIONALLY DELETED
10. INTENTIONALLY DELETED
11. THE TERMS AND PROVISIONS CONTAINED IN THE DOCUMENT ENTITLED "AGREEMENT INDEMNIFICATION, IMPLEMENTATION OF MITIGATION MEASURES, AND PAYMENT OF CERTAIN FEES IN CONNECTION WITH THE APPROVAL OF THE GENERAL PLAN AMENDMENT, GENERAL AND OTHER DEVELOPMENT PLANS FOR THE OTAY RANCH" RECORDED FEBRUARY 7, 1994 AS INSTRUMENT NO. 1994-0084743 OF OFFICIAL RECORDS.
12. THE TERMS AND PROVISIONS CONTAINED IN THE DOCUMENT ENTITLED "RESTATED AND AMENDED PRE-ANNEXATION DEVELOPMENT AGREEMENT WITH OTAY RANCH, L.P." RECORDED MAY 12, 1997 AS INSTRUMENT NO. 1997-0219970 OF OFFICIAL RECORDS.
13. INTENTIONALLY DELETED
14. INTENTIONALLY DELETED
15. THE TERMS AND PROVISIONS CONTAINED IN THE DOCUMENT ENTITLED "DRAINAGE MAINTENANCE AND INDEMNIFICATION AGREEMENT (OTAY RANCH VILLAGE 7 MASS GRADING)" RECORDED FEBRUARY 2, 2005 AS INSTRUMENT NO. 2005-0089011 OF OFFICIAL RECORDS.
16. AN EASEMENT SHOWN OR DEDICATED ON THE MAP AS REFERRED TO IN THE LEGAL DESCRIPTION FOR: ROAD AND INCIDENTAL PURPOSES.
17. THE TERMS AND PROVISIONS CONTAINED IN THE DOCUMENT ENTITLED "SUPPLEMENTAL SUBDIVISION IMPROVEMENT AGREEMENT FOR THE OTAY RANCH VILLAGE SEVEN "A" MAP" RECORDED SEPTEMBER 27, 2005 AS INSTRUMENT NO. 2005-0835113 OF OFFICIAL RECORDS.
18. COVENANTS, CONDITIONS, RESTRICTIONS, EASEMENTS, ASSESSMENTS, LIENS, CHARGES, TERMS AND PROVISIONS IN THE DOCUMENT RECORDED OCTOBER 4, 2005 AS INSTRUMENT NO. 2005-0856104 OF OFFICIAL RECORDS, WHICH PROVIDE THAT A VIOLATION THEREOF SHALL NOT DEFEAT OR RENDER INVALID THE LIEN OF ANY FIRST MORTGAGE OR DEED OF TRUST MADE IN GOOD FAITH AND FOR VALUE, BUT DELETING ANY COVENANT, CONDITION OR RESTRICTION INDICATING A PREFERENCE, LIMITATION OR DISCRIMINATION BASED ON RACE, COLOR, RELIGION, SEX, SEXUAL ORIENTATION, MARITAL STATUS, ANCESTRY, DISABILITY, HANDICAP, FAMILIAL STATUS, NATIONAL ORIGIN OR SOURCE OF INCOME (AS DEFINED IN CALIFORNIA GOVERNMENT CODE §12955(P)), TO THE EXTENT SUCH COVENANTS, CONDITIONS OR RESTRICTIONS VIOLATE 42 U.S.C. §3604(C) OR CALIFORNIA GOVERNMENT CODE §12955. LAWFUL RESTRICTIONS UNDER STATE AND FEDERAL LAW ON THE AGE OF OCCUPANTS IN SENIOR HOUSING OR HOUSING FOR OLDER PERSONS SHALL NOT BE CONSTRUED AS RESTRICTIONS BASED ON FAMILIAL STATUS.
19. DOCUMENT(S) DECLARING MODIFICATIONS THEREOF RECORDED FEBRUARY 13, 2007 AS INSTRUMENT NO. 2007-0099449 OF OFFICIAL RECORDS.
20. COVENANTS, CONDITIONS, RESTRICTIONS, EASEMENTS, ASSESSMENTS, LIENS, CHARGES, TERMS AND PROVISIONS IN THE DOCUMENT RECORDED OCTOBER 4, 2005 AS INSTRUMENT NO. 2005-0856104 OF OFFICIAL RECORDS, WHICH PROVIDE THAT A VIOLATION THEREOF SHALL NOT DEFEAT OR RENDER INVALID THE LIEN OF ANY FIRST MORTGAGE OR DEED OF TRUST MADE IN GOOD FAITH AND FOR VALUE, BUT DELETING ANY COVENANT, CONDITION OR RESTRICTION INDICATING A PREFERENCE, LIMITATION OR DISCRIMINATION BASED ON RACE, COLOR, RELIGION, SEX, SEXUAL ORIENTATION, MARITAL STATUS, ANCESTRY, DISABILITY, HANDICAP, FAMILIAL STATUS, NATIONAL ORIGIN OR SOURCE OF INCOME (AS DEFINED IN CALIFORNIA GOVERNMENT CODE §12955(P)), TO THE EXTENT SUCH COVENANTS, CONDITIONS OR RESTRICTIONS VIOLATE 42 U.S.C. §3604(C) OR CALIFORNIA GOVERNMENT CODE §12955. LAWFUL RESTRICTIONS UNDER STATE AND FEDERAL LAW ON THE AGE OF OCCUPANTS IN SENIOR HOUSING OR HOUSING FOR OLDER PERSONS SHALL NOT BE CONSTRUED AS RESTRICTIONS BASED ON FAMILIAL STATUS.
21. DOCUMENT(S) DECLARING MODIFICATIONS THEREOF RECORDED JUNE 1, 2006 AS INSTRUMENT NO. 2006-0389164 OF OFFICIAL RECORDS.
22. DOCUMENT RE-RECORDED JULY 11, 2006 AS INSTRUMENT NO. 2006-0487654 OF OFFICIAL RECORDS.
23. A DECLARATION OF ANNEXATION RECORDED JUNE 20, 2007 AS INSTRUMENT NO. 2007-0415537 OF OFFICIAL RECORDS.
24. DOCUMENT(S) DECLARING MODIFICATIONS THEREOF RECORDED AUGUST 30, 2006 AS INSTRUMENT NO. 2006-0619008 OF OFFICIAL RECORDS.
25. A DECLARATION OF ANNEXATION RECORDED JUNE 20, 2007 AS INSTRUMENT NO. 2007-0415537 OF OFFICIAL RECORDS.
26. DOCUMENT(S) DECLARING MODIFICATIONS THEREOF RECORDED MARCH 02, 2023 AS INSTRUMENT NO. 2023-0053648 OF OFFICIAL RECORDS.
27. A DECLARATION OF ANNEXATION RECORDED MARCH 02, 2023 AS INSTRUMENT NO. 2023-0053648 OF OFFICIAL RECORDS.

TITLE EXCEPTIONS (CONT.)

- 20. WATER RIGHTS, CLAIMS OR TITLE TO WATER, WHETHER OR NOT SHOWN BY THE PUBLIC RECORDS.
21. RIGHTS OF PARTIES IN POSSESSION.
22. WITH RESPECT TO BALDWIN & SONS, LLC, A CALIFORNIA LIMITED LIABILITY COMPANY:
A. A COPY OF ITS OPERATING AGREEMENT OR SIMILAR DOCUMENT AND ANY AMENDMENTS THERETO.
B. A OFFICIAL COPY OF ITS ARTICLES OF ORGANIZATION OR SIMILAR INCORPORATION DOCUMENT AND ANY CORRECTIONS, AMENDMENTS OR RESTATEMENTS THERETO.
C. EVIDENCE THAT THE LIMITED LIABILITY COMPANY IS PROPERLY FORMED AND IS IN GOOD STANDING IN THE STATE OF ITS DOMICILE.
D. OTHER REQUIREMENTS WHICH THE COMPANY MAY IMPOSE FOLLOWING ITS REVIEW OF THE MATERIAL REQUIRED HEREIN AND OTHER INFORMATION WHICH THE COMPANY MAY REQUIRE.
23. DOCUMENT(S) DECLARING MODIFICATIONS THEREOF RECORDED MARCH 7, 2001 AS INSTRUMENT NO. 2001-0130147 OF OFFICIAL RECORDS.
24. AN EASEMENT FOR UNDERGROUND FACILITIES AND APPURTENANCES FOR THE TRANSMISSION AND DISTRIBUTION OF ELECTRICITY, COMMUNICATION FACILITIES, AND APPURTENANCES AND INCIDENTAL PURPOSES, RECORDED APRIL 5, 2006 AS INSTRUMENT NO. 2006-0433349 OF OFFICIAL RECORDS.
25. AN EASEMENT FOR UNDERGROUND FACILITIES AND APPURTENANCES FOR THE TRANSMISSION AND DISTRIBUTION OF ELECTRICITY, COMMUNICATION FACILITIES, AND APPURTENANCES AND INCIDENTAL PURPOSES, RECORDED APRIL 5, 2006 AS INSTRUMENT NO. 2006-0433349 OF OFFICIAL RECORDS.
26. RIGHTS OF THE PUBLIC IN AND TO THAT PORTION OF THE LAND LYING WITHIN ANY ROAD, STREET, ALLEY OR HIGHWAY.
27. WATER RIGHTS, CLAIMS OR TITLE TO WATER, WHETHER OR NOT SHOWN BY THE PUBLIC RECORDS.
28. RIGHTS OF PARTIES IN POSSESSION.
29. WITH RESPECT TO BALDWIN & SONS, LLC, A CALIFORNIA LIMITED LIABILITY COMPANY:
A. A COPY OF ITS OPERATING AGREEMENT OR SIMILAR DOCUMENT AND ANY AMENDMENTS THERETO.
B. A OFFICIAL COPY OF ITS ARTICLES OF ORGANIZATION OR SIMILAR INCORPORATION DOCUMENT AND ANY CORRECTIONS, AMENDMENTS OR RESTATEMENTS THERETO.
C. EVIDENCE THAT THE LIMITED LIABILITY COMPANY IS PROPERLY FORMED AND IS IN GOOD STANDING IN THE STATE OF ITS DOMICILE.
D. OTHER REQUIREMENTS WHICH THE COMPANY MAY IMPOSE FOLLOWING ITS REVIEW OF THE MATERIAL REQUIRED HEREIN AND OTHER INFORMATION WHICH THE COMPANY MAY REQUIRE.
30. WITH RESPECT TO OTAY PROJECT L.P., A CALIFORNIA LIMITED PARTNERSHIP:
A. THAT A CERTIFIED COPY OF THE CERTIFICATE OF LIMITED PARTNERSHIP (FORM LP-1) AND ANY AMENDMENTS THERETO (FORM LP-2) BE RECORDED IN THE PUBLIC RECORDS;
B. A FULL COPY OF THE PARTNERSHIP AGREEMENT AND ANY AMENDMENTS;
C. SATISFACTORY EVIDENCE OF THE CONSENT OF A MAJORITY IN INTEREST OF THE LIMITED PARTNERS TO THE CONTEMPLATED TRANSACTION;
D. OTHER REQUIREMENTS WHICH THE COMPANY MAY IMPOSE FOLLOWING ITS REVIEW OF THE MATERIAL REQUIRED HEREIN AND OTHER INFORMATION WHICH THE COMPANY MAY REQUIRE.

LEGAL DESCRIPTION

REAL PROPERTY IN THE CITY OF CHULA VISTA, COUNTY OF SAN DIEGO, STATE OF CALIFORNIA, DESCRIBED AS FOLLOWS:
PORTIONS OF LOTS 22, 28 AND 29 OF OTAY RANCH, IN THE CITY OF CHULA VISTA, COUNTY OF SAN DIEGO, STATE OF CALIFORNIA, ACCORDING TO MAP THEREOF NO. 862, FILED IN THE OFFICE OF THE COUNTY RECORDER OF SAN DIEGO COUNTY, FEBRUARY 7, 1980, BEING A 1500-FOOT BY 1500-FOOT SQUARE PARCEL, DESCRIBED AS FOLLOWS:

COMMENCING AT THE SAN DIEGO CITY HORIZONTAL AND VERTICAL CONTROL POINT "38F948" HAVING ZONE VI LAMBERT COORDINATES OF 156973.960 NORTH, 1774867.290 EAST; THENCE NORTH 91°16'4" EAST A DISTANCE OF 5,283.79 FEET TO A POINT ABOUT THE COORDINATE LATITUDE 32°56'37.008" NORTH, LONGITUDE 116°58'41.568" WEST, SAID POINT HEREIN AFTER REFERRED TO AS POINT "A"; THENCE SOUTH 00°24'00.6" WEST A DISTANCE OF 750.00 FEET TO THE TRUE POINT OF BEGINNING; THENCE NORTH 89°35'58.4" WEST A DISTANCE OF 750.00 FEET; THENCE NORTH 00°24'00.6" EAST A DISTANCE OF 1500.00 FEET; THENCE SOUTH 89°35'58.4" EAST A DISTANCE OF 1500.00 FEET; THENCE SOUTH 00°24'00.6" WEST A DISTANCE OF 1500.00 FEET; THENCE NORTH 89°35'58.4" WEST A DISTANCE OF 750.00 FEET TO THE TRUE POINT OF BEGINNING.

EXCEPTING THEREFROM ALL OIL AND GAS IN AND UNDER SAID LAND AND ALL APPURTENANT RIGHTS FOR THE EXPLORATION, DEVELOPMENT, PRODUCTION, AND REMOVAL OF SAID OIL AND GAS, BUT WITHOUT THE RIGHT TO ENTER UPON OR OVER THE SURFACE OF SAID LAND FOR THE PURPOSE OF DRILLING AND EXTRACTING THEREFROM SAID OIL AND GAS; PROVIDED, HOWEVER, THAT SAID OIL AND GAS, AND APPURTENANT RIGHTS SO EXCEPTED AND EXCLUDED ARE SUBORDINATED TO THE PRIOR RIGHT OF THE UNITED STATES OF AMERICA TO UTILIZE THE LAND IN CONNECTION WITH THE OPERATION AND MAINTENANCE OF THE VORTAC FACILITY, AS EXCEPTED IN THE AMENDED COMPLAINT IN CONDEMNATION CIVIL NO. 79-0907-N RECORDED APRIL 23, 1980 AS INSTRUMENT NO. 80-137651, OF OFFICIAL RECORDS.

TITLE REFERENCE

THE INFORMATION SHOWN HEREON IS BASED ON THE PRELIMINARY TITLE REPORT ISSUED BY FIRST AMERICAN TITLE COMPANY AS ORDER NO. NHC5-6968467 (JD) DATED MARCH 22, 2023.

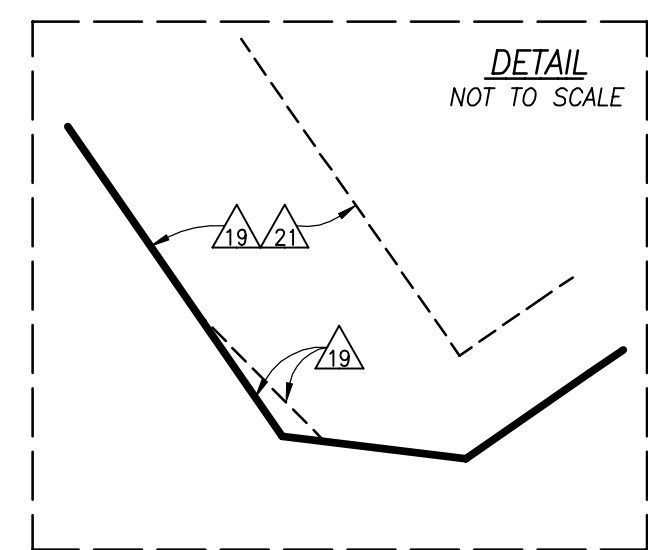
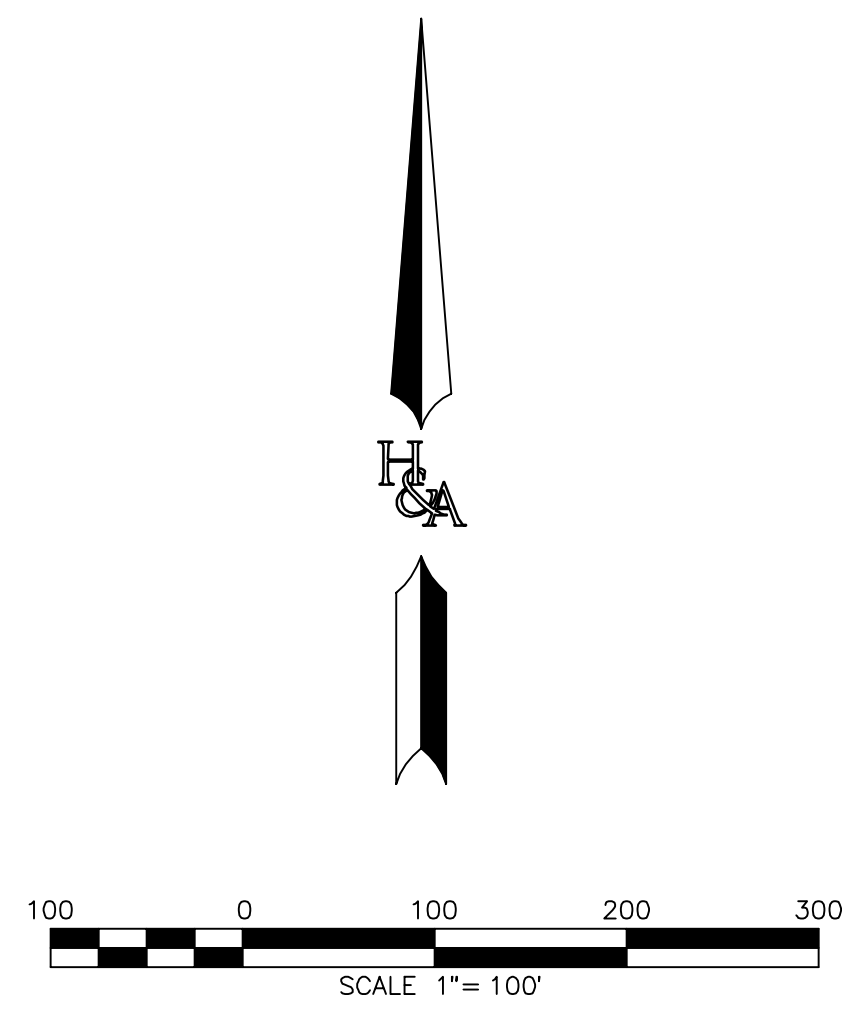
ASSESSOR'S PARCEL NUMBER

644-241-06-00


TITLE EXCEPTIONS

- 1. GENERAL AND SPECIAL TAXES AND ASSESSMENTS FOR THE FISCAL YEAR 2023-2024, A LIEN NOT YET DUE OR PAYABLE.
2. GENERAL AND SPECIAL TAXES AND ASSESSMENTS FOR THE FISCAL YEAR 2022-2023 ARE EXEMPT.
3. THE LIEN OF SUPPLEMENTAL TAXES, IF ANY, ASSESSED PURSUANT TO CHAPTER 3.5 COMMENCING WITH SECTION 75 OF THE CALIFORNIA REVENUE AND TAXATION CODE.
4. THE TERMS AND PROVISIONS CONTAINED IN THE DOCUMENT ENTITLED "AMENDED COMPLAINT IN CONDEMNATION CIVIL NO. 79-0907-N RECORDED ON APRIL 23, 1980 AS INSTRUMENT NO. 82-120168, OF OFFICIAL RECORDS.
5. AN EASEMENT FOR THE TRANSMISSION AND DISTRIBUTION OF ELECTRICITY AND INCIDENTAL PURPOSES, RECORDED AUGUST 21, 1980 AS INSTRUMENT NO. 80-267891 OF OFFICIAL RECORDS.
6. THE EFFECT OF A MAP PURPORTING TO SHOW THE LAND AND OTHER PROPERTY, FILED IN SURVEY MAP NO. 8723 OF RECORD OF SURVEYS.
7. AN EASEMENT TO CONSTRUCT AND MAINTAIN (PLACE, OPERATE, INSPECT, REPAIR, REPLACE AND REMOVE) SUCH AERIAL AND UNDERGROUND COMMUNICATION FACILITIES AND INCIDENTAL PURPOSES, RECORDED JUNE 14, 1982 AS INSTRUMENT NO. 82-181896 OF OFFICIAL RECORDS.
8. THE EFFECT OF A MAP PURPORTING TO SHOW THE LAND AND OTHER PROPERTY, FILED IN SURVEY MAP NO. 12371 OF RECORD OF SURVEYS.
9. WATER RIGHTS, CLAIMS OR TITLE TO WATER, WHETHER OR NOT SHOWN BY THE PUBLIC RECORDS.
10. RIGHTS OF PARTIES IN POSSESSION.
11. THE CONDITION THAT THE PROPERTY SHALL BE USED FOR THE SOLE PURPOSE OF PROVIDING, CONSTRUCTING AND MAINTAINING AFFORDABLE HOUSING, AS MORE FULLY DEFINED THEREIN, AND UPON THE TERMS, COVENANTS AND CONDITIONS IN THAT CERTAIN DOCUMENT RECORDED AUGUST 12, 2002 AS INSTRUMENT NO. 02-0679120 OF OFFICIAL RECORDS.
12. THE TERMS AND PROVISIONS CONTAINED IN THE DOCUMENT ENTITLED "DRAINAGE MAINTENANCE AND INDEMNIFICATION AGREEMENT RECORDED NOVEMBER 3, 2004 AS INSTRUMENT NO. 04-1042418 OF OFFICIAL RECORDS.
13. THE TERMS AND PROVISIONS CONTAINED IN THE DOCUMENT ENTITLED "SUPPLEMENTAL SUBDIVISION IMPROVEMENT AGREEMENT FOR THE MCMILLIN OTAY RANCH VILLAGE SEVEN "A" MAP" RECORDED MAY 6, 2005 AS INSTRUMENT NO. 05-0383256 OF OFFICIAL RECORDS.
14. AN EASEMENT SHOWN OR DEDICATED ON THE MAP AS REFERRED TO IN THE LEGAL DESCRIPTION FOR: ASSIGNABLE AND IRREVOCABLE GENERAL UTILITY, ACCESS, LANDSCAPE AND INCIDENTAL PURPOSES.
15. WATER RIGHTS, CLAIMS OR TITLE TO WATER, WHETHER OR NOT SHOWN BY THE PUBLIC RECORDS.
16. RIGHTS OF PARTIES IN POSSESSION.
17. ANY FACTS, RIGHTS, INTERESTS OR CLAIMS WHICH WOULD BE DISCLOSED BY A CORRECT ALTA/NPS/S SURVEY.
18. WITH RESPECT TO MCMILLIN ROLLING HILLS RANCH, LLC, A DELAWARE LIMITED LIABILITY COMPANY:
A. A COPY OF ITS OPERATING AGREEMENT OR SIMILAR DOCUMENT AND ANY AMENDMENTS THERETO.
B. A OFFICIAL COPY OF ITS ARTICLES OF ORGANIZATION OR SIMILAR INCORPORATION DOCUMENT AND ANY CORRECTIONS, AMENDMENTS OR RESTATEMENTS THERETO.
C. EVIDENCE THAT THE LIMITED LIABILITY COMPANY IS PROPERLY FORMED AND IS IN GOOD STANDING IN THE STATE OF ITS DOMICILE.
D. OTHER REQUIREMENTS WHICH THE COMPANY MAY IMPOSE FOLLOWING ITS REVIEW OF THE MATERIAL REQUIRED HEREIN AND OTHER INFORMATION WHICH THE COMPANY MAY REQUIRE.
19. WITH RESPECT TO MCMILLIN ROLLING HILLS RANCH, LLC, A DELAWARE LIMITED LIABILITY COMPANY, A CERTIFICATE OF REVIVOR AND A CERTIFICATE OF RELIEF FROM CONTRACT VIOLABILITY ISSUED BY THE FRANCHISE TAX BOARD OF THE STATE OF CALIFORNIA.
20. AN ALTA/NPS/S SURVEY OF RECENT DATE WHICH COMPLIES WITH THE CURRENT MINIMUM STANDARD DETAIL REQUIREMENTS FOR ALTA/NPS/S LAND TITLE SURVEYS.

PREPARED BY: HUNSAKER & ASSOCIATES SAN DIEGO, INC. 9707 Waples Street, San Diego, CA 92121. BOUNDARY & ENCUMBRANCES VILLAGE 7 R-3, R-4 & R-8 CITY OF CHULA VISTA, CALIFORNIA SHEET C2



MAP NO. 16428
 PARCEL D CERT. OF COMP.
 2022-0473499
 APN 644-071-33

<p>PREPARED BY:</p>  <p>HUNSAKER & ASSOCIATES SAN DIEGO, INC.</p> <p>PLANNING 9707 Waples Street ENGINEERING San Diego, Ca 92123 SURVEYING PH(619)558-4500 FX(619)558-1414</p>	<p>BOUNDARY & ENCUMBRANCES</p> <p>VILLAGE 7 R-3, R-4 & R-8 CITY OF CHULA VISTA, CALIFORNIA</p>	<p>SHEET</p> <p>C3</p>
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Project Name/_____

ATTACHMENT 5

Drainage Report

Attach project's drainage report. Refer to the Subdivision Manual to determine the reporting requirements.


PRELIMINARY DRAINAGE STUDY
for
TENTATIVE MAP CVT 23-0001
CONCEPTUAL GRADING PLAN
OTAY RANCH VILLAGE 7, R-3, R-4, & R-8

APN: 644-241-06, 644-241-07 & 644-241-08
C.V.T. 23-0001

Prepared for:
Baldwin and Sons LLC
20 Corporate Plaza Dr.
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&
Otay Project, L.P.
20 Corporate Plaza Dr.
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W.O. 2807-0121
May 10, 2024




Alisa Vialpando, RCE # 47945
Hunsaker & Associates
San Diego, Inc.

07/26/2024
Date

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Project Name/_____

ATTACHMENT 6

Project's Geotechnical and Groundwater Investigation Report

Attach project's geotechnical and groundwater investigation report. Refer to Appendix C.4 to determine the reporting requirements.